GENERAL NOTES: ROBBIAN DESIGN AL ROBBIAN A.I.B.D. 6397 CONNEWOOD SQ. NEW PORT RICHEY, FL 34653 (777) 848-2228 MAIL-al(@robbinndesign.com THE FOLLOWING TECHNICAL CODES SHALL APPLY: 2010 FLORIDA BUILDING CODE, PLUMBING, MECHANICAL, FUEL GAS, ENERGY EFFICIENCY, ACCESSIBILITY, AND NATIONAL ELECTRICAL CODES **NEC 2008** TANK TYPE WATER CLOSET VOLUME 2. WALL MOUNT WATER CLOSET VOLUME 3.5 GALLONS 3. WATER - FLOW RATE. PUBLIC FACILITIES 0.5 G.P.M. PRIVATE FACILITIES 2.2 G.P.M. 2.5 G.P.M. SHOWER HEADS VTR LOCATIONS ARE APPROXIMATE AND MAY CHANGE DUE TO JOBSITE AIBD 7059 Blair Road NW Suite 201 Washington DC 2001: 2001 CONDITIONS THE FOLLOWING SHALL COMPLY WITH THE 2010 FBC. □ PORCHES AND BALCONIES SOFTPLAN ☐ HANDRAILS ☐ GUARDRAILS ☐ STAIRS ☐ CHIMNEY & FIREPLACE **—**□ ☐ EGRESS WINDOWS AB 4. ALL OPENINGS SHALL COMPLY WITH 2010 FBC WIND LOADS AS STATED BELOW. ATTACHMENTS OF WINDOWS, DOORS, SLIDING GLASS DOORS AND O.H. GARAGE DOORS ARE DELEGATED THE MANUFACTURER OF THESE ITEMS. THE MANUFACTURER OF THESE ITEMS SHALL SUBMIT ATTACHMENTS TO ENGINEER OF RECORD FOR REVIEW PRIOR TO INSTALLATION. SEE ATTACHED SPECIFICATION SHEETS FOR MANUFACTURERS DESIGN CRITERIA AND INSTALLATION METHODS FOR WINDOWS, DOORS, SLIDING GLASS DOORS, OVERHEAD GARAGE DOORS, AND ROOFING. 5. ALL DOORS INTERIOR & EXTERIOR ARE 8' 0" UNLESS OTHERWISE NOTED ALL SHOWER ENCLOSURES TO BE TEMPERED GLASS 6. ALL WINDOWS WITHIN 24" OF DOORS (INTERIOR & EXTERIOR) AND WITHIN 18" OFF FLR TO BE TEMPERED GLASS. TITLE SHEET COVER SHEET STRUCTURAL ENGINEER NOTES S1 STRUCTURAL ENGINEER NOTES \$2 STRUCTURAL ENGINEER NOTES **S3** WIND LOAD DESIGN DATA **S4** FOUNDATION PLAN **FLOOR PLAN NOTES** DIMENSION PLAN **WINDOW INSTALLATION NOTES: NOTICE TO BUILDER EXTERIOR ELEVATIONS NOTICE TO SUBCONTRACTORS:** 4A **ENTRY TOWER DETAILS** 4B INTERIOR DETAILS IT IS THE INTENT OF THIS DESIGNER THAT **EXTERIOR ELEVATIONS** 5 DUE TO SPACE LIMITATIONS IN THIS 11"X 17" PLAN THESE PLANS ARE ACCURATE AND ARE WINDOWS MUST BE FASTENED INTO STRUCTURAL MEMBERS 6 **ROOF PLAN** FORMAT, AND TO ELIMINATE CLUTTER AND TEXT CLEAR ENOUGH FOR THE LICENSED PROFESSIONAL PER MFG'S. DETAIL REQUIREMENTS PER DESIGN CRITERIA TRUSS PLAN 6A READABILITY ISSUES, SOME DETAILS AND NOTATIONS TO CONSTRUCT THIS PROJECT. NOTED ON THESE DRAWINGS. 6B MAY OR MAY NOT BE LOCATED ON THE SAME SHEETS

IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION..STOP..AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROFESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY REVIEW THESE DOCUMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE BEFORE ANY WORK IS DONE.

OR IN THE SAME LOCATIONS AS PROVIDED FOR BY

THE JOB BEFORE BEGINNING CONSTRUCTION.

IT WOULD BE IN YOUR BEST INTREST TO REVIEW THESE

PLANS AND LOCATE THE APPROPORIATE INFORMATION

REQUIRED TO COMPLETE YOUR SPECIFIC PORTION OF

OTHER CONTRACTORS OR ARCHITECTS.

- WINDOWS ARE NOT IMPACT RESISTANT TYPE. STORM SHUTTERS OR PANELS ARE REQUIRED.
- . ROOF ,WALLS AND WINDOW FASTENINGS MUST BE ENGINEERED AND SPECIFIED FOR CUMULATIVE INTERNAL PRESSURE AND EXTERNAL NEGATIVE (SUCTION) PRESSURES WHICH VARIES ACCORDING TO AREAS AS NOTED IN THE DESIGN CRITERIA AS NOTED ON PAGE \$4.



INDEX OF DRAWINGS

ENRTRY TOWER TRUSS PLAN **ELECTRICAL PLAN** CONSTRUCTION DETAILS CONSTRUCTION DETAILS TYPICAL WALL SECTIONS TYPICAL FOOTING DETAILS

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11

14074

ALECS

IRVICES, INC. (AECS) IS NOT IGN, ITS FEATURES AND RAL INFORMATION IS ACCEPT SLELY FOR THE PURPOSE OF 4, AND FLOOD RESISTANCE

ALLEN ENGINEERING AND CONSTRUCTION SER RESPONSIBLE FOR THE ARCHITECTURAL DESIGNSCIATED DIMENSIONS. THE ARCHITECTURAL AS BEING ACCURATE AND IS USED BY AECS SOID DETERMINING STRENGTH, FIRE PROTECTION, CONSTRUCTION REQUIREMENTS.

WILLOW 291

ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEER P.E. # 5920 C.A. # 9542

INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

SHEET

COVER

1. THE ENGINEERING FIRM FOR THIS STRUCTURAL DESIGN IS ALLEN ENGINEERING AND CONSTRUCTION SERVICES, INC. HEREIN REFERRED TO AS " AECS OR " A.E.C.S ".

2. THE ENGINEER FOR THIS STRUCTURAL DESIGN IS RICHARD E. ALLEN, PE. HEREIN REFERRED TO AS "STRUCTURAL ENGINEER"

ENGINEER".

3. THE STRUCTURAL ENGINEER DESIGN NOTES ARE PART OF THE STRUCTURAL DESIGN AND ARE TO BE TAKEN AS TYPICAL REQUIREMENTS UNLESS NOTED OTHERWISE, "UNO", IN THE STRUCTURAL PLANS AND STRUCTURAL DETAILS.

4. THE DESIGN SHOWN IN THESE PLANS CONFORM TO THE PLORUMENT OF THE CHAPTER 16 OF THE FLORUMENT.

STRUCTURAL PROVISIONS OF THE CHAPTER 16 OF THE FLORIDA BUILDING CODE, SECTION R301 OF THE FLORIDA RESIDENTIAL BUILDING CODE 2010, THE SECTIONS TITLED "STRUCTURAL" OF THE FLORIDA EXISTING BUILDING CODE 2010.

5. THE PURPOSE OF THESE PLANS IS TO OBTAIN A BUILDING PERMIT AND FOR SUBSEQUENT CONSTRUCTION OF THE DESIGN AS SHOWN. THESE PLANS ARE TO BE CONSIDERED VOID IF WORK COMMENCES PRIOR TO A PERMIT BEING ISSUED, A CHANGE IN THE BUILDING CODE OCCURES PRIOR TO THE PLANS BEING SUBMITTED FOR PERMIT OR AFTER SIX MONTHS OF THE DATE THAT THESE PLANS ARE SIGNED AND SEALED WITHOUT BEING SUBMITTED FOR PERMITTING, WHICHEVER OCCURES FIRST, ONCE A BUILDING PERMIT HAS BEEN ISSUED BASED ON THESE PLANS, THE BUILDING DEPARTMENT IS NOT AUTHORIZED TO REISSUE OR TRANSFER BUILDING PERMITS WITHOUT THE EXPRESSED WRITTEN

CONSENT OF THE STRUCTURAL ENGINEER.
6. CONSTRUCTION BASED ON THE STRUCTURAL DESIGN IS TO BE DONE AS SHOWN IN THE PLANS WITHOUT DEVIATION, CHANGE OR OMISSION WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. IF ADDITIONAL DETAIL INFORMATION, OR EXPLANATION IS NEEDED, IT IS TO BE OBTAINED FROM THE STRUCTURAL ENGINEER.
THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ADDITIONAL PARTS OF THESE PLANS, INCLUDING PROVISIONS AS STATED IN ITEM 4.

7. IT IS IMPORTANT TO UNDERSTAND THAT
STRUCTURAL PROVISIONS OF THE BUILDING CODE ARE
COMPLICATED AND THESE PLANS ARE INTENDED TO BE USED BY
AN EXPERIENCED BUILDING CONTRACTOR. PROPERTY OWNERS OBTAINING OWNER-BUILDING CONTRACTOR, PROFERTY OWNER
OBTAINING OWNER-BUILDER PERMITS ARE PROCEEDING AT
THEIR OWN RISK. THE STRUCTURAL ENGINEER IS NOT
RESPONSIBLE FOR ANY ERRORS OR OMISSIONS BY PROPERTY OWNERS OR THEIR AGENTS AS A RESULT OF ANY MISUNDERSTANDING OF THE PLANS THE OTHERWISE WOULD

BE UNDERSTOOD BY A LICENSED CONTRACTOR.

8. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, AND SCHEDULE. 9. THE STRUCTURAL PLANS AND ANY RELEVANT DESIGN DOCUMENTS PRODUCED UNDER THE DIRECT CHARGE OF THE STRUCTURAL ENGINEER ARE THE PROPERTY OF THE STRUCTURAL ENGINEER AND MAY NOR BE USED BY ANY PERSON OTHER THAN THE CONTRACTED CLIENT AND FOR ANY PURPOSE OTHER THAN THAN THAT STATED IN ITEM 5 ABOVE WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.
MOREOVER, NO OTHER ENGINEER OR ARCHITECT IS TO BE DESIGNATED A DELEGATED ENGINEER FOR ANY PURPOSE RELATED TO THESE STRUCTURAL PLANS OR CONSTRUCTION BASED ON THESE PLANS PRIOR TO THE ISSUANCE OF A CERTIFICATE OF COMPLETION OR OCCUPANCY WITHOUT THE

DESIGN CRITERIA

10. LOAD COMBINATIONS: THIS DESIGN IS BASED ON AN "ALLOWABLE -STRESS " FORMULATION RELYING ON THE LOAD COMBINATIONS DEFINED IN FBC 2010 SECTION 1605.3.1 OR SECTION 1605,3.2 WHERE OMEGA EQUALS 1.3 11. FOUNDATION LOADS: SEE NOTES ON " SITE CONDITIONS, SOILS, AND FOUNDATIONS".

EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

12. FLOOR LIVE LOADS: RESIDENTIAL ONE AND TWO STORY FAMILY DWELLINGS: ALL LIVE LOADS PER TABLE R301.5 UNINHABITABLE ATTICS WITHOUT STORAGE: 10 PSF UNINHABITABLE ATTICS WITH STORAGE: 20 PSI HABITABLE ATTICS AND SLEEPING AREAS: 30 PSF BALCONIES: 60 PSF ALL OTHER ROOMS 40 PSF GUARDRAILS /HANDRAILS : 200PSF CONCENTRATED LOAD APPLIED IN ANY DIRECTION.

13. INFORMATION CONTAINED ON A PLAN SHEET WHERE HIS SIGNATURE AND SEAL APPEAR, THAT DOES NOT PERTAIN TO THE RELEVANT STRUCTURAL PROVISIONS AS STATED IN ITEM 4, INCLUDING, BUT NOT LIMITED TO THE BUILDING OCCUPANCY, THE ARCHITECTURAL DESIGN, IT'S FEATURES, FINISHES (I.E., DECORATIVE STUCCO, SIDING, ROOFING, SOFFITS, FLASHING, PAINTING, ETC) AND THEIR INSTALLATION, DIMENSIONS, AND ANY DESIGN OF FIRE PROTECTION, ELECTRICAL, PLUMBING, AND MECHANICAL COMPONENTS OR SYSTEMS.

THE ARCHITECTURAL INFORMATION, INCLUDING DIMENSIONS SHOWN IN THESE PLANS AND PROVIDED TO THE ENGINEER.

SITE CONDITIONS

18. SITE PLAN AND TOPOGRAPHY
A. THE STRUCTURAL ENGINEER IS NOT A SUVEYOR AND IS
NOT RESPONSIBLE FOR THE SITE PLAN, ESTABLISHING REQUIRED SET-BACKS, AND LOCATING THE BUILDING ON THE PROPERTY.
B. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE
GRADING OF THE SITE OR ITS COMPLIANCE WITH ANY DRAINAGE PLAN WHETHER INDIVIDUAL OR AS A PART OF A MASTER DRAINAGE PLAN.

C. THE FOUNDATION DESIGN IS BASED ON THESE PRESUMED CONDITIONS INCLUDING THAT DIFFERENTIAL SETTLING DOES NOT EXCEED THE SAFE LIMITS OF THE FOUNDATION DESIGN (INCLUDING STEMWALLS AND MASONRY ABOVE GRADE WALLS) AS STATED IN ITEM 19 BELOW.

AS STATED IN THEM 19 BELOW.

D. IT IS IMPORTANT TO KNOW THAT THE FOUNDATION DESIGN BASED ON A PRESUMED ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF RELIES ON LESS THAN L/500 (E.G.,0.25 INCHES OVER 10 FEET) OF DIFFERENTIAL SETTLEMENT, CRACKS IN MASONRY WALLS SHOULD BE EXPECTED WHERE DIFFERENTIAL SETTLEMENT EXCEEDS L/150.THIS STATEMENT SHOULD BE TAKEN AS A CAUTIONARY NOTE FOR PROCEEDING WITHOUT A SOILS ANALYSIS AND FOUNDATION RECOMMENDATION BY A GEOTECHNICAL ENGINEER FOR THE SITE.

E. COPIES OF ANY AND ALL REQUIRED COMPACTION TESTS ARE TO BE PROVIDED TO THE BUILDING DEPARTMENT FOR THEIR

STRUCTURAL ELEMENTS
19. FOUNDATION, FOOTING AND GROUND FLOOR SLAB
A. THE FOUNDATION AND FOOTINGS ARE TO BEAR A MINIMUM ON 12 INCHES BELOW GRADE AND ARE TO BE PLACED ON UNDISTURBED SOIL OR FILL COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR PURSUANT TO ASTM D 1557 WITH FILL LIFTS LESS THAN 12".

COMMERCIAL

ALL LIVE LOADS PER FBC 2010 TABLE 1607.1 14. ROOF LIVE LOADS:

ALL ROOF / WOOD CONSTRUCTION TYPES ARE 30 PSF.

15. DEAD LOADS: FLOOR WOOD FRAME: 35 PSF FOR TILE/MARBLE FLOOR COVERING, 15 PSF FOR ALL OTHERS.

ROOF WOOD FRAME: 25 PSF FOR SHINGLES, 35 PSF FOR TILE 16. WIND LOADS:

A. WIND LOADS ARE BASED ON THE SPECIFIC REQUIREMENTS AND DEFINITIONS OF FLORIDA RESIDENTIAL BUILDING CODE 2010 EDITION ASCE-7-10.

B. THE COMPONENT AND CLADDING WIND PRESSURES ARE THE MINIMUM REQUIREMENTS FOR STRENGTH AND IMPACT

PROTECTION NEEDED FOR SELECTING SATISFACTORY COMPONENTS AND CLADDING, BY OTHERS, FOR THE STRUCTURE.

ENGINEERING BY OTHERS IS PRESUMED ACCURATE AND IS RELIED UPON BY THE STRUCTURAL ENGINEER SOLEY FOR THE PURPOSE OF ACHIEVING COMPLIANCE WITH THE RELEVANT STRUCTURE

20. FOOTINGS (AND ANY ASSOCIATED MONOLITHIC FLOOR SLABS) SHALL BE CONSTRUCTED OF CONCRETE WITH A SPECIFIC COMPRESSIVE STRENGTH OF 3,00 PSI, 3 TO 5 INCH SLUMP, AND

A. IN ADDITION, THE STRUCTURAL ENGINEER IS NOT A CIVIL OR GEOTECHNICAL ENGINEER AND IS NOT RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SITE FOR CONSTRUCTION, DETERMINING THE SUITABILITY OF THE SITE FOR CONSTRUCTION, INCLUDING ITS TOPOGRAPHY, DRAINAGE AND SUB-SURFACE CONDITIONS (INCLUDING WATER TABLE DEPTH) AND FOR INTERPRETING GEOTECHNICAL DATA CONCERNING THE SITE. B. IF SOIL CONDITIONS AT THE SITE APPEAR QUESTIONABLE AS DETERMINED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER, A SOILS ANALYSIS SHALL BE PERFORMED BY A LICENSED GEOTECHNICAL ENGINEER THAT WILL GIVE SPECIFIC GEOTECHNICAL ENGINEER THAT WILL GIVE SPECIFIC RECOMMENDATIONS FOR A FOUNDATION TYPE. IF THE BUILDING CONTRACTOR OR OWNER-BUILDER DO NOT MAKE THAT DETERMINATION AND A SOILS ANALYSIS IS NOT PERFORMED THE STRUCTURAL ENGINEER SHALL PROCEED WITH THE DESIGN BASED ON THE PRESUMPTIONS ALLOWED BY THE FBC 2012, SEC. 1804. C. THE DETERMINATIONS OF THE SUITABILITY OF THE SITE FOR CONSTRUCTION (INCLUDING TOPOGRAPHICAL INFORMATION) AND THE SOIL CONDITIONS SHALL HAVE BEEN COMPLETED AND ANY RECOMMENDATIONS RESULTING FROM THAT ANALYSIS SHALL HAVE BEEN PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO HAVE BEEN PROVIDED TO THE STRUCTURAL ENGINEER PROVIDED TO THE SIGNING AND SEALING OF THE STRUCTURAL PLANS.

D. IN THE ABSENCE OF GEOTECHNICAL INFORMATION, THE SITE IS PRESUMED TO HAVE AN ALLOWABLE SOIL BEARING CAPACITY OF 2000 PSF AND THE TOPOGRAPHY AS IT RELATES TO THE STRUCTURE IS PRESUMED TO BE THAT SHOWN IN THE PLANS.

E. THE SIZE AND REQUIRED REINFORCEMENT FOR THE FOOTINGS ARE SHOWN ON THE FOUNDATION PLAN. THE GROUND FLOOR SLAB SHALL BE PLACED OVER A 6 MIL. POLYETHYLENE MOISTURE RETARDER.

I. THE TRUSS SYSTEM DESIGN PROVIDED IN THIS PLAN IS FOR THE USE OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL ROOF TRUSS SYSTEM DESIGN. IT IS NOT TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL DESIGN. IL MANUFACTURED FLOOR TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.

III. THE MANUFACTURED TRUSS DESIGN SHALL INCLUDE SPECIFYING THE TRUSS TO TRUSS AND TRUSS TO GRIDER CONNECTIONS ON EITHER THE INDIVIDUAL TRUSS COMPONENT SHAPETS OF THE CHAPTER THE INDIVIDUAL TRUSS COMPONENT SHEETS OR THE GIRDER TRUSS COMPONENTS SHEETS AS APPLICABLE. A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION THAT A HANGER IS REQUIRED IN THE

IV. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION ON THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED UPON THE FINAL FLOOR TRUSS

F. CONVENTIONAL FRAMED JOISTS WITH A MINIMUM 6 INCH

OVERLAP OF JOINTS. OVERLAY OUTSIAN

G. TERMITE TREATMENT OF THE SITE SHALL BE SPECIFIED BY
THE BUILDING CONTRACTOR OR OWNER-BUILDER.
H. SHRINKAGE CONTROL OF THE FLOOR SLAB SHALL BE ACCOMPLISHED BY 6 INCH BY 6 INCH . W 1.4 BY 1.4 WELDED WIRE FABRIC AS SPECIFIED BY FBC 2010 SECTION 1910.2 EXCEPTION 2 OR FIBERMESH ADMIXTURE AS SPECIFIED BY FBC 2010, SECTION 1910.2 EXCEPTION 1. THE WELDED WIRE FABRIC SHALL BE PLACED BETWEEN THE MIDDLE AND UPPER 1/3 DEPTH OF THE SLAB AND HELD IN POSITION BY APPROPLATE SUPPORTS SPACED NOT GREATER THAN 3 FEET APART. I. CONTRACTION JOINTS ARE TO BE PROVIDED FOR THE PURPOSE OF CONTROLLING SHRINKAGE ONE INCH DEEP CUTS (FOR A FOUR INCH THICK SLAB OR 25 PERCENT OF THE SLAB THICKNESS OTHERWISE) ARE TO BE PROVIDED ACROSS THE WIDTH AND LENGTH OF ANY FLOOR SLAB AT A DISTANCE OF NOT TO EXCEED 30 TIMES THE SLAB THICKNESS. FOR EXAMPLE A FOUR INCH THICK SLAB , CONTRACTION JOINTS SHALL NOT EXCEED 10 FEET ON CENTER BACH WAY THE CONTRACTION JOINTS ARE OPTIONAL FOR ONE AND TWO STORY FAMILY RESIDENTIAL WHEN WELDED WIRE FABRIC OR FIBERMESH ARE USED IN THE FLOOR SLAB.

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ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEER P.E. # 56920 C.A. # 9542

INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

DEEB FAMILY HOMES, LTD. HOMES,

- A COMPOSITE COLUMN HERE IS DEFINED AS A HOLLOW COLUMN CONSISTING OF ANY MATERIAL SPECIFICALLY DESIGNED BY ITS MANUFACTURER TO BE LOAD BEARING. ANY OTHER TYPE OF HOLLOW COLUMN IS CONSIDERED AN ARCHITECTURAL FINISH INTENDED TO FIT OVER A STRUCTURAL COLUMN AND ITS USE AND DETAILS OF INSTALLATION ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER
- STRUCTURAL ENGINEER.

 IL LOAD BEARING COMPOSITE COLUMNS ARE A MANUFACTURED PRODUCT SUBJECT TO THE DESIGN AND LOAD BEARING CAPACITY AS DETERMINED BY THE MANUFACTURER. A SHOP DRAWING OR A LETTER FOR THE INSTALLATION OF THE COLUMN SHALL BE PROVIDED BY THE STRUCTURAL ENGINEER TO SUPPLEMENT THE CONSTRUCTION PLANS AFTER THE SPECIFIC COLUMN AND MANUFACTURER HAVE BEEN IDENTIFIED.

 III.IN ALL CASES, THE COLUMN MANUFACTURES INFORMATION SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT FOR REVIEW PRIOR TO ITS ACCEPTANCE FOR THE STRUCTURAL DESIGN. THE INFORMATION SHALL INCLUDE THE LATERAL AS WELL AS UPLIFT AND GRAVITY LOAD BEARING CAPACITIES.
- AND GRAVITY LOAD BEARING CAPACITIES.
- D., STEEL TUBE COLUMNS:
- D. STEEL TUBE COLUMNS:

 1. LOAD BEARING STEEL TUBE COLUMNS SHALL HAVE A MINIMUM WALL
 THICKNESS OF 1/4 INCH AND BE MADE OF STEEL WITH A DESIGN YIELD
 STRENGTH OF 46 PSI UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN
 IL THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL
- DESIGN WHERE THE STEEL TUBE COLUMN IS TO BE INSTALLED.
- E. ALUMINUM COLUMNS:
- I. LOAD BEARING ALUMINUM COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS
- II. ALL FASTENERS AND CONNECTORS FOR ALUMINUM COLUMNS SHALL BE STAINLESS STEEL OR MONEL TO AVOID CORROSION DUE TO DISSIMILAR METALS BEING IN CONTACT.
- III. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE ALUMINUM COLUMN IS TO BE INSTALLED.

- A. MANUFACTURED WOOD TRUSSES

 I. THE MANUFACTURED ROOF TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL SYSTEM DESIGN. IT IS NOT INTENDED TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL
- II. MANUFACTURED ROOF TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS TRUBE OF THE TRUE OF THE TRUE OF T PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY
- SUBORDINATED TO THE BUILDING CONTRACTOR.

 III. THE TRUSS PLAN " SIGNED AND SEALED" BY THE DELEGATED ENGINEER SHALL

 BE PROVIDED TO AND PRIOR TO CONSTRUCTION OF THE UNDERLYING STRUCTURE

 AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL

 CHANGES BASED ON THE ENAL ELOOD TRUCK OVERSAL
- CHANGES BASED ON THE FINAL FLOOR TRUSS SYSTEM.

 VI. THE TRUSS MANUFACTURER SHALL PROVIDE ALL LATERAL BRACING
 REQUIREMENTS TO THE BUILDING CONTRACTOR. IF NOT, THE BUILDING CONTRACTOR IS TO NOTIFY THE STRUCTURAL ENGINEER FOR GUIDANCE.
- V. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH TRUSS IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 10d COMMON NAILS (TOB-NAILED) VI. A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND
- CONCRETE / MASONRY
- 23.2 CONVENTIONAL FRAME
- I. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH RAFTER IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 3- 10d COMMON NAILS (TOE-NAILED)

 II. ANY WOOD COMING IN CONTACT WITH MASONRY OR CONCRETE IS TO BE DESCRIPTION OF A MOSTURE BARDLER IS TO BE DISTANTED.
- PRESSURE TREATED OR A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE OR MASONRY.

- III. COLLAR TIES ARE TO BE INSTALLED BETWEEN RAFTERS AT 2/3 OF THE RIDGE HEIGHT FROM WHERE THE RAFTERS BEAR ON WALLS. THE COLLAR TIES ARE TO BE FASTENED WITH A MINIMUM OF 4-10d 16 COMMON NAILS (CLINCHED) AT EACH LAP JOINT. EACH RAFTER IS TO BE ATTACHED TO THE RIDGE BEAM WITH A LIGHT ANGLE HANGER AS SHOWN IN THE FRAMING PLAN. IN ADDITION, A FLAT METAL STRAP SHALL BE INSTALLED ACROSS THE RIDGE BEAM TO TWO OPPOSING
 RAFTER. TO BE REVIEWED BY THE STRUCTURAL ENGINEER FOR
 COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN
 TO FOR ANY CHANGE TO THE THEORY OF THE UNIDERLYING. AND FOR ANY CHANGES TO THE "TRUSS TO THE UNDERLYING STRUCTURE" CONNECTIONS.
- IV. AS PART OF THE REVIEW, THE STRUCTURAL ENGINEER WILL
 DETERMINE WHETHER THE TRUSS TO WALL / BEAM METAL
 CONNECTORS SHOWN IN THE ORIGINAL PLANS ARE ACCEPTABLE
 OR WHETHER THEY NEED TO BE CHANGED OR SUPPLEMENTED TO ACCOMMODATE THE LOADS SHOWN IN THE TRUSS COMPONENT
- V. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR VERIFYING THE DIMENSIONAL, ARCHITECTURAL, OR FORM ASPECTS OF THE OF THE TRUSS MANUFACTURERS PLAN WITH THE ORGINAL PLANS.
- VI. THE MINIMUM LIVE LOADS FOR THE ROOF TRUSS DESIGN IS TO BE ON FBC 2010 SECTION 1607 FOR ROOF TYPE AND ROOFING MATERIAL.
- VII. THE DEAD LOADS ARE LASTED IN ITEM 16 ABOVE.
 VIII. ALL TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTORS ARE TO BE SPECIFIED BY THE TRUSS AND TRUSS TO GIRDER CONNECTORS ARE TO BE SPECIFIED BY THE TRUSS MANUFACTURER, INCLUDING CONNECTORS FOR TRUSS TO MANUFACTURED BEAM (LE. GLUELAM, OR MICROI AM) SPECIFIED BY THE TRUSS MANUFACTURER. A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION, A HANGER IS PROTURBED IN THE TRUSS SYSTEM.
- SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION A HANGER IS REQUIRED IN THE TRUSS SYSTEM.

 IX. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORGINALPLAN AND FOR ANY CHANGES TO THE TRUSS TO LINDED LYING STRUCTURES CONNECTIONS. THE BITAM "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER.
- X. A RIDGE BEAM TERMINATING AT A GABLE END SHALL BE SUPPORTED BY A MINIMUM 3 STUD PACK COLUMN BEARING ON THE UNDERLYING WALL OR BEAM.
- XI. TREATED LUMBER-DOUBLE 1 1/2 INCH BY A HEIGHT SHOWN ON THE PLANS. FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL
- PLANS. FOR CONCRETE OR MASONKY WALLS THE FASTENERS SHALF
 BE 5/8 INCH BY 5 1/2 INCH SIMPSON TITEN HD CONCRETE BOLTS.
 XII. SLEEPERS SHALL BE FASTENED TO UNDERLYING ROOF TRUSSES
 OR RAFTERS (NOT SHEATHING) WITH A MINIMUM OF 2-3/8 INCH BY
 3 1/2 INCH LAG BOLTS AND WASHERS AT EACH TRUSS OR RAFTER
 INTERSECTION AND NO GREATER THAN 24 INCHES ON CENTER
 AND SHALL CONSIST OF DIMENSIONAL LUMBER 1 1/2 INCH THICK BY A WIDTH SHOWN IN THE PLANS.
- XIII. USE 2 INCH BY 4 INCH BLOCKING ATTACHED BETWEEN UNDERLYING STUDS, TRUSSES OR RAFTERS WITH A MINIMUM OF 3-10d NAILS AT EACH IN ORDER TO SATISFY THE ON CENTER SPACING FOR THE LEDGERS OR SLEEPERS.
- BLAMS:
 XIV BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS ARE TO BE ATTACHED
 AS SPECIFIED IN THE ROOF FRAMING PLANS.
 24. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN
 THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES
 THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.
 LEDGERS/ SLEEPERS
- I. LEDGERS / NAILERS SHALL BE FASTENED TO WOOD STUDS (NOT SHEATHING)
 WITH A MINIMUM OF 2-3/8 INCH BY 5-1/2 INCH LAG BOLTS WITH WASHERS
 AT EACH STUD INTERSECTION AND NO GREATER THAN 16 INCHES ON CENTER
- AND SHALL CONSIST ON PRESSURE TREATED WOOD.

 II. MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (I.E. GLUELAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS SPECIFICATIONS.

- III. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED
- I. FOR TWO PLY BEAMS ONE ROW OF 10d GALVANIZED COMMON NAILS AT 6 INCHES ON CENTER ON EACH SIDE OF BEAM.
- II. FOR THREE PLY BEAMS- TWO ROWS OF 16d GALVANIZED COMMON NAILS AT 6" ON CENTER (TOP AND BOTTOM) THRU EACH SIDE OF THE BEAM
- DILFOR FOUR PLY BEAMS AND LARGER- TWO ROWS OF 1/2 INCH DIAMETER CARRIAGE BOLTS OR ALL THREAD RODS WITH NUTS AND WASHERS SPACED AT 12" ON CENTER 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM.
- B. SHEATHING:
- B. SHEATHING:

 I. ROOF SHEATHING COVERED BY COMPOSITE ROOFING SHALL
 BE A MINIMUM OF 15/32 INCH THICK (NOMINAL) O.S.B.
 MANUFACTURED WITH EXTERIOR GLUE.

 II. ROOF SHEATHING COVERED BY TILE SHALL BE A MINIMUM
- OF 5/8 INCH THICK (NOMINAL) MANUFACTURED WITH EXTERIOR
- III. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE ROOF TRUSS SYSTEM. IV. FASTENING SHALL BE 8d RING SHANK NAILS AT 4 INCHES ON
- CENTER AT BOUNDARY AND EDGES AND 6 INCHES ON CENTER IN THE FIELD WITH A SETBACK OF 5 '-0" FROM ALL EDGES.
- V. METAL "H" CLIPS OR SOLID WOOD BLOCKING SHALL BE USED AT ALL UNSUPPORTED BUTT JOINTS BETWEEN TRUSSES OR RAFTERS.
- 25. PRECAST CONCRETE LINTELS
- A. PRECAST AND PRESTRESSED CONCRETE LINTELS SHALL BE MANUFACTURED BY CASTCRETE AND INSTALLED PER MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS.
- B. THE SIZE OF THE LINTELS SHALL BE BASED ON THE SPAN AND LOAD. REFER TO THE ATTACHED SCHEDULE UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN FOR THE SPECIFIED LINTEL
- C. LINTEL SCHEDULE U.N.O. ON PLANS:
- L SPAN UP TO 3'- 8F8-0B II. SPAN UP TO 3' TO < 6' - 8F8-OB
- III. SPAN 6' TO > 14' 8F16- 1B/1T
 D. THE MINIMUM SPECIFIED GROUT COMPRESSIVE STRENGTH TO BE USED FOR LINTELS IS 3,000 PSI.

- E. THE REINFORCING STEEL SHALL BE ASTM GRADE 60
 26. FASTENERS (METAL CONNECTORS.
 A. ALL FASTENERS AND METAL CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE AND INSTALLED PER THE MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS.
- B. THESE FASTENERS DO NOT INCLUDE TYPICAL NAILS AND SCREWS WHICH MAY BE MANUFACTURED BY OTHERS.
- C. FOLLOW ALL MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS FOR ALL FASTENERS, METAL CONNECTIONS, SCREWS, NAILS, ETC. THAT ARE IN CONTACT WITH PRESSURE TREATED LUMBER.
- 27. DIMENSIONAL LUMBER:
- A. ALL LOAD BEARING WALLS SHALL BE SOUTHERN YELLOW PINE #2 OR BETTER GRADED AND STAMPED BY THE CERTIFYING AGENCY. IN ADDITION, ALL WOOD SHALL BE PRESSURE TREATED FOR EXTERIOR USE WHERE EXPOSED TO MOISTURE, PLACED WITHIN 12 INCHES OF SOME BLOOD TAKEN THE CONCEPTE ON MASCAINED. SOIL OR IN CONTACT WITH CONCRETE OR MASONRY.
- 28. STRUCTURAL SHEATHING:
- A. ALL SHEATHING USED FOR EXTERIOR APPLICATIONS SHALL BE EXTERIOR GRADE AND ADA STAMPED AND VERIFYING ITS RATING.
- A. CONCRETE MASONRY UNITS SHALL CONFORM WITH AMERICAN MASONRY INSTITUTE STANDARD 530
- B. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI
- C. MORTAR SHALL BE OF TYPE M OR S GRAY MORTAR.
- A. ALL GROUT SHALL BE A FINE TYPE HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI UNLESS SPECIFICALLY SHOWN OTHERWISE BY A MANUFACTURER PURSUANT TO GROUT USE WITH ITS PRODUCTS.
- A. ALL REINFORCING STEEL SHALL BE ASTM GRADE 40 EXCEPT GRADE 60 SHALL BE USED FOR GRADE BEAMS, ALL LINTEL TYPES (I.E. PRECAST AND FIELD PREFORMED) COLUMNS UNLESS OTHERWISE SHOWN IN THE STRUCTURAL PLANS.

WILLOW 291

ALLEN ENGINEERING & CONSTRUCTION SERVICE

RICH ALLEN PROFESSIONAL I P.E. # 56920 C.A. # 9542

AECS

INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

PLAN DATE

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

ENGINEER

32. STRUCTURAL STEEL AND CONNECTION ACCESSORY MATERIAL:
A. I-BEAMS, FORMED STRUCTURAL STEEL, FLAT BAR OR PLATE
SHALL BE ASTM GRADE A36 UNLESS STATED OTHERWISE.
B. ALL STRUCTURAL STEEL SHALL HAVE A MINIMUM OF TWO
COATS OF PRIMER AND TWO COATS OF EPOXY AS A
CORROSION PREVENTIVE. THE BUILDING CONTRACTOR MAY
VARY FROM THIS SPECIFICATION WITH THE APPROVAL OF THE
STRUCTURAL ENGINEER IF IT CAN BE DEMONSTRATED ANOTHER
MEANS OF CORROSION CONTROL IS EQUALLY EFFECTIVE.
C. ALL WELDING OF STRUCTURAL STEEL SHALL BE MADE WITH

C. ALL WELDING OF STRUCTURAL STEEL SHALL BE MADE WITH E60/70 TYPE ELECTRODES. THE DEPTH AND LENGTH FOR THE WELD SHALL BE SPECIFIED IN THE STRUCTURAL DESIGN FOR THE SPECIFIC CONNECTION.

33. VENTILATION:

A. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR DETERMINING VENTILATION REQUIREMENTS OF CRAWL SPACES, FLOORS AND ATTICS NOR THE MEANS AND METHODS FOR IMPLEMENTING THESE REQUIREMENTS.

A. ANY RENDERING OF NOTES OF WATERPROOFING MEASURES FOR BASEMENTS OR HALF BASEMENTS SHOWN IN THESE PLANS WHERE A SPECIFIC CONSTRUCTION DETAIL IS NOT SHOWN IN THE STRUCTURAL DESIGN IS AN ADDITION OF THE WAY WATER TO SHOW DESIGN IS AN ARCHITECTURAL ILLUSTRATION ONLY AND IS NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

B. CRICKETS ARE ASSOCIATED WITH THE ARCHITECTURAL FINISHES AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

35. FIRE RESISTANT DESIGN:

A. FIRE RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON UNDERWRITERS LABORATORY OR GYPSUM ASSOCIATION DESIGN FOR FIRE RATED FLOOR, WALL AND ROOF ASSEMBLIES.

36. FLOOD RESISTANT DESIGN:

- 36. FLOOD RESISTANT DESIGN:
 A. FLOOD RESISTANT DESIGN OF FLOOD RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DEIGN AND SHALL BE BASED ON THE REQUIREMENTS STATED IN TITLE 44 CFR SECTIONS 59 AND 60, AND ON THOSE OF THE INDIVIDUAL COMMUNITY RATING AGENCIES FOR THE GOVERNMENTAL JURISDICTION WHERE THE CONSTRUCTION IS TO BE DONE
- CONSTRUCTION IS TO BE DONE.

 B. HOWEVER, THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR IDENTIFYING AND SHOWING ON THE PLANS THE FLOOD ZONE CATEGORY, BASE FLOOD ELEVATION, AND THE FLOOR AND STORY HEIGHTS OF THE BUILDING IN RELATION TO THE BASE FLOOD ELEVATION. THIS INFORMATION IS CONSIDERED ARCHITECTURAL AND SITE RELATED AND SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT.

37. SPECIAL CONSTRUCTION:
I. ALUMINUM STRUCTURAL COLUMNS.

- I. ALUMINUM STRUCTURAL COLUMNS.

 A. ANY ALUMINUM STRUCTURES SHOWN IN THESE PLANS SUCH AS PORCH AND POOL ENCLOSURES OR GUARDRAILS AND HANDRAILS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL
- B. WHERE THE ALUMINUM STRUCTURE ATTACHES TO THE MAIN STRUCTURE OR IS INCORPORATED IN THE MAIN STRUCTURE, SHOP DRAWINGS FOR THESE STRUCTURES SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER TO DETERMINE THEIR EFFECT ON THE MAIN STRUCTURE.

II. SWIMMING POOLS:

II. SWIMMING POOLS:

A. ANY SWIMMING POOL OR HOT TUBS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL DESIGN.

III. FENCES AND RETAINING WALLS:
A. ANY RENDERING OF FENCES, RETAINING WALLS OR EXTERIOR PLANTERS
WHERE A SPECIFIC STRUCTURAL DETAIL IS NOT SHOWN FOR THEIR CONSTRUCTION ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

IV, DRIVEWAYS AND WALKWAYS:

A. ANY DRIVEWAYS OR WALKWAYS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION PURPOSES ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

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22.6 psf max.,	-30.2 psf mir				
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da Building Code.					
l Information					
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X					
Flood Design Data					

Door and Window Openings							
No.	Description	Opening Width	Opening Height	Disiance from Corner	Design Pressure Requirements		
1	Entry Door - example	3	8	12	21.4 psf max., -23.3 psf min.		
2	Sliding Glass Door - example	- 6	6.67	6	20.7 psf max., -23.2 psf min.		
3	Fixed Glass Window - examp	2	2	3	22.6 psf max., -30.2 psf min.		
4	10 sqft zone 4	3	3.3	10	22.6 psf max., -24.5 psf min.		
4	20 sqft zone 4	4	5	10	21.6 psf max.,23.5 psf min.		
4	30 sqft zone 4	5	_6	10	21.1 psfmax., -23.0 psfmin		
4	40 sqfl zone 4	5	В	10	20.7 psf.max22.6 psf.min.		
4	50 sqft zone 4	5	10	10	20.2 psfmax, -22.1 psfmin.		
4	100 sqft zone 4	10	10	10	19.2 psf max., -21.2 psf min.		
5	10 sqfl zone 5	4	2.5	2	22.6 թչք max., -30.2 թչք min.		
5	20 soft zone 5	4	5	2	21.6 psf max., -28.2 psf min.		
	30 soft zone 5	4	7.5	2	21.1 psf max., -27.3 psf min.		
5	40 sqft zone 5	4	10	2	20.7 psf max., -26.4 psf min.		
3	50 soft zone 5	4	12.5	2	20.2 psf max., -25.5 psf min.		
	100 sqfl zone 5	4	25	2 -	19.2 psf max., -23.5 psf min.		
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WILLOW 291

ALLEN ENGINEERING & CONSTRUCTION SERVICES

RICH ALLEN PROFESSIONAL EN P.E. # 56920 C.A. # 9542

AECS 14074

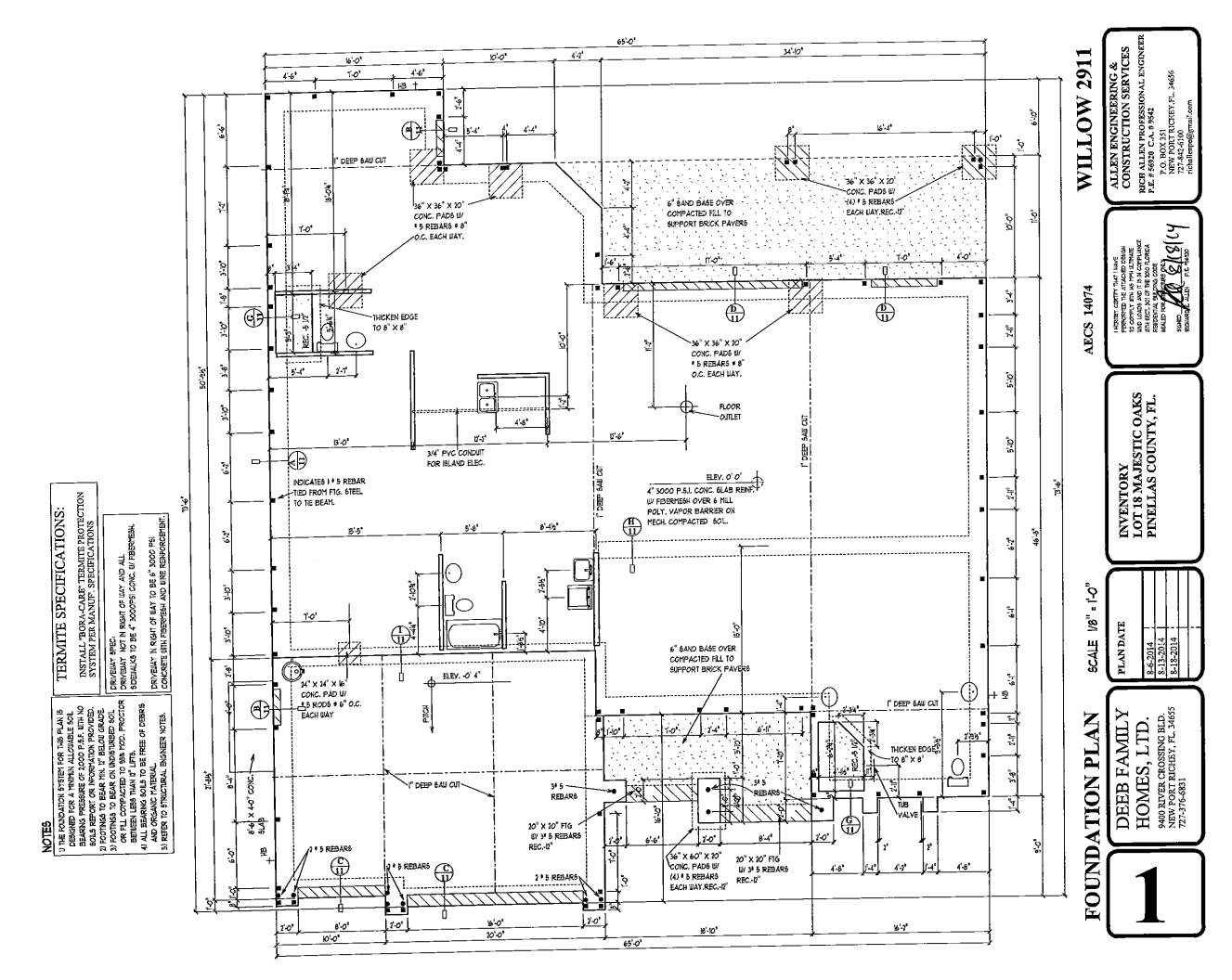
INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

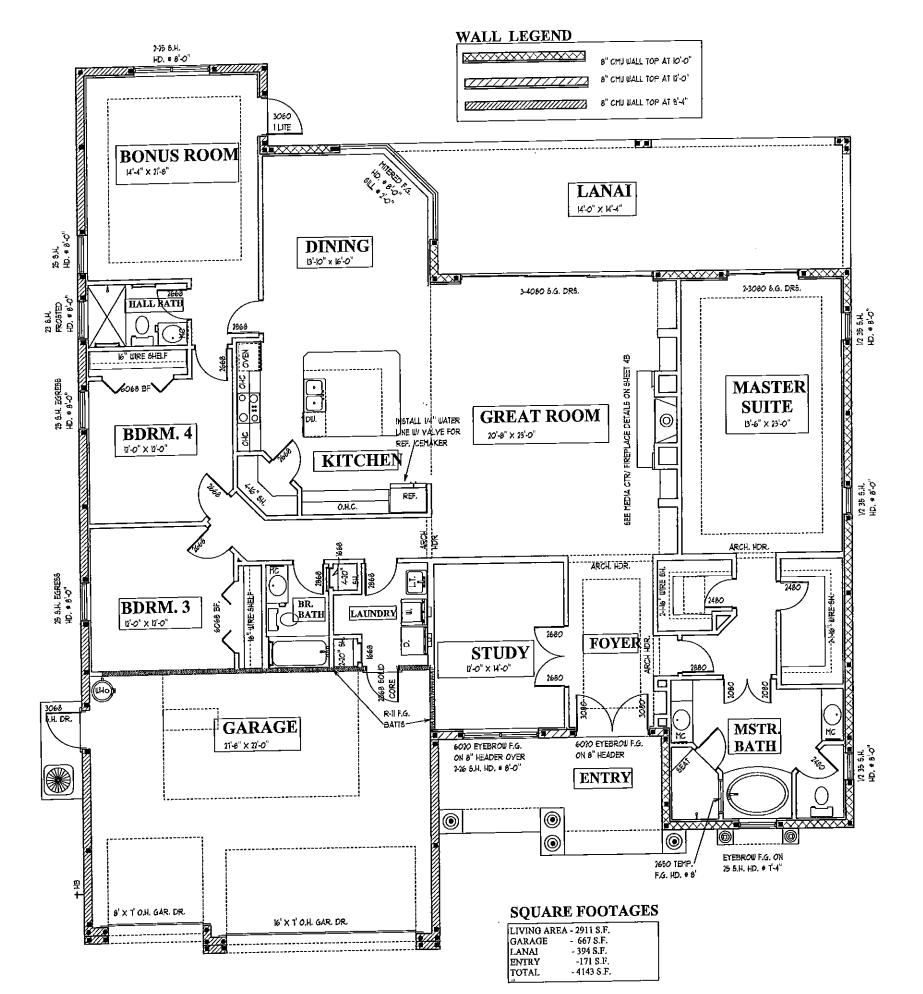
PLAN DATE

DEEB FAMILY HOMES, LTD.

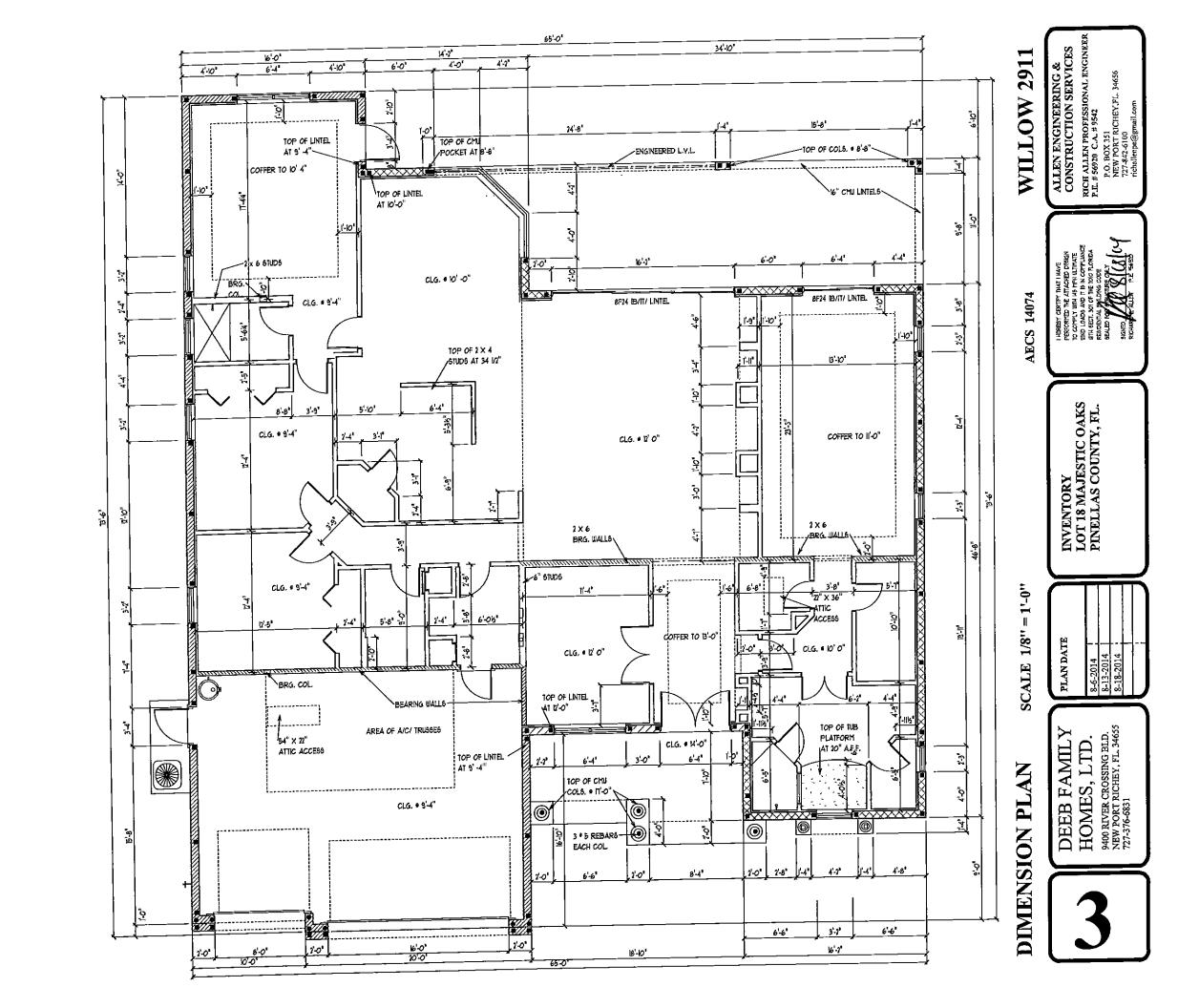
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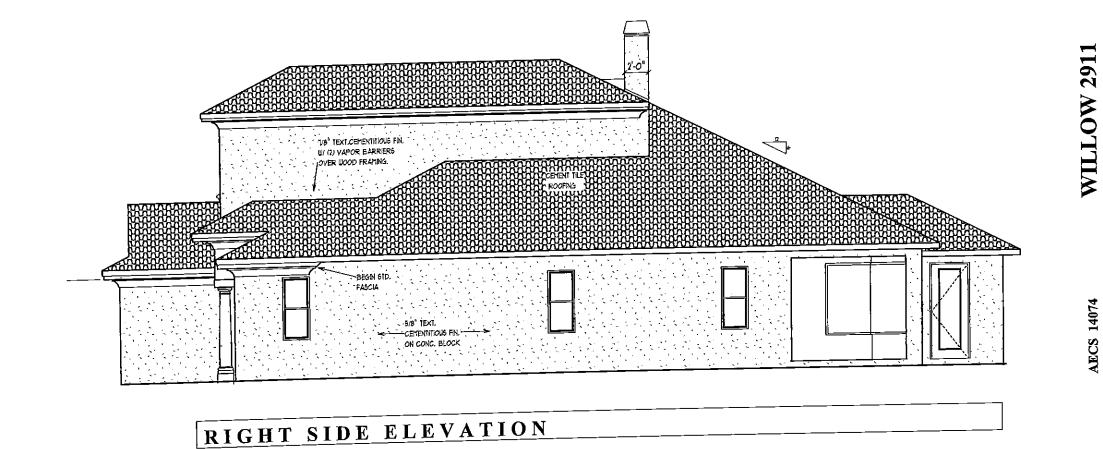
OAD DESIGN

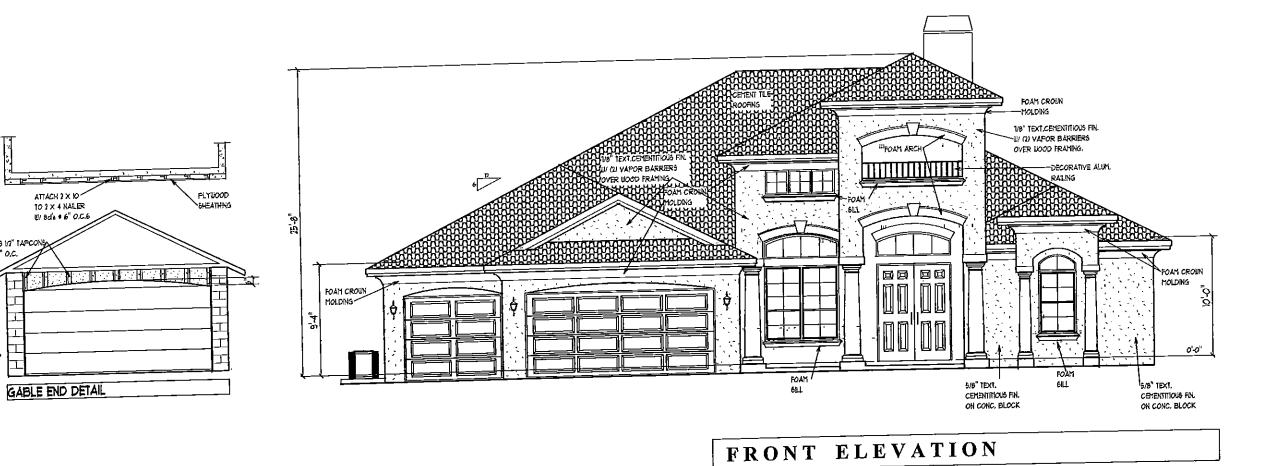




WILLOW 2911 AECS 14074 INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL. ō <u>-</u>8 DEEB FAMILY HOMES, LTD. 9400 RIVER, CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831 FLOOR FIRST







1/4" x 3 1/3" TAPCOT

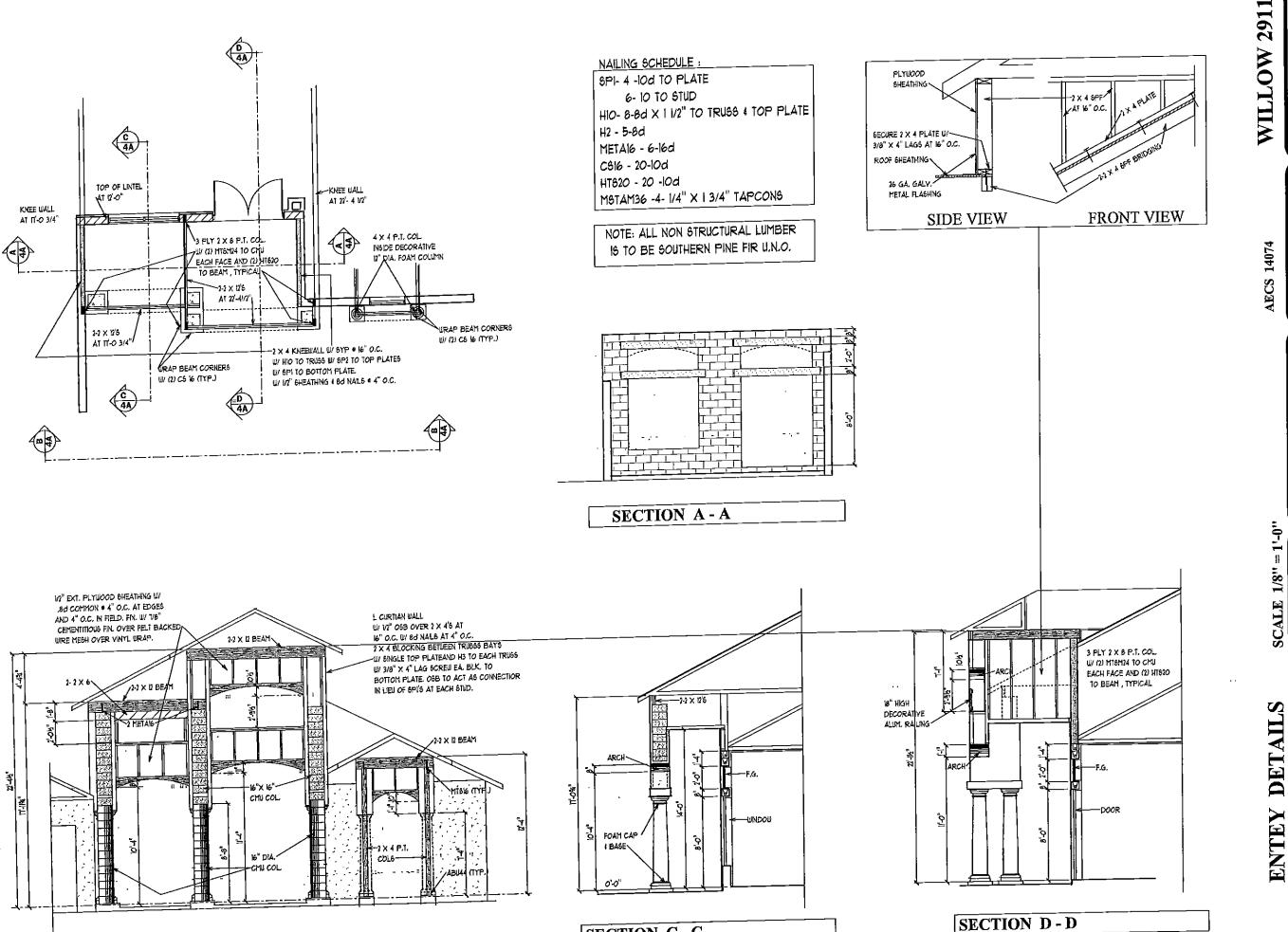


HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEER P.E.# 56920 C.A.# 9542

PLAN DATE

INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.



SECTION C-C

ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEER P.E. # 56910 C.A. # 9542 P.O. BOX 351 NEW PORTRICHEY FL. 34656

INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

PLAN DATE

DEEB FAMILY HOMES, LTD.

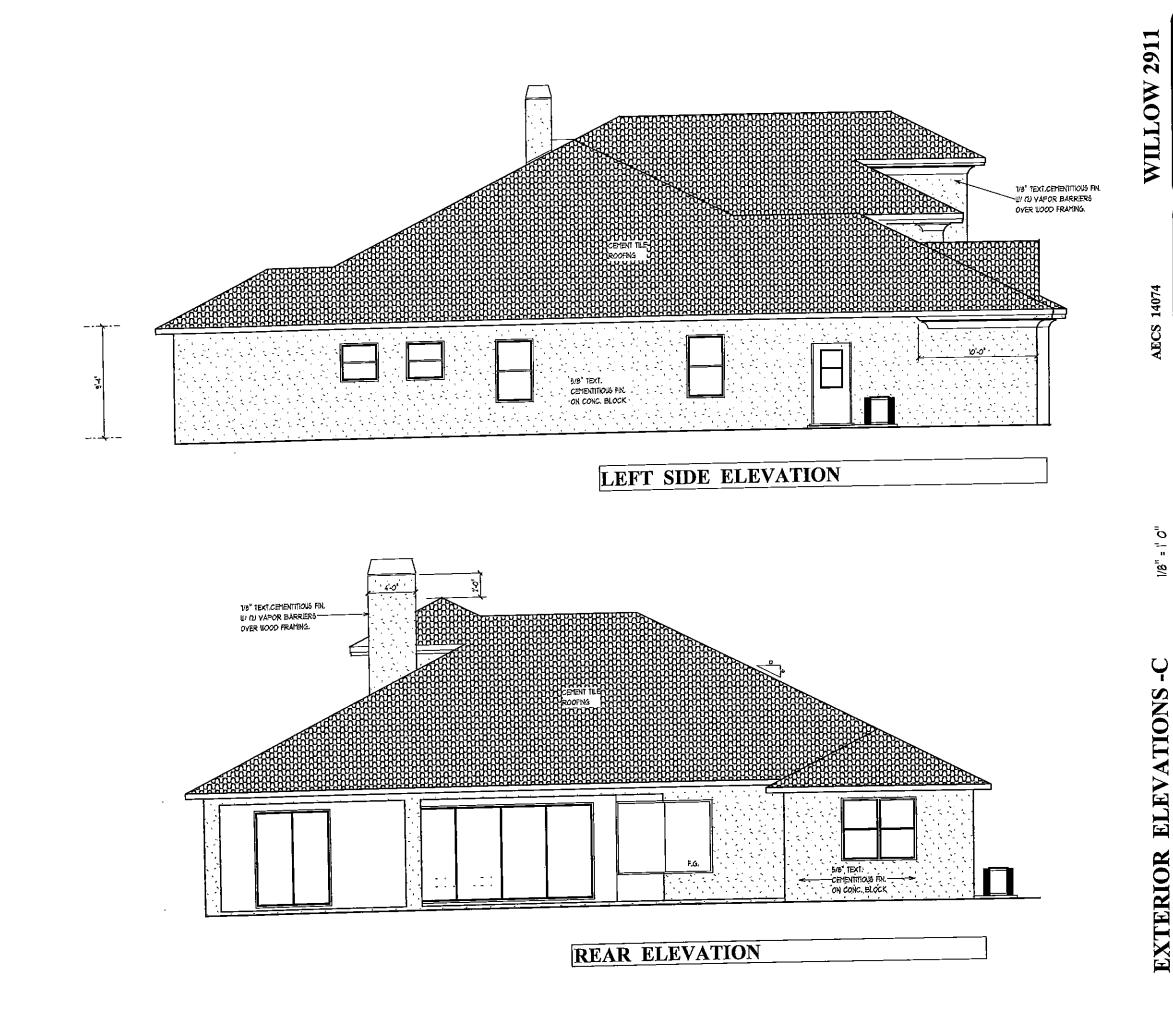




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9400 RIVEN CROSSING BLD.
NEW PORT RICHEY, FL. 34655
727-376-6831

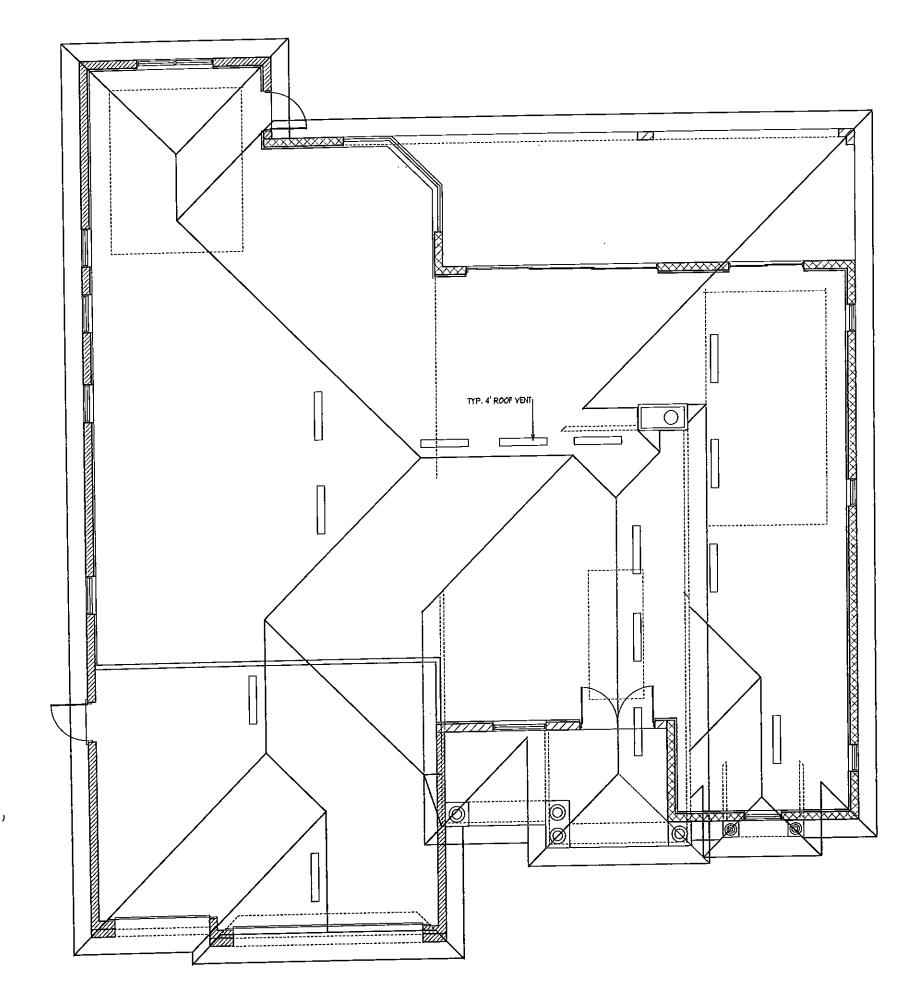
INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

4B



INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

DEEB FAMILY HOMES, LTD.



2911

WILLOW

AECS 14074

SCALE 1/8"

PLAN

ROOF

INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

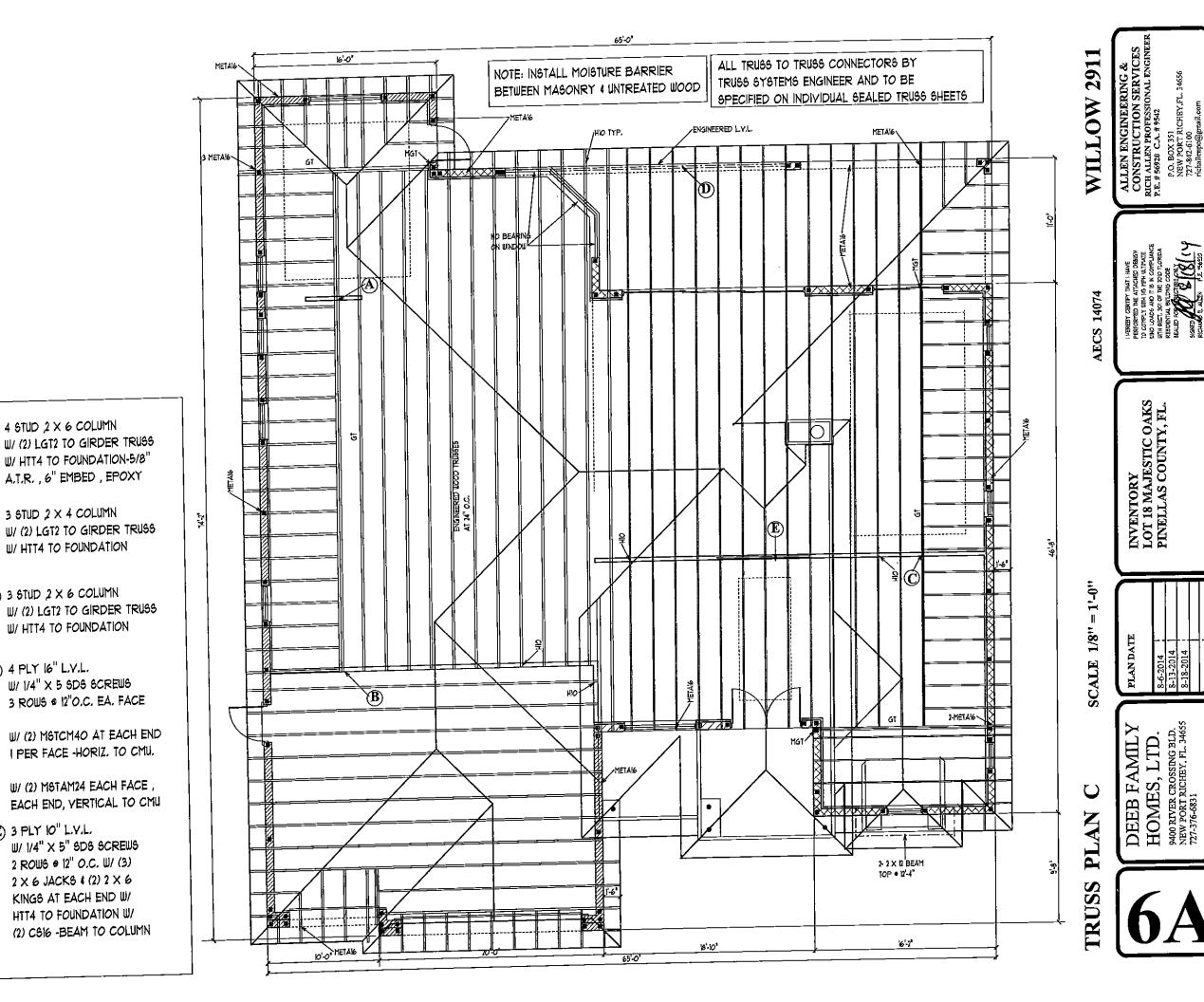
DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

TOTAL NET FREE VENTILATING AREA SHALL NOT BE
LESS THAN 1 TO 300 PROVIDED THAT AT LEAST 50 %
AND NOT MORE THAN 80 % IS PROVIDED BY VENTILATORS
LOCATED IN THE UPPER PORTION OF THE SPACE TO BE
VENTILATED PER SECT. R806.2

TOTAL AREA TO BE VENTILATED = 3980 S.F. 3980/300 = 13.27 S.F. OR 1911 SQUARE INCHES.

ROOF VENTS ARE RATED AT 36 BOWARE INCHES OF OPENING PER LINEAL FT. INII 5.1/36 6.1 • 53.1 LINEAL FEET REQUIRED,

INSTALLATION FOR THIS ROOF IS 56 LINEAL FEET OF ROOF VENTS (14- 4'-0" VENTS)



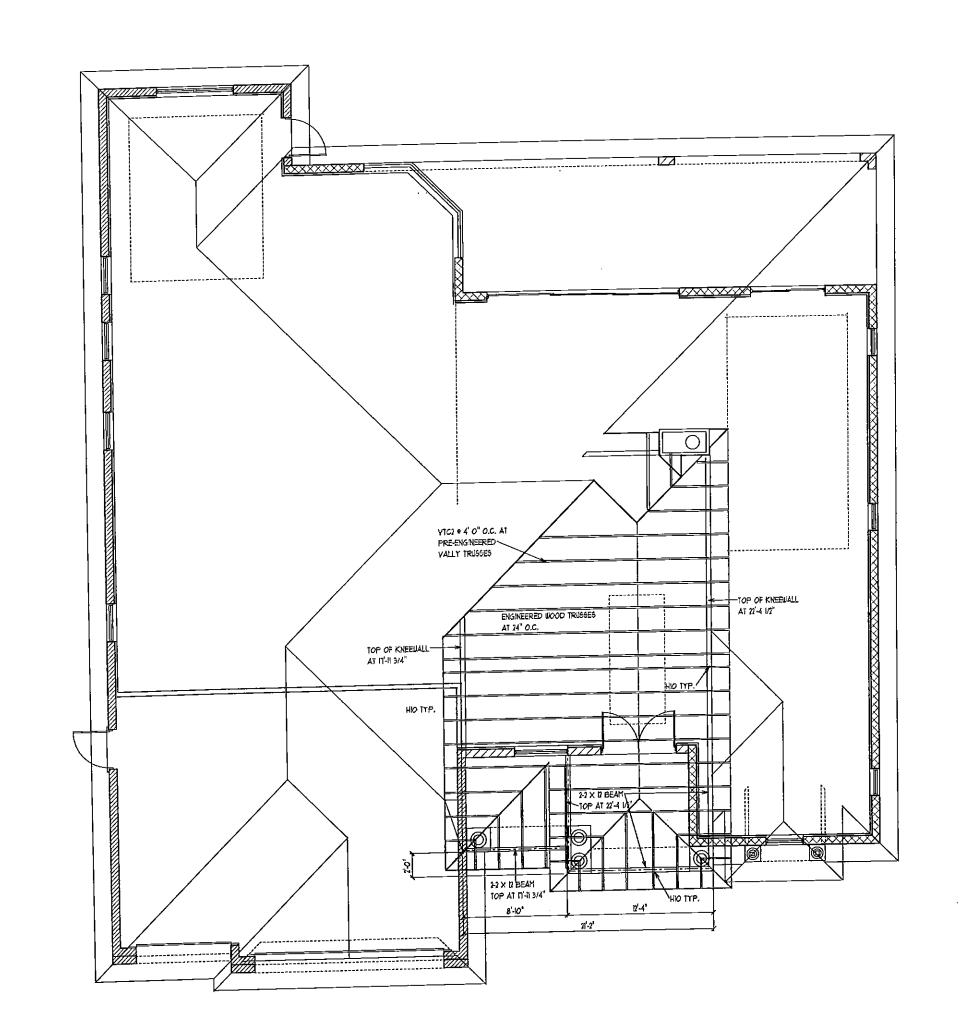
(A) 4 STUD 2 X 6 COLUMN

(B) 3 STUD 2 X 4 COLUMN

(C) 3 STUD 2 X 6 COLUMN

(D) 4 PLY 16" L.Y.L.

(E) 3 PLY 10" L.Y.L.



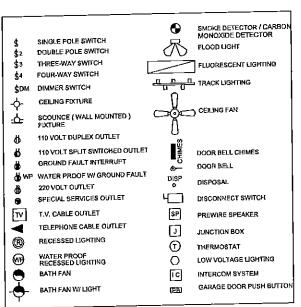
 $= 1^{1} 0^{11}$ 1/8" PLAN DATE SCALE TRUSS PLAN-C HOMES, LTD.
9400 RIVER CROSSING BLD.
NEW PORT RICHEY, FL. 34655
727-376-6831 ENTRY

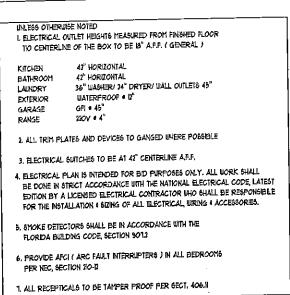
INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

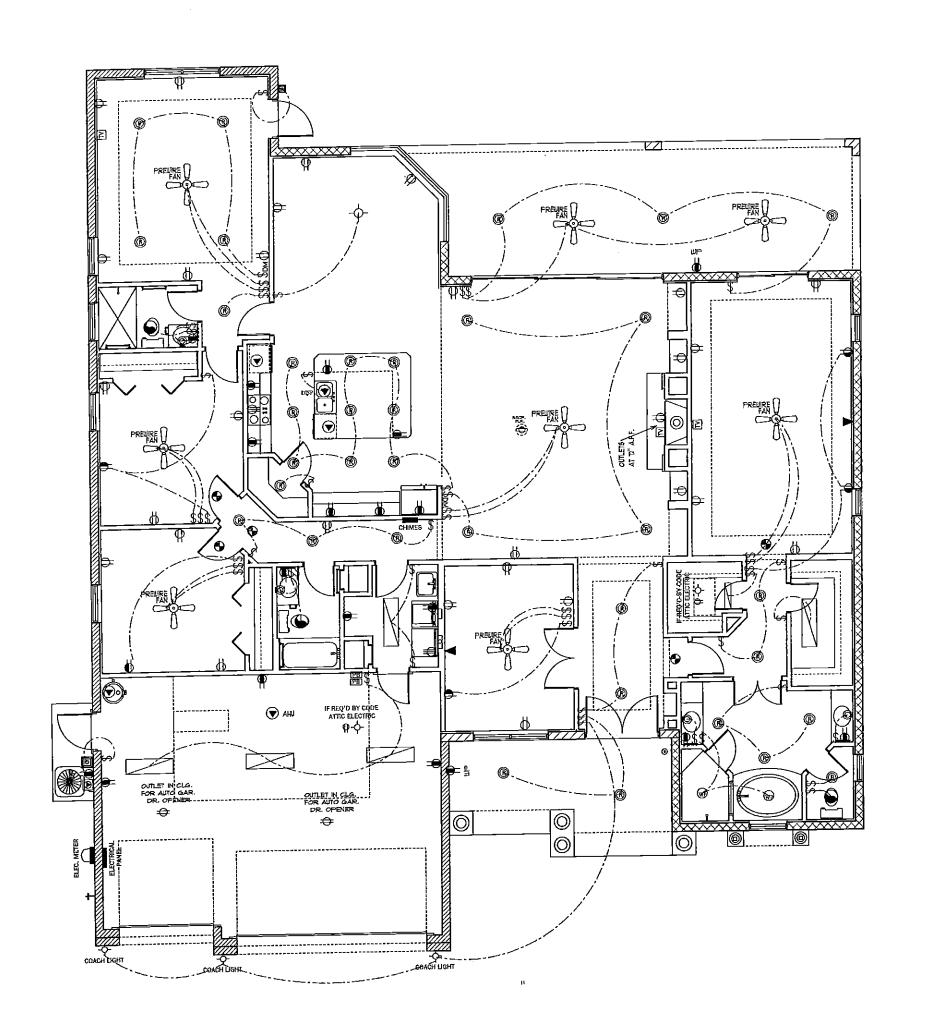
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WILLOW 2911

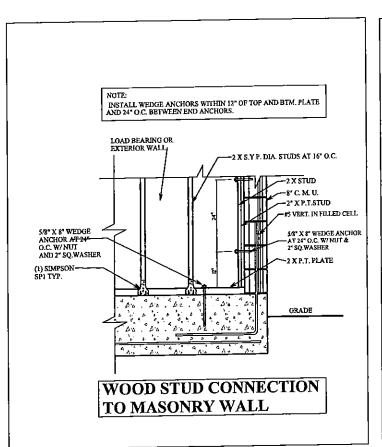
ALLEN ENGINEERING & CONSTRUCTION SERVICES RICHALLEN PROFESSIONAL ENGINEER P.E. # 56920 C.A. # 9542 P.E. # 56920 C.A. # 9542 P.E. # 56920 C.A. # 9542 P.E. # 3442-6100 richallenpe@gmail.com

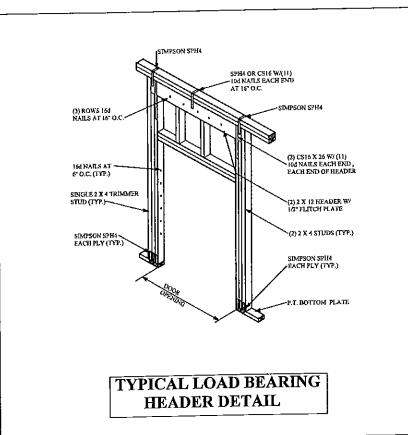


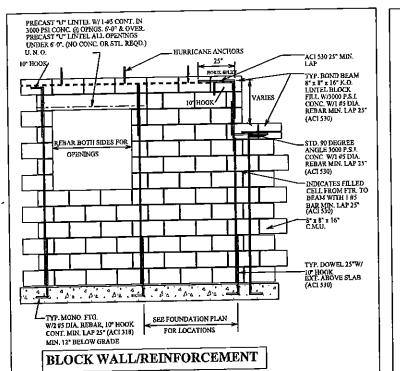


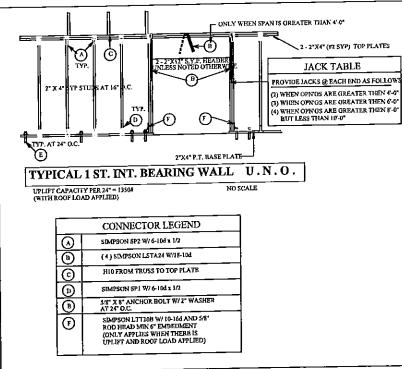


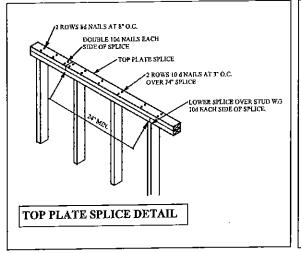
291 WILLOW **AECS** INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL. Ō <u>'8/</u> PLAN DATE DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831 TRICAL PLAN ELEC

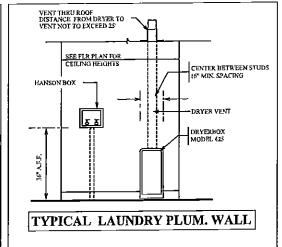


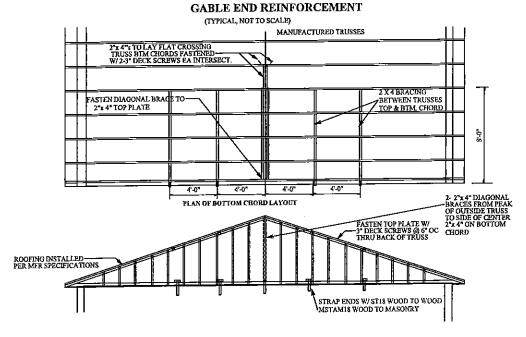


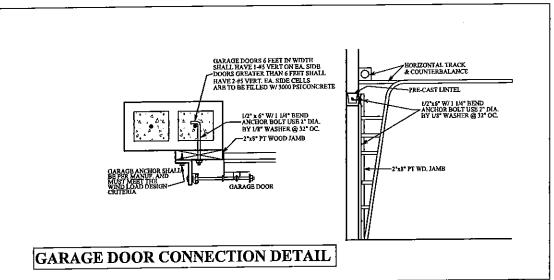


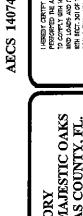












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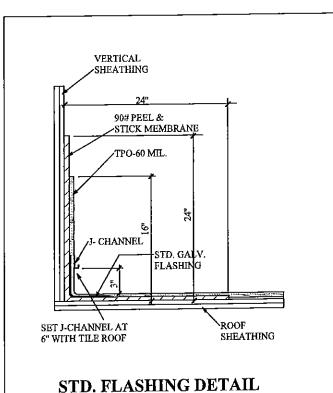
INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

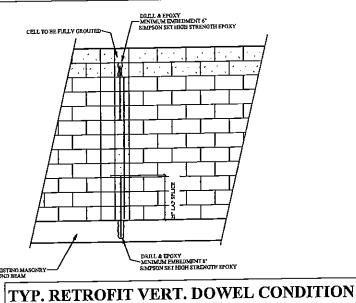
PLAN DATE

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655

DETAIL

ISNO





NOTE: MISSING DOWELS: WHERE FOOTING DOWELS ARE PLACED INCORRECTLY OR MISSIAKENLY ELIMINATED, REPLACE DOWEL AT PROPER LOCATION W/ GRADE 40 #5 BAR. INSTALL IN SLAB W/8" MINIMUM EMBERDHENT, USE PYOXY GROUT.

MISSING ANCHOR BOLTS AT BEARING WALL:

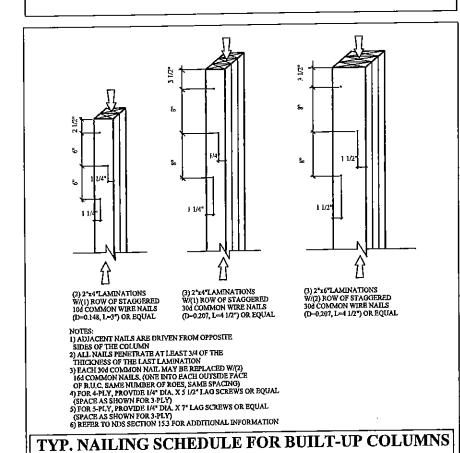
EXTERIOR BEARING WALL:

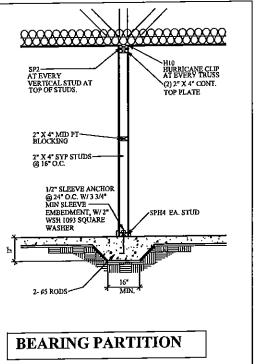
IN ADDITION TO THE GENERAL PLACEMENT REQUIREMENTS:

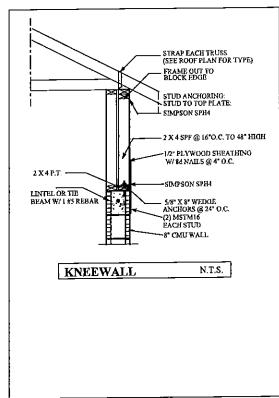
1.) 5/8" DIAMETER x 6" EMBEDMENT SIMPSON TITEN HD ANCHORS SPACED A MAXIMUM OF 24" O.C. INTERIOR BEARING WALL:

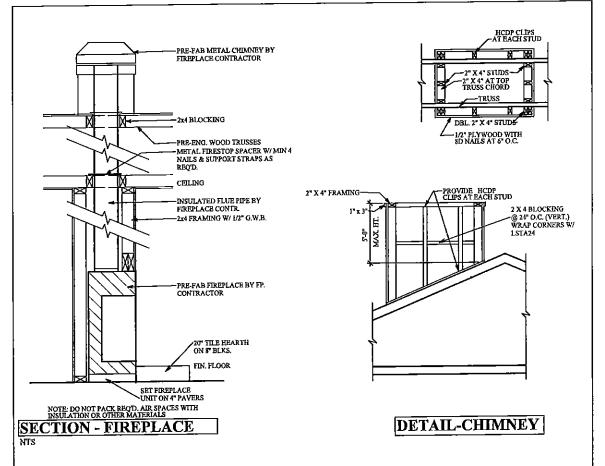
IN ADDITION TO THE GENERAL PLACEMENT REQUIREMENTS:

5.%* DIAMETER x 6* EMBEDMENT SIMPSON TITEN HD ANCHORS SPACED A MAXIMUM OF 24* O.C. IF RESISTING UPLIFT LOADS OR 3 1/3* EMBEDMENT AT 48* O.C. IF RESISTING GRAVITY LOADS











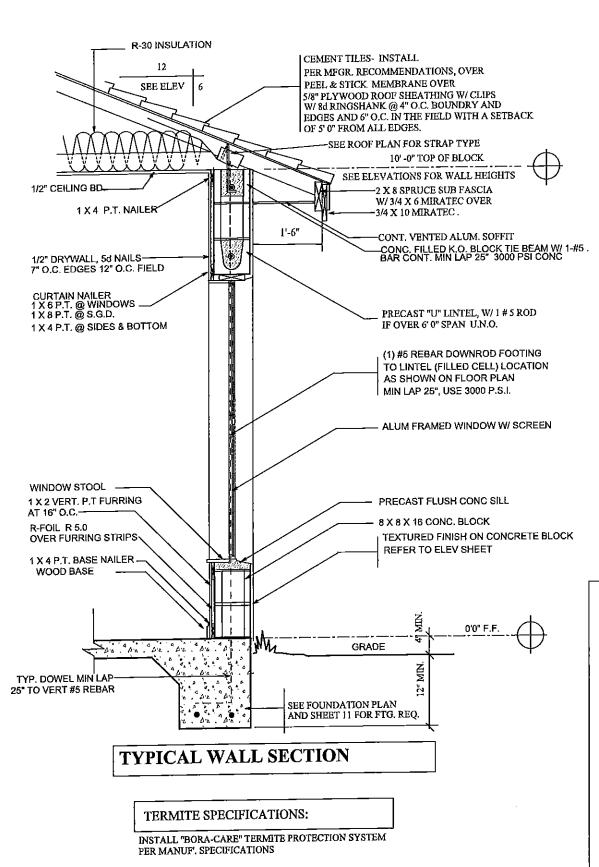
AECS

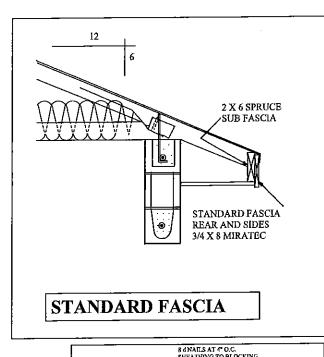
INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

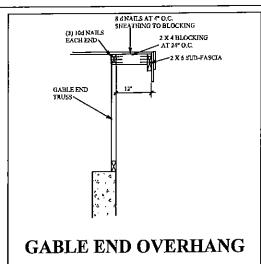
DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

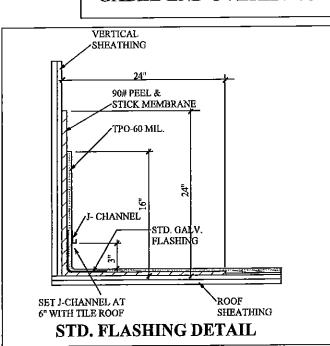
DETAILS

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AECS 14074

INVENTORY LOT 18 MAJESTIC OAKS PINELLAS COUNTY, FL.

DETAILS

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

PLAN DATE

