# NOTICE ALL WORK SHALL COMPLY WITH PREVAILING CODES, FLORIDA BUILDING PREVAILING CODES, FLORIDA BUILDING CODE, MECHANICAL, PLUMBING, FUEL GAS, ALUMINUM AND N.E.C. BUILDING

Pasco County Florida REVIEWED FOR CODE COMPLIANCE Approval of these documents constitutes authority to proceed with the work but does not grant authority to violete, cancel, after or set aside any of the technical codes

PER FFPC FIFTH EDITION 1:1.14.4 Review and approval by the AHJ shall not relieve the applicant of the responsibility of compliance with this code.

> SPRINKLER SYSTEM having a full sprinkler system. Ellmination of the sprinkler system will void the permit.

# **COMMENTS**

- 1. ALL BATHROOM AND BEDROOM DOORS TO BE 34" WIDE X 80" HIGH
- 2. BUILDING TYPE TO BE 5B
- 3. UL DESIGN # FOR 8" BLOCK WALL IS U905
- 4. INTERIOR FLAME / SMOKE DEVELOPMENT CLASSIFICATION FOR INTERIOR FINISHES IS CLASS - C PER TABLE 803.3

# FIRE PREVENTION CODE

- 1. THIS BUILDING IS LIGHT WEIGHT TRUSS CONSTRUCTION AND IS CURRENTLY IN COMPLIANCE WITH SS 666.222
- 2. THIS BUILDINGIS IN COMPLIANCE WITH THE FLORIDA FIRE PREVENTION CODE FIFTH EDITION
- 3. SMOKE ALARMS SHALL BE INSTALLED IN ACCORDANCE WITH 9.6.2.10 AND SHALL BE INTERCONNECTED
- 4. BUILDING IS PROTECTED BY AN APPROVED, SUPERVISED AUTOMATIC SPRINKLER SYSTEM.
- 5. FIRE SEPERATION RATING FOR THE BLOCK WALL IS 2 HOUR, (U905) FRAME WALL IS 1 HOUR (U338)
- 6. NO APARTMENT EXCEEDS THE MAX. 3000 SQUARE FEET REQUIREMENT FOR DRAFTSTOPS, EACH UNIT IS SEPERATED BY A ONE HOUR FIREWALL (U338) IN THE ATTIC

# **GENERAL NOTES:**

THE FOLLOWING TECHNICAL CODES SHALL APPLY: 2014 FLORIDA BUILDING CODE, PLUMBING, MECHANICAL, FUEL GAS, ENERGY EFFICIENCY, ACCESSIBILITY, AND NATIONAL ELECTRICAL CODES

- 1. TANK TYPE WATER CLOSET VOLUME
- 2. WALL MOUNT WATER CLOSET VOLUME 3.5 GALLONS
- 3. WATER FLOW RATE.

PUBLIC FACILITIES 0.5 G.P.M. PRIVATE FACILITIES 2.2 G.P.M. 2.5 G.P.M.

VTR LOCATIONS ARE APPROXIMATE AND MAY CHANGE DUE TO JOBSITE CONDITIONS

THE FOLLOWING SHALL COMPLY WITH THE 2014 FBC.

- □ PORCHES AND BALCONIES
- ☐ HANDRAILS
- ☐ GUARDRAILS
- ☐ STAIRS
- ☐ CHIMNEY & FIREPLACE
- □ EGRESS WINDOWS
- 4. ALL OPENINGS SHALL COMPLY WITH 2014 FBC WIND LOADS AS STATED BELOW. ATTACHMENTS OF WINDOWS, DOORS, SLIDING GLASS DOORS AND O.H. GARAGE DOORS ARE DELEGATED THE MANUFACTURER OF THESE ITEMS. THE MANUFACTURER OF THESE ITEMS SHALL SUBMIT ATTACHMENTS TO ENGINEER OF RECORD FOR REVIEW PRIOR TO INSTALLATION. SEE ATTACHED SPECIFICATION SHEETS FOR MANUFACTURERS DESIGN CRITERIA AND INSTALLATION METHODS FOR WINDOWS, DOORS, SLIDING GLASS DOORS, OVERHEAD GARAGE DOORS, AND ROOFING.
- 5. ALL DOORS INTERIOR & EXTERIOR ARE 8' 0" UNLESS OTHERWISE NOTED ALL SHOWER ENCLOSURES TO BE TEMPERED GLASS
- 6. ALL WINDOWS WITHIN 24" OF DOORS (INTERIOR & EXTERIOR) AND WITHIN 18" OFF FLR TO BE TEMPERED GLASS.



AL ROBBIAN DESIGN AL ROBBIAN A.I.B.D. 6397 CONNEWOOD SQ. INW PORT NICHEY, FL. 34653



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ALLEN ENGINEERING AND CONSTRUCTION SERVICES, INC. (AECS) IS NOT RESPONSIBLE FOR THE ARCHITECTURAL DESIGN, IT'S FEATURES AND ASSOCIATED DIMENSIONS. THE ARCHITECTURAL INFORMATION IS ACCEP AS BEING ACCURATE AND IS USED BY AECS SOLELY FOR THE PURPOSE OF DETERMINING STRENGTH, FIRE PROTECTION, AND FLOOD RESISTANCE CONSTRUCTION REQUIREMENTS.

# INDEX OF DRAWINGS

SHEET TITLE

13

STRUCTURAL ENGINEER NOTESHALL COMPLESTRUCTURAL ENGINEER NOTESHALL COMPLESTRUCTURAL ENGINEER NOTES DES, FLORID STRUCTURAL ENGINEER NOTES AL, PLUMBIN WIND LOAD DESIGN DATA AND N.E. **COVER SHEET** S1 S2 S3 S4 BUILDING -E-FLOOR PLANLUMINUM AND N.1 **BUILDING -E- ELEVATION B2 B**3 **BUILDING -E- FOUNDATION FOUNDATION PLANS** FLOOR PLAN NOTES **DIMENSION PLANS EXTERIOR ELEVATIONS** 5 **ELECTRICAL RISERS** ROOF PLANS TRUSS PLANS 6A **ELECTRICAL PLANS** CONSTRUCTION DETAILS CONSTRUCTION DETAILS 10 **TYPICAL WALL SECTIONS** 11 TYPICAL FOOTING DETAILS 12 **ACCESSIBILITY REQUIREMENTS** 

**ACCESSIBILITY REQUIREMENTS** 

**ACCESSIBILITY REQUIREMENTS** 

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE DEEB FAMILY HOMES, LTD

# **NOTICE TO SUBCONTRACTORS:**

DUE TO SPACE LIMITATIONS IN THIS 11"X 17" PLAN FORMAT, AND TO ELIMINATE CLUTTER AND TEXT READABILITY ISSUES, SOME DETAILS AND NOTATIONS MAY OR MAY NOT BE LOCATED ON THE SAME SHEETS OR IN THE SAME LOCATIONS AS PROVIDED FOR BY OTHER CONTRACTORS OR ARCHITECTS. IT WOULD BE IN YOUR BEST INTREST TO REVIEW THESE PLANS AND LOCATE THE APPROPORIATE INFORMATION REQUIRED TO COMPLETE YOUR SPECIFIC PORTION OF THE JOB BEFORE BEGINNING CONSTRUCTION.

IT IS THE INTENT OF THIS DESIGNER THAT THESE PLANS ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE LICENSED PROFESSIONAL TO CONSTRUCT THIS PROJECT. IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION..STOP..AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROFESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY REVIEW THESE DOCUMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE

BEFORE ANY WORK IS DONE.

**NOTICE TO BUILDER** 

WINDOW INSTALLATION NOTES:

PER MFG'S. DETAIL REQUIREMENTS PER DESIGN CRITERIA NOTED ON THESE DRAWINGS. , WINDOWS ARE IMPACT RESISTANT TYPE, STORM SHUTTERS OR PANELS ARE NOT REQUIRED. . ROOF , WALLS AND WINDOW FASTENINGS MUST BE

. WINDOWS MUST BE FASTENED INTO STRUCTURAL MEMBERS

ENGINEERED AND SPECIFIED FOR CUMULATIVE INTERNAL PRESSURE AND EXTERNAL NEGATIVE (SUCTION) PRESSURES WHICH VARIES ACCORDING TO AREAS AS NOTED IN THE DESIGN CRITERIA AS NOTED ON PAGE S4.

# BUILDING SHEET COVE

NOTICE

#### ADMINISTRATIVE

- 1. THE ENGINEERING FIRM FOR THIS STRUCTURAL DESIGN IS ALLEN ENGINEERING AND CONSTRUCTION SERVICES,INC. HEREIN REFERRED TO AS " AECS OR " A.E.C.S ".

  2. THE ENGINEER FOR THIS STRUCTURAL DESIGN IS
- RICHARD B. ALLEN, PE. HEREIN REFERRED TO AS "STRUCTURAL ENGINEER".
- 3. THE STRUCTURAL ENGINEER DESIGN NOTES ARE PART OF THE STRUCTURAL DESIGN AND ARE TO BE TAKEN AS TYPICAL REQUIREMENTS UNLESS NOTED OTHERWISE, "UNO", IN THE STRUCTURAL PLANS AND STRUCTURAL DETAILS. 4. THE DESIGN SHOWN IN THESE PLANS CONFORM TO THE STRUCTURAL PROVISIONS OF THE CHAPTER 16 OF THE FLORIDA BUILDING CODE.
- 5. THE PURPOSE OF THESE PLANS IS TO OBTAIN A BUILDING PERMIT AND FOR SUBSEQUENT CONSTRUCTION OF THE DESIGN AS SHOWN. THESE PLANS ARE TO BE CONSIDERED VOID IF WORK COMMENCES PRIOR TO A PERMIT BEING ISSUED, A CHANGE IN THE BUILDING CODE OCCURES PRIOR TO THE PLANS BEING SUBMITTED FOR PERMIT OR AFTER SIX MONTHS OF THE DATE THAT THESE PLANS ARE SIGNED AND SEALED WITHOUT BEING SUBMITTED FOR PERMITTING, WHICHEVER OCCURES FIRST. ONCE
  A BUILDING PERMIT HAS BEEN ISSUED BASED ON THESE PLANS,
  THE BUILDING DEPARTMENT IS NOT AUTHORIZED TO REISSUE OR
  TRANSFER BUILDING PERMITS WITHOUT THE EXPRESSED WRITTEN
  CONSENT OF THE STRUCTURAL ENGINEER.
- 6. CONSTRUCTION BASED ON THE STRUCTURAL DESIGN IS TO BE DONE AS SHOWN IN THE PLANS WITHOUT DEVIATION, CHANGE OR OMISSION WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER, IF ADDITIONAL DETAIL INFORMATION, OR EXPLANATION IS NEEDED. IT IS TO BE OBTAINED FROM THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ADDITIONAL PARTS OF THESE PLANS, INCLUDING PROVISIONS AS STATED IN ITEM 4.
- 7. IT IS IMPORTANT TO UNDERSTAND THAT STRUCTURAL PROVISIONS OF THE BUILDING CODE ARE COMPLICATED AND THESE PLANS ARE INTENDED TO BE USED BY AN EXPERIENCED BUILDING CONTRACTOR. PROPERTY OWNERS OBTAINING OWNER-BUILDER PERMITS ARE PROCEEDING AT THEIR OWN RISK. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ERRORS OR OMISSIONS BY PROPERTY OWNERS OR THEIR AGENTS AS A RESULT OF ANY MISUNDERSTANDING OF THE PLANS THE OTHERWISE WOULD BE UNDERSTOOD BY A LICENSED CONTRACTOR. 8. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, AND SCHEDULE. 9. THE STRUCTURAL PLANS AND ANY RELEVANT DESIGN DOCUMENTS PRODUCED UNDER THE DIRECT CHARGE OF THE STRUCTURAL ENGINEER ARE THE PROPERTY OF THE STRUCTURAL ENGINEER AND MAY NOR BE USED BY ANY PERSON OTHER THAN THE CONTRACTED CLIENT AND FOR ANY PURPOSE OTHER THAN THAN THAT STATED IN ITEM 5 ABOVE WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER. MOREOVER, NO OTHER ENGINEER OR ARCHITECT IS TO BE DESIGNATED A DELEGATED ENGINEER FOR ANY PURPOSE RELATED TO THESE STRUCTURAL PLANS OR CONSTRUCTION BASED ON THESE PLANS PRIOR TO THE ISSUANCE OF A CERTIFICATE OF COMPLETION OR OCCUPANCY WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

#### DESIGN CRITERIA

10. LOAD COMBINATIONS: THIS DESIGN IS BASED ON AN " ALLOWABLE -STRESS " FORMULATION RELYING ON THE LOAD COMBINATIONS DEFINED IN FBC 2014 SECTION 1605.3.1 OR SECTION 1605.3.2 WHERE OMEGA EQUALS 1.3 11. FOUNDATION LOADS: SEE NOTES ON " SITE CONDITIONS, SOILS, AND FOUNDATIONS".

13. INFORMATION CONTAINED ON A PLAN SHEET WHERE HIS SIGNATURE AND SEAL APPEAR, THAT DOES NOT PERTAIN TO THE RELEVANT STRUCTURAL PROVISIONS NOT PERTAIN TO THE RELEVANT STRUCTURAL PROVISIONS
AS STATED IN ITEM 4, INCLUDING, BUT NOT LIMITED TO THE
BUILDING OCCUPANCY, THE ARCHITECTURAL DESIGN, ITS
FEATURES, FINISHES (I.E., DECORATIVE STUCCO, SIDING,
ROOFING, SOFFITS, FLASHING, PAINTING, ETC.) AND THEIR
INSTALLATION, DIMENSIONS, AND ANY DESIGN OF FIRE
PROTECTION, ELECTRICAL, PLUMBING, AND MECHANICAL COMPONENTS OR SYSTEMS. THE ARCHITECTURAL INFORMATION, INCLUDING DIMENSIONS

SHOWN IN THESE PLANS AND PROVIDED TO THE ENGINEER.

#### SITE CONDITIONS

18. SITE PLAN AND TOPOGRAPHY

18. STEPLAN AND TO CORATI A. THE STRUCTURAL ENGINEER IS NOT A SUVEYOR AND IS NOT RESPONSIBLE FOR THE SITE PLAN, ESTABLISHING REQUIRED SET-BACKS, AND LOCATING THE BUILDING ON THE PROPERTY. B. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE GRADING OF THE SITE OR ITS COMPLIANCE WITH ANY DRAINAGE PLAN WHETHER INDIVIDUAL OR AS A PART OF A MASTER

DRAINAGE PLAN.
C. THE FOUNDATION DESIGN IS BASED ON THESE PRESUMED
CONDITIONS INCLUDING THAT DIFFERENTIAL SETTLING DOES
NOT EXCEED THE SAFE LIMITS OF THE FOUNDATION DESIGN
(INCLUDING STEMWALLS AND MASONRY ABOVE GRADE WALLS)
AS STATED IN ITEM 19 BELOW.

AS STATED IN TIEM 13 BELOW.

D. IT IS IMPORTANT TO KNOW THAT THE FOUNDATION DESIGN
BASED ON A PRESUMED ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF RELIES ON LESS THAN L/500 (E.G.,0.25 INCHES OVER 10 FEET ) OF DIFFERENTIAL SETTLEMENT, CRACKS IN MASONRY WALLS SHOULD BE EXPECTED WHERE DIFFERENTIAL SETTLEMENT EXCEEDS L/150.THIS STATEMENT SHOULD BE TAKEN AS A CAUTIONARY NOTE FOR PROCEEDING WITHOUT A SOILS ANALYSIS AND FOUNDATION RECOMMENDATION BY A GEOTECHNICAL ENGINEER FOR THE SITE.

E. COPIES OF ANY AND ALL REQUIRED COMPACTION TESTS ARE TO BE PROVIDED TO THE BUILDING DEPARTMENT FOR THEIR

STRUCTURAL ELEMENTS
19. FOUNDATION, FOOTING AND GROUND FLOOR SLAB
A. THE FOUNDATION AND FOOTINGS ARE TO BEAR A MINIMUM ON 12 INCHES BELOW GRADE AND ARE TO BE PLACED ON UNDISTURBED SOIL OR FILL COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR PURSUANT TO ASTM D 1557 WITH

FILL LIFTS LESS THAN 12". COMMERCIAL

ALL LIVE LOADS PER FBC 2014 TABLE 1607.1
14. ROOF LIVE LOADS:
ALL ROOF / WOOD CONSTRUCTION TYPES ARE 30 PSF.

15. DEAD LOADS: FLOOR WOOD FRAME: 35 PSF FOR TILE/MARBLE FLOOR

COVERING, 15 PSF FOR ALL OTHERS. ROOF WOOD FRAME: 25 PSF FOR SHINGLES, 35 PSF FOR TILE 16. WIND LOADS:

A. WIND LOADS ARE BASED ON THE SPECIFIC REQUIREMENTS AND DEFINITIONS OF FLORIDA BUILDING CODE 2014 EDITION ASCE-7-10.

B. THE COMPONENT AND CLADDING WIND PRESSURES ARE THE MINIMUM REQUIREMENTS FOR STRENGTH AND IMPACT PROTECTION NEEDED FOR SELECTING SATISFACTORY COMPONENTS AND CLADDING, BY OTHERS, FOR THE STRUCTURE.

ENGINEERING BY OTHERS IS PRESUMED ACCURATE AND IS RELIED UPON BY THE STRUCTURAL ENGINEER SOLEY FOR THE PURPOSE OF ACHIEVING COMPLIANCE WITH THE RELEVANT STRUCTURE

20. FOOTINGS (AND ANY ASSOCIATED MONOLITHIC FLOOR SLABS) SHALL BE CONSTRUCTED OF CONCRETE WITH A SPECIFIC COMPRESSIVE STRENGTH OF 3,000 PSI, 3 TO 5 INCH SLUMP, AND

3/8" AGGREGATE SOILS.

A. IN ADDITION, THE STRUCTURAL ENGINEER IS NOT A CIVIL OR GEOTECHNICAL ENGINEER AND IS NOT RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SITE FOR CONSTRUCTION, INCLUDING ITS TOPOGRAPHY, DRAINAGE AND SUB-SURFACE CONDITIONS (INCLUDING WATER TABLE DEPTH ) AND FOR INTERPRETING GEOTECHNICAL DATA CONCERNING THE SITE. B. IF SOIL CONDITIONS AT THE SITE APPEAR QUESTIONABLE AS DETERMINED BY THE BUILDING CONTRACTOR OR OWNER-AS DETERMINED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER, A SOILS ANALYSIS SHALL BE PERFORMED BY A LICENSED GEOTECHNICAL ENGINEER THAT WILL GIVE SPECIFIC RECOMMENDATIONS FOR A FOUNDATION TYPE. IF THE BUILDING CONTRACTOR OR OWNER-BUILDER DO NOT MAKE THAT DETERMINATION AND A SOILS ANALYSIS IS NOT PERFORMED, THE STRUCTURAL ENGINEER SHALL PROCEED WITH THE DESIGN BASED ON THE PRESUMPTIONS ALLOWED BY THE FBC 2012, SEC. 1804 C. THE DETERMINATIONS OF THE SUITABILITY OF THE SITE FOR CONSTRUCTION (INCLUDING TOPOGRAPHICAL INFORMATION) AND THE SOIL CONDITIONS SHALL HAVE BEEN COMPLETED AND ANY RECOMMENDATIONS RESULTING FROM THAT ANALYSIS SHALL HAVE BEEN PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO THE SIGNING AND SEALING OF THE STRUCTURAL PLANS.
D. IN THE ABSENCE OF GEOTECHNICAL INFORMATION, THE SITE IS PRESUMED TO HAVE AN ALLOWABLE SOIL BEARING CAPACITY OF 2000 PSF AND THE TOPOGRAPHY AS IT RELATES TO THE STRUCTURE IS PRESUMED TO BE THAT SHOWN IN THE PLANS. E. THE SIZE AND REQUIRED REINFORCEMENT FOR THE FOOTINGS ARE SHOWN ON THE FOUNDATION PLAN.
THE GROUND FLOOR SLAB SHALL BE PLACED OVER A 6 MIL.
POLYETHYLENE MOISTURE RETARDER.

I. THE TRUSS SYSTEM DESIGN PROVIDED IN THIS PLAN IS FOR THE USE OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL ROOF TRUSS SYSTEM DESIGN. IT IS NOT TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL DESIGN. II. MANUFACTURED FLOOR TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.

III. THE MANUFACTURED TRUSS DESIGN SHALL INCLUDE SPECIFYING THE TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTIONS ON EITHER THE INDIVIDUAL TRUSS COMPONENT SHEETS OR THE GIRDER TRUSS COMPONENTS SHEETS AS APPLICABLE. A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION THAT A HANGER IS REQUIRED IN THE

TRUSS SYSTEM.

IV. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION ON THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED UPON THE FINAL FLOOR TRUSS

F. CONVENTIONAL FRAMED JOISTS WITH A MINIMUM 6 INCH OVERLAP OF JOINTS.

G. TERMITE TREATMENT OF THE SITE SHALL BE SPECIFIED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER. NOTICE H. SHRINKAGE CONTROL OF THE FLOOR SLAB SHALL BE
ACCOMPLISHED BY 6 INCH BY 6 INCH. W 1.4 BY 1.4 WELDED

WIRE FABRIC AS SPECIFIED BY FBC 2014 SECTION 1910.2 WORK SHALL COMPLY
WIRE FABRIC AS FECIFIED BY FBC 2014 SECTION 1910.2 WORK SHALL COMPLY
FBC 2014, SECTION 1910.2 EXCEPTION 1. THE WELDED WIRE
FBC 2014, SECTION 1910.2 EXCEPTION 1. THE WELDED WIRE
FABRIC SHALL BE PLACED BETWEEN THE MIDDLE AND TEPER ANICAL, PLUMBING,
FABRIC SHALL BE PLACED BETWEEN THE MIDDLE AND TEPER ANICAL, PLUMBING,
1/3 DEPTH OF THE SLAB AND HELD IN POSITION BY APPROPLATE JMINUM AND N.E. C
SUPPORTS SPACED NOT GREATER THAN 3 FEET APART.
L CONTRACTION IOINTS ARE TO BE PROVIDED FOR THE H. SHRINKAGE CONTROL OF THE FLOOR SLAB SHALL BE I. CONTRACTION JOINTS ARE TO BE PROVIDED FOR THE PURPOSE OF CONTROLLING SHRINKAGE ONE INCH DEEP CUTS (FOR A FOUR INCH THICK SLAB OR 25 PERCENT OF THE SLAB THICK NESS OTHERWISE) ARE TO BE PROVIDED ACROSS THE WIDTH AND LENGTH OF ANY FLOOR SLAB AT A DISTANCE OF NOT TO EXCEED 30 TIMES THE SLAB THICKNESS. FOR EXAMPLE A FOUR INCH THICK SLAB, CONTRACTION JOINTS SHALL NOT EXCEED 10 FEET ON CENTER EACH WAY.

ALLEN ENGINEERING & CONSTRUCTION SERVICI RICH ALLEN PROFESSIONAL ENGI P.E. # 56920 C.A. # 9542 8899 SKYMASTER DR. NEW PORT RICHEY FL. 34654 S. 1602

& ELK MODE

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

NOTE

GINEER

ENS ENS ENS

STRUCTUR

FAMILY LTD HOMES,

A. MANUFACTURED FLOOR TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPANY

- FLOOR JOISTS ARE SIZED BASED ON THE SOUTHERN PINE COUNCIL SPAN TABLES FOR NO. 2 GRADE DIMENSIONAL LUMBER.
- II. FLOOR JOISTS FOR EXTERIOR DECKS SHALL BE PRESSURE TREATED.
- B. FOR ALL WOOD FLOORS:
  L. THE TRUSS TO WALL CONNECTIONS ARE IDENTIFIED ON THE FLOOR FRAMING PLAN.
- II. A STRUCTURAL BAND JOIST IS TO BE PROVIDED ON THE EXTERIOR PERIMETER OF ALL BOTTOM BEARING FLOOR TRUSSES AND JOISTS. THE STRUCTURAL BAND JOIST IS TO BE FASTENED TO EACH END OF A FLOOR TRUSS OR JOIST WITH A SIMPSON L50 BRACKET USING SIMPSON SHORT 10d COMMON NAILS.
- III. FLOOR TRUSSES OR JOISTS BEARING ON WOOD WALLS ARE TO BE SET WITH A MINIMUM OF THREE 10d COMMON NAILS.(TOE NAILED) TO THE TOP PLATE OF THE WALL.
- IV. A MOISTURE BARRIER SHALL BE INSTALLED BETWEEN ANY UNTREATED WOOD TRUSSES OR JOISTS AND CONCRETE OR ANY MASONRY.
- V. LEDGERS/ NAILERS SHALL BE FASTENED TO WOOD STUDS
  OR BAND JOISTS (NOT SHEATHING) WITH A MINIMUM 2 3/8" X
  5 1/2" LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION
  AT 16 INCHES ON CENTER AND SHALL CONSIST OF PRESSURE TREATED LUMBER 2 PLY 1 1/2" THICK BY A HEIGHT SHOWN IN THE PLANS, FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8" X 5 1/2" SIMPSON TITEN HEAD CONCRETE BOLTS.
- VI. FLOOR BEAMS
- 1. BEAMS SUPPORTING FLOOR TRUSSES AND JOISTS ARE TO BE ATTACHED AS SPECIFIED IN THE FLOOR FRAMING PLAN.
- 2. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.
- 3. MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (I.E. GLULAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS
- MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE INDIVIDUAL PLIES INTERCONNECTED AS FOLLOWS: A. FOR TWO PLY BEAMS- ONE ROW OF 10d GALVANIZED COMMON
- NAILS AT 6" O.C. ON EACH SIDE OF THE BEAM B. FOR THREE PLY BEAMS- TWO ROWS OF 16d GALVANIZED COMMON NAILS SPACED AT 6" O.C. (TOP AND BOTTOM) THRU EACH SIDE OF BEAM.
- C. FOR FOUR PLY BEAMS OR LARGER-TWO ROWS OF 1/2" DIAMETER CARRIAGE BOLTS OR ALL THREAD ROD WITH NUTS AND WASHERS SPACED AT 12 INCHES ON CENTER, 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM.
- D. FLOOR SHEATHING:
- I. ALL FLOOR SHEATHING IS TO BE 3/4" TONGUE AND GROOVE PLYWOOD RATED FOR FLOOR SHEATHING APPLICATION.
- II. FLOOR SHEATHING SHALL BE FASTENED TO THE FLOOR TRUSSES JOISTS WITH 10d RING SHANK NAILS AT 6" ON CENTER WITH CONSTRUCTION GRADE ADHESIVE.
- III. FLOOR SHEATHING SPECIFIED FOR SEALED EXTERIOR DECKS AND ITS INSTALLATION SHALL BE THE SAME AS THAT FOR INTERIOR APPLICATION EXCEPT PRESSURE TREATED AND THE FASTENERS TO BE GALVANIZED. E. EXTERIOR DECK FLOORING:
- DECK FLOORING SHALL BE INDIVIDUALLY SPECIFIED ON THE FLOOR FRAMING PLANS AND SHALL BE FASTENED TO THE UNDERLYING PRESSURB TREATED JOISTS WITH 3-3 INCH DECK SCREWS AE EACH FLOORING JOIST INTERSECTION

- CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI.
- II. WALL CMU SHALL BE 8 INCH X 16 INCH IN SIZE OR 8 INCH X
- 8 INCH X 8 INCH FOR EDGE FINISHES.

  III. CMU SHALL BE PLACED IN A RUNNING BOND AND THERE SHALL BE NO VERTICAL BUTT JOINTS EXCEPT AS SHOWN ON THE PLOOR PLAN FOR CONSTRUCTION JOINTS.
- IV. REINFORCED FILLED CELLS AS SHOWN ON THE PLANS SHALL BE FILLED WITH "FINE" GRADE GROUT, HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AND 8 TO 11 INCH SLUMP TO ENSURE CONSOLIDATION.
- V. BOND BEAMS SHALL BE POURED WITH GROUT MONOLITHICALLY WITH THE FILLED WALL CELLS-NO COLD JOINTS.
- VI. VERTICAL STEEL REINFORCEMENT SHALL BE CONTINUOUS BETWEEN THE MIDDLE AND BOTTOM 1/3 OF THE FOOTING HEIGHT AND END IN THE TOP COURSE OF THE BOND BEAM WITH A STANDARD 10 INCH 90 DEGREE BEND.
- VII. HORIZONTAL REINFORCING STEEL SHALL BE CONTINUOUS, INCLUDING
- VIII. REINFORCING STEEL SPLICES SHALL CONSIST OF WIRE LAPS NO LESS THAN 40 TIMES THE STEEL BAR DIAMETER (I.E. 25 INCHES FOR #5 REBAR, 15 INCHES FOR #3 REBAR, AND 52 INCHES FOR #7 REBAR)
- B. WOOD FAME WALLS:
- I. WALL STUD SIZES ARE SHOWN IN THE TYPICAL WALL SECTION.
- II. LOAD BEARING.

  1. WOOD STUDS IN WALLS SHALL BE SPACED 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES PER THE TOP PLATE SPLICE DETAIL, ALL LOAD BEARING STUDS TO BE SOUTHERN YELLOW PINE #2 GRADE OR BETTER
- 2. LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED) IN CONTACT WITH MASONRY OR CONCRETE. SEE THE TOP PLATE SPICE DETAIL FOR TOP PLATE NAILING AND SPLICING REQUIREMENTS.
- 3. THE WOOD STUDS SHALL HAVE A SIMPSON SP2 AT THE TOP PLATE AND A PROPERLY SIZED SPH FOR THE BOTTOM PLATE (I.E. 4" STUD WALL = SPH4, 6" STUD WALL = SPH6)
- 4. 3 STUD PACK SHALL BE INSTALLED DIRECTLY BENEATH BEARING POINTS OF ALL GIRDERS AND BEAMS HAVING A GRAVITY LOAD OF UP TO 3,000 LBS.

  5. STEEL TUBE COLUMNS SHALL BE INSTALLED IN THE WALL DIRECTLY BENEATH
- STEEL TUBE COLUMNS SHALL BE INSTALLED IN THE WALL DIRECTLY BENEFORDERS AND BEAMS HAVING GRAVITY LOADS GREATER THAN 3000 LBS.
   BASE PLATES SHALL BE FASTENED TO MONOLITHIC FOOTINGS WITH 5/8" X 8 INCH ANCHOR BOLTS OR SIMPSON TITEN HID. CONCRETE BOLTS OF THE SAME SIZE AT 24 INCHES ON CENTER. ALL CONNECTIONS SHALL BE MADE WITH 3 INCH SQUARE BY 1/8 INCH THICK WASHERS
   BASE PLATES BEARING ON WOOD SHALL BE FASTENED WITH 16d COMMON NAILS AT 8" O.C. THROUGH ANY FLOOR SHEATHING AND TO UNDERLYING LUMBER (NOT SHEATHING ONLY ) AND USE BLOCKING AS NEEDED TO MAINTAIN NAILING SPACING REQUIREMENTS.
   FOR EXTERIOR LOAD BEARING WALLS, EACH STUD ABOVE THE BASE PLATE SHALL BE FASTENED TO THE UNDERLYING BAND JOIST OR BEAM WITH A SIMPSON LSTAIS STRAP.FOR THIS SITUATION THE SIMPSON SPH BRACKET TO THE BASE PLAN MAY BE OMITTED.
- TO THE BASE PLAN MAY BE OMITTED.
- 9. FOR INTERIOR LOAD BEARING WALLS, 1/2 INCH ALL THREAD ROD SHALL BE INSTALLED AT 32" O.C. FROM THE BASE PLATE THROUGH THE SHEATHING AND TOP PLATE OF UNDERLYING SUPPORTING WALL. ALL CONNECTIONS SHALL INCLUDE A STANDARD 3 INCH SQUARE WASHER.
- 10. HEADER BEAMS SHALL BE SIZED ACCORDING TO THE ENCLOSED HEADER SCHEDULE AND FASTENED WITH A MINIMUM OF TWO SIMPSON LSTA36 STRAPS OVER EACH END TO THE JACK STUDS BELOW. IN ADDITION, THE HEADER BEAMS SHALL BE FASTENED WITH A MINIMUM OF 3-10d COMMON NAILS ( TOE NAILED ON EACH FACE SIDE AT EACH END TO THE ABUTTING FULL LENGTH STUDS.
- III. NON LOAD BEARING WALLS:
- WOOD STUDS IN WALLS SHALL BE SPACED AT 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES WITH A MINIMUM OF THREE 10d COMMON NAILS. NAILS INSTALLED IN PRESSURE TREATED WOOD SHALL
- 2. INCIDENTAL, NON STRUCTURAL FRAMING ITEMS SUCH AS KNEE WALLS, DROP CEILINGS, BUILT IN SHELVING, NICHES, ETC. MAY BE CONSTRUCTED WITH 2 X 4 'S AT 24" O.C. AT THE DISCRETION OF THE BUILDER

- 2. NON LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED AGAINST MASONRY AND CONCRETE ) AND A SINGLE TOP PLATE.
- 3. BASE PLATES SHALL BE FASTENED TO CONCRETE SLABS WITH 1/4 INCH BY 3 1/2 INCH TAPCON SCREWS AT 12 " ON
- 4. BASE PLATES ON WOOD SHALL BE FASTENED WITH 16d COMMON NAILS AT 8" ON CENTER.
- C. SHEATHING
- I, PLYWOOD SHEATHING.
- 1. EXTERIOR WALL SHEATHING COVERED BY AN ARCHITECTURAL FINISH SHALL BE MINIMUM 7/16 INCH THICK (NOMINAL) 4 PLY PLYWOOD MANUFACTURED WITH EXTERIOR GLUE.

  2. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE WALL STUDS.

  1. EASTEN TO STUDS AND BY OCKING WITH 84 RING SHANK NAILS.
- 3. FASTEN TO STUDS AND BLOCKING WITH 8d RING SHANK NAILS
- AT 4 INCHES ON CENTER ALL LOCATIONS.

  AIT AND TO THE REGULAR FASTENING, A SECOND ROW SHALL BE INSTALLED AT THE DOUBLE TOP PLATE AND TO THE LOWEST HORIZONTAL WOOD MEMBER ON AN EXTERIOR WALL.
- (I.E. SILL PLATE, BAND JOIST)
  5. FOR PLYWOOD SHEATHING COVERED WITH A CEMENTITIOUS FINISH ALL BUTT JOINTS NOT ON WALL STUDS SHALL BE BLOCKED WITH 2 X BLOCKING, TOE NAILED AT EACH END TO THE WALL STUDS WITH 3-8d COMMON NAILS.
- II. PARTICLE BOARD
- 1. PARTICLE BOARD IS NOT TO BE USED WITHOUT THE EXPRESS, WRITTEN CONSENT OF THE STRUCTURAL ENGINEER AND THE PROPERTY OWNER.
- III. ARCHITECTURAL FINISHES
- 1. ARCHITECTURAL WALL FINISHES, SUCH AS STUCCO, CEMENTITIOUS COATING, SIDING OR PAINT ARE MENTIONED HERE ONLY FOR THE PURPOSE OF UNDERSTANDING THAT THEIR INSTALLATION AND ASSOCIATED DETAILS ARE NOT THE RESPONSIBILITY OF THE

- A. CONCRETE / MASONRY COLUMNS

  1. MASONRY COLUMNS SHALL BE CONSTRUCTED OF PILASTER CONCRETE
  BLOCK OR FORMED AND POURED. WALL BLOCK SHALL NOT BE USED FOR MASONRY COLUMNS.
- II. REINFORCING STEEL SHALL BE GRADE 60 AND HELD IN PLACE BY STRUPS SPACED AT 12 INCHES ON CENTER VERTICALLY.

  III. PILASTER BLOCK COLUMNS SHALL BE FILLED WITH A FINE GROUT
- HAVING A MINIMUM OF COMPRESSIVE STRENGTH OF 3,000 PSI
- IV. FORMED AND POURED COLUMNS SHALL CONSIST OF A MINIMUM OF 3,000 PSI CONCRETE, OR IN AREAS OF HIGH CHLORIDES, SUCH AS NEAR THE COAST OR BODIES OF SALT WATER, THE MINIMUM
- V. ALL MASONRY COLUMNS SHALL BEGIN AT THE FOUNDATION OR AT A MONOLITHIC FOOTING, IN NO CASE SHALL THERE BE A BREAK OR A COLD JOINT IN THE GROUT OF A COLUMN EXCEPT AT 1 FOOT FROM THE TOP IN PREPARATION FOR INSTALLATION OF A CONCRETE LINTEL
- VI. METAL CONNECTORS AT THE TOP OF THE COLUMN FOR HOLDING
  WOOD BEAMS OR GIRDERS SHALL BE INSTALLED WITH THE MINIMUM
  EMBEDMENT OF THE ASSOCIATED FASTENERS FOR THE CONNECTOR

- AS SHOWN ON THE PLANS.

  B. WOOD COLUMNS:

  1. ALL LOAD BEARING WOOD COLUMNS SHALL BE A MINIMUM OF #2
  GRADE PRESSURE TREATED WOOD.

  II. DIMENSIONAL WOOD COLUMNS OF 4 INCHES BY 4 INCHES IN CROSS
  SECTION SHALL ONLY BE USED FOR SUPPORTING OPEN WOOD DECKS
  WHERE THE FLOOR HEIGHT ABOVE THE FLOOR BELOW IS 8 FEET OR LESS.
  ALL OTHER DIMENSIONAL WOOD COLUMNS SHALL HAVE A MINIMUM OF
  6 INCHES BY 6 INCHES.

  III. METAL CONNECTORS AT THE BASE AND THE TOP OF WOOD COLUMNS
  SHALL BE OF THE TYPE THAT RESISTS LATERAL LOADS AS WELL AS USED UNLESS OMPL
  AND GRAVITY LOADS. IN NO CASE SHALL FLAT STRAPS BE USED UNLESS OMPL
  SPECIFICALLY SHOWN IN THE PLANS OR CROSS SECTION PHARIS.

  PREVAILING CODES, FLORIDA

  CODE, MECHANICAL, PLUMBING, FUR

  ALUMINUM AND N.E.

  CODE, MECHANICAL, PLUMBING.

ELK MODE ALLEN ENGINEERING & CONSTRUCTION SERVIC RICH ALLEN PROFESSIONAL EW. # 56920 C.A. # 9542 8809 SKYMASTER DR. NEW PORT RICHEY, FL. 34654 8

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HUNTERS RIDGE NEW PORT RICHEY

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PLAN DATE

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- C. COMPOSITE COLUMNS
- 1. A COMPOSITE COLUMN HERE IS DEFINED AS A HOLLOW COLUMN CONSISTING OF ANY MATERIAL SPECIFICALLY DESIGNED BY ITS MANUFACTURER TO BE LOAD BEARING, ANY OTHER TYPE OF HOLLOW COLUMN IS CONSIDERED AN ARCHITECTURAL FINISH INTENDED TO FIT OVER A STRUCTURAL COLUMN AND ITS USE AND DETAILS OF INSTALLATION ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- STRUCTORAL ENGINEER.

  II. LOAD BEARING COMPOSITE COLUMNS ARE A MANUFACTURED PRODUCT SUBJECT TO THE DESIGN AND LOAD BEARING CAPACITY AS DETERMINED BY THE MANUFACTURER. A SHOP DRAWING OR A LETTER FOR THE INSTALLATION OF THE COLUMN SHALL BE PROVIDED BY THE STRUCTURAL ENGINEER TO SUPPLEMENT THE CONSTRUCTION PLANS AFTER THE SPECIFIC COLUMN AND MANUFACTURER HAVE BEEN IDENTIFIED.

  III. N ALL CASES, THE COLUMN MANUFACTURES INFORMATION SHALL BE
- PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT FOR REVIEW PRIOR TO ITS ACCEPTANCE FOR THE STRUCTURAL DESIGN. THE INFORMATION SHALL INCLUDE THE LATERAL AS WELL AS UPLIFT AND GRAVITY LOAD BEARING CAPACITIES.
- D., STEEL TUBE COLUMNS:
- 1. LOAD BEARING STEEL TUBE COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS OF 1/4 INCH AND BE MADE OF STEEL WITH A DESIGN YIELD STRENGTH OF 46 PSI UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN
- II. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE STEEL TUBE COLUMN IS TO BE INSTALLED.
- I. LOAD BEARING ALUMINUM COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS
- II. ALL FASTENERS AND CONNECTORS FOR ALUMINUM COLUMNS SHALL BE STAINLESS STEEL OR MONEL TO AVOID CORROSION DUE TO DISSIMILAR METALS BEING IN CONTACT.
- III. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE ALUMINUM COLUMN IS TO BE INSTALLED.

- A. MANUFACTURED WOOD TRUSSES

  I. THE MANUFACTURED ROOF TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL SYSTEM DESIGN. IT IS NOT INTENDED TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL
- II. MANUFACTURED ROOF TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.
- III. THE TRUSS PLAN " SIGNED AND SEALED" BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND PRIOR TO CONSTRUCTION OF THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED ON THE FINAL FLOOR TRUSS SYSTEM.
- VI. THE TRUSS MANUFACTURER SHALL PROVIDE ALL LATERAL BRACING
  REQUIREMENTS TO THE BUILDING CONTRACTOR. IF NOT, THE BUILDING
  CONTRACTOR IS TO NOTIFY THE STRUCTURAL ENGINEER FOR GUIDANCE.
  V. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE
  ORIGINAL PLANS, EACH TRUSS IS TO BE SET ON WOOD FRAME BEARING WALLS
  OR SILL PLATES WITH 10d COMMON NAILS (TOE-NAILED)
  VI. A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND
  CONCRETE (MASCONEY)
- CONCRETE/MASONRY
- 23.2 CONVENTIONAL FRAME
- I. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH RAFTER IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 3- 10d COMMON NAILS (TOE-NAILED)
- II. ANY WOOD COMING IN CONTACT WITH MASONRY OR CONCRETE IS TO BE PRESSURE TREATED OR A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE OR MASONRY.

- III. COLLAR TIES ARE TO BE INSTALLED BETWEEN RAFTERS AT 2/3 OF THE RIDGE HEIGHT FROM WHERE THE RAFTERS BEAR ON WALLS. THE COLLAR TIES ARE TO BE FASTENED WITH A MINIMUM OF 4-10d 16 COMMON NAILS (CLINCHED) AT EACH LAP JOINT, EACH RAFTER IS TO BE ATTACHED TO THE RIDGE BEAM WITH A LIGHT ANGLE HANGER AS SHOWN IN THE FRAMING PLAN. IN ADDITION, A FLAT METAL STRAP SHALL BE INSTALLED ACROSS THE RIDGE BEAM TO TWO OPPOSING RAFTER. TO BE REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO THE UNDERLYING STRUCTURE" CONNECTIONS
- STRUCTURE" CONNECTIONS.

  IV. AS PART OF THE REVIEW, THE STRUCTURAL ENGINEER WILL
  DETERMINE WHETHER THE TRUSS TO WALL/BEAM METAL CONNECTORS SHOWN IN THE ORIGINAL PLANS ARE ACCEPTABLE OR WHETHER THEY NEED TO BE CHANGED OR SUPPLEMENTED TO ACCOMMODATE THE LOADS SHOWN IN THE TRUSS COMPONENT
- V. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR VERIFYING THE DIMENSIONAL, ARCHITECTURAL, OR FORM ASPECTS OF THE OF THE TRUSS MANUFACTURERS PLAN WITH THE ORGINAL PLANS.
- VI. THE MINIMUM LIVE LOADS FOR THE ROOF TRUSS DESIGN IS TO BE ON FBC 2014 SECTION 1607 FOR ROOF TYPE AND ROOFING MATERIAL. VII. THE DEAD LOADS ARE LASTED IN ITEM 16 ABOVE.
- VII. THE DEAD LOADS ARE LASTED IN ITEM 16 ABOVE.

  VIII. ALL TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTORS ARE TO BE SPECIFIED BY THE TRUSS MANUFACTURER, INCLUDING CONNECTORS FOR TRUSS TO MANUFACTURER, INCLUDING OR MICROLAM) SPECIFIED BY THE TRUSS MANUFACTURER. A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION, A HANGER IS REQUIRED IN THE TRUSS SYSTEM.

  IX. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORGINALPLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER.
- MUST BE PROVIDED TO THE STRUCTURAL ENGINEER
- X. A RIDGE BEAM TERMINATING AT A GABLE END SHALL BE SUPPORTED BY A MINIMUM 3 STUD PACK COLUMN BEARING ON THE UNDERLYING
- XI. TREATED LUMBER-DOUBLE 1 1/2 INCH BY A HEIGHT SHOWN ON THE PLANS. FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8 INCH BY 5 1/2 INCH SIMPSON TITEN HD CONCRETE BOLTS.
- XII. SLEEPERS SHALL BE FASTENED TO UNDERLYING ROOF TRUSSES OR RAFTERS ( NOT SHEATHING ) WITH A MINIMUM OF 2-3/8 INCH BY 3 1/2 INCH LAG BOLTS AND WASHERS AT EACH TRUSS OR RAFTER INTERSECTION AND NO GREATER THAN 24 INCHES ON CENTER AND SHALL CONSIST OF DIMENSIONAL LUMBER 1 1/2 INCH THICK BY A WIDTH SHOWN IN THE PLANS.
- XIII. USE 2 INCH BY 4 INCH BLOCKING ATTACHED BETWEEN UNDERLYING STUDS, TRUSSES OR RAFTERS WITH A MINIMUM OF 3-104 NAILS AT EACH IN ORDER TO SATISFY THE ON CENTER SPACING FOR THE LEDGERG OF STREET
- BEAMS:

  IV BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS ARE TO BE ATTACHED

  AS SPECIFIED IN THE ROOF FRAMING PLANS.

  24. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUILT JOINTS BETWEEN

  THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES

  ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.
- A. LEDGERS/ SLEEPERS
- LEDGERS / NAILERS SHALL BE FASTENED TO WOOD STUDS (NOT SHEATHING) WITH A MINIMUM OF 2- 3/8 INCH BY 5 1/2 INCH LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AND NO GREATER THAN 16 INCHES ON CENTER AND SHALL CONSIST ON PRESSURE TREATED WOOD.
- IL MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (I.E. GLUELAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS SPECIFICATIONS.

- III. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED
- L FOR TWO PLY BEAMS ONE ROW OF 10d GALVANIZED COMMON NAILS AT 6 INCHES ON CENTER ON EACH SIDE OF BEAM.

  II. FOR THREE PLY BEAMS TWO ROWS OF 16d GALVANIZED
- COMMON NAILS AT 6" ON CENTER (TOP AND BOTTOM) THRU EACH SIDE OF THE BEAM.
- IILFOR FOUR PLY BEAMS AND LARGER- TWO ROWS OF 1/2 INCH DIAMETER CARRIAGE BOLTS OR ALL THREAD RODS WITH NUTS AND WASHERS SPACED AT 12" ON CENTER 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM.
- B. SHEATHING:
- ROOF SHEATHING COVERED BY COMPOSITE ROOFING SHALL BB A MINIMUM OF 15/32 INCH THICK (NOMINAL.) O.S.B. MANUFACTURED WITH EXTERIOR GLUE.
- II. ROOF SHEATHING COVERED BY TILE SHALL BE A MINIMUM OF 5/8 INCH THICK (NOMINAL ) MANUFACTURED WITH EXTERIOR
- III. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE ROOF TRUSS SYSTEM.
- IV. FASTENING SHALL BE 8d RING SHANK NAILS AT 4 INCHES ON CENTER AT BOUNDARY AND EDGES AND 6 INCHES ON CENTER IN THE FIELD WITH A SETBACK OF 5 '-0" FROM ALL EDGES.
- V. METAL "H" CLIPS OR SOLID WOOD BLOCKING SHALL BE USED AT ALL UNSUPPORTED BUTT JOINTS BETWEEN TRUSSES OR RAFTERS.
- 25. PRECAST CONCRETE LINTELS
- 25. PRECAST CONCRETE LINTELS
  A. PRECAST AND PRESTRESSED CONCRETE LINTELS SHALL BE
  MANUFACTURED BY CASTCRETE AND INSTALLED PER MANUFACTURES
  SPECIFICATIONS AND INSTRUCTIONS.
  B. THE SIZE OF THE LINTELS SHALL BE BASED ON THE SPAN AND LOAD.
  REFER TO THE ATTACHED SCHEDULE UNLESS OTHERWISE SHOWN IN
  THE STRUCTURAL DESIGN FOR THE SPECIFIED LINTEL
  C. LINTEL SCHEDULE U.N.O. ON PLANS:
  I. SPAN UP TO 3'- 8F8-0B
- I. SPAN UP TO 3'- 8F8-0B
- II. SPAN UP TO 3' TO < 6' 8F8-OB III. SPAN 6' TO > 14' 8F16- 1B/1T
- D. THE MINIMUM SPECIFIED GROUT COMPRESSIVE STRENGTH TO BE USED FOR LINTELS IS 3,000 PSI.
- E. THE REINFORCING STEEL SHALL BE ASTM GRADE 60
- 26. FASTENERS / METAL CONNECTORS.
- A. ALL FASTENERS AND METAL CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE AND INSTALLED PER THE MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS.
- B. THESE FASTENERS DO NOT INCLUDE TYPICAL NAILS AND SCREWS WHICH MAY BE MANUFACTURED BY OTHERS.
- C. FOLLOW ALL MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS FOR ALL FASTENERS, METAL CONNECTIONS, SCREWS, NAILS, ETC. THAT ARE IN CONTACT WITH PRESSURE TREATED LUMBER.
- 27. DIMENSIONAL LUMBER:
- A. ALL LOAD BEARING WALLS SHALL BE SOUTHERN YELLOW PINE #2 OR BETTER GRADED AND STAMPED BY THE CERTIFYING AGENCY. IN ADDITION, ALL WOOD SHALL BE PRESSURE TREATED FOR EXTERIOR USE WHERE EXPOSED TO MOISTURE, PLACED WITHIN 12 INCHES OF SOIL OR IN CONTACT WITH CONCRETE OR MASONRY.
- 28. STRUCTURAL SHEATHING: A. ALL SHEATHING USED FOR EXTERIOR APPLICATIONS SHALL BE EXTERIOR GRADE AND ADA STAMPED AND VERIFYING ITS RATING.
- A. CONCRETE MASONRY UNITS SHALL CONFORM WITH AMERICAN MASONRY INSTITUTE STANDARD 530

  B. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE
- STRENGTH OF 1900 PSI
- C. MORTAR SHALL BE OF TYPE M OR S GRAY MORTAR.
- A. ALL GROUT SHALL BE A FINE TYPE HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI UNLESS SPECIFICALLY SHOWN OTHERWISE BY A MANUFACTURER PURSUANT TO GROUT USE WITH ITS PRODUCTS.

A. ALL REINFORCING STEEL SHALL BE ASTM GRADE 40 EXCEPT GRADE 60 OMPLY WITH SHALL BE USED FOR GRADE BEAMS, ALL LINTEL TYPES USE SHOWS S, FLORIDA BUILDI IN THE STRUCTURAL PLANS.

PREVAIL NOTICE

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HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

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ENGINE

- 32. STRUCTURAL STEEL AND CONNECTION ACCESSORY MATERIAL:
- A. I-BEAMS, FORMED STRUCTURAL STEEL, FLAT BAR OR PLATE SHALL BE ASTM GRADE A36 UNLESS STATED OTHERWISE.
- B. ALL STRUCTURAL STEEL SHALL HAVE A MINIMUM OF TWO B. ALL STRUCTURAL STEEL SHALL HAVE A MINIMUM OF TWO
  COATS OF PRIMER AND TWO COATS OF EPOXY AS A
  CORROSION PREVENTIVE. THE BUILDING CONTRACTOR MAY
  VARY FROM THIS SPECIFICATION WITH THE APPROVAL OF THE
  STRUCTURAL ENGINEER IF IT CAN BE DEMONSTRATED ANOTHER
  MEANS OF CORROSION CONTROL IS EQUALLY EFFECTIVE.
  C. ALL WELDING OF STRUCTURAL STEEL SHALL BE MADE WITH
  E60/70 TYPE ELECTRODES. THE DEPTH AND LENGTH FOR THE WELD
- SHALL BE SPECIFIED IN THE STRUCTURAL DESIGN FOR THE SPECIFIC CONNECTION.
- A. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR DETERMINING VENTILATION REQUIREMENTS OF CRAWL SPACES, FLOORS AND ATTICS NOR THE MEANS AND METHODS FOR IMPLEMENTING THESE
- A. ANY RENDERING OF NOTES OF WATERPROOFING MEASURES FOR BASEMENTS OR HALF BASEMENTS SHOWN IN THESE PLANS WHERE A SPECIFIC CONSTRUCTION DETAIL IS NOT SHOWN IN THE STRUCTURAL DESIGN IS AN ARCHITECTURAL ILLUSTRATION ONLY AND IS NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- B. CRICKETS ARE ASSOCIATED WITH THE ARCHITECTURAL FINISHES AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- ARE NOT THE RESIGNABILITY OF THE STATE OF THE RESISTANT DESIGN:

  A. FIRE RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON UNDERWRITERS LABORATORY OR GYPSUM ASSOCIATION DESIGN FOR FIRE RATED FLOOR, WALL AND ROOF ASSEMBLIES.
- 36. FLOOD RESISTANT DESIGN:
- A. FLOOD RESISTANT DESIGN OF FLOOD RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DEIGN AND SHALL BE BASED ON THE REQUIREMENTS STATED IN TITLE 44 CFR SECTIONS 59 AND 60, AND ON THOSE OF THE INDIVIDUAL COMMUNITY RATING AGENCIES FOR THE GOVERNMENTAL JURISDICTION WHERE THE CONSTRUCTION IS TO BE DONE.
- B. HOWEVER, THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR IDENTIFYING AND SHOWING ON THE PLANS THE FLOOD ZONE CATEGORY, BASE FLOOD ELEVATION, AND THE FLOOR AND STORY HEIGHTS OF THE BUILDING IN RELATION TO THE BASE FLOOD ELEVATION. THIS INFORMATION IS CONSIDERED ARCHITECTURAL AND SITE RELATED AND SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT
- 37. SPECIAL CONSTRUCTION:
- 31. SPECIAL CONSTRUCTION:

  I. ALUMINUM STRUCTURAL COLUMNS.

  A. ANY ALUMINUM STRUCTURES SHOWN IN THESE PLANS SUCH AS PORCH
  AND POOL ENCLOSURES OR GUARDRAILS AND HANDRAILS ARE FOR
  ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE
  STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL
- B. WHERE THE ALUMINUM STRUCTURE ATTACHES TO THE MAIN STRUCTURE OR IS INCORPORATED IN THE MAIN STRUCTURE, SHOP DRAWINGS FOR THESE STRUCTURES SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER TO DETERMINE THEIR EFFECT ON THE MAIN STRUCTURE.
- II. SWIMMING POOLS:
- A. ANY SWIMMING POOL OR HOT TUBS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL DESIGN. III. FENCES AND RETAINING WALLS:
- A. ANY RENDERING OF FENCES, RETAINING WALLS OR EXTERIOR PLANTERS WHERE A SPECIFIC STRUCTURAL DETAIL IS NOT SHOWN FOR THEIR CONSTRUCTION ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- IV, DRIVEWAYS AND WALKWAYS:
- A. ANY DRIVEWAYS OR WALKWAYS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION PURPOSES ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

Hunters Ridge

Floor and Roof Live Loads							
Attics:	20 psf w/ storage, 10 psf w/o storage						
Habitable Attics, Bedroom:	30 psf						
All Other Rooms:	40 psf						
Garage:	40 psf						
Roofs:	20 psf						

Wind Design Data	
Ultimate Wind Speed:	145 mph
Nominal Wind Speed:	112 mph
Risk Category:	II
Wind Exposure:	В
Enclosure Classification:	Enclosed
Internal Pressure Coefficient:	0.18 +/-
Components and Cladding Design Pressures:	
Roofing Zone 1: +16.0 psf max.,	-20.7 psf min.
Roofing Zone 2: +16.0 psf max.,	-36.0 psf min.
Roofing Zone 3:	-53.2 psf min.
Roofing at Zone 2 Overhangs:	-42.1 psf min.
Roofing at Zone 3 Overhangs:	-70.9 psf min.

Stucco, Cladding, Doors & Windows: +22.6 psf max., -24.5 psf min. Zone 4: +22.6 psf max., -30.2 psf min. Zone 5:

4.00 ft. End Zone Width:

The Nominal Wind Speed was used to determine the above Component and Cladding Design Pressures.

All exterior glazed openings shall be protected from wind-borne debris as per Section 1609.1.2 of the 2014 FBC,

The site of this building is not subject to special topographic wind effects as per Section 1609.1.1.1 of the 2014 FBC.

1
2,000 psf

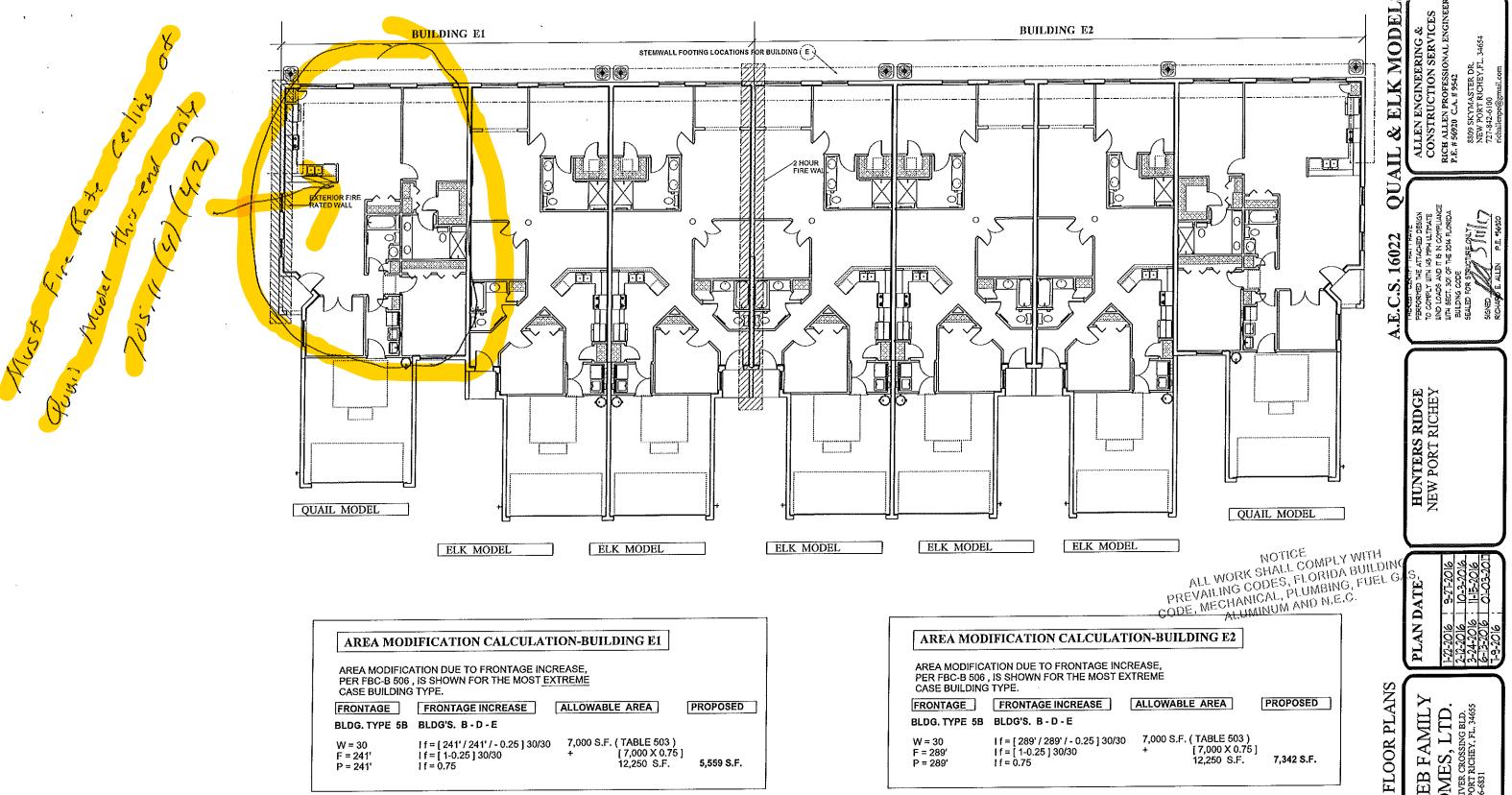
O.6 ALLOWABLE STRESS DESIGN USEDAIL NIG CODES, FLORIDA BUILD ALL WORK SHALL COMPLY WIND AND N.E.C.

WES, LTD.

"C FIRE SPRINKLER SYSTEM PER FBC 903.3
ROVIDED , DESIGNED AND ENGINEERED

AUTOMATIC FIRE SPRINKLER SYSTEM PER FBC 903.3 SHALL BE PROVIDED , DESIGNED AND ENGINEERED BY OTHERS

PLAN DATE



[ 7,000 X 0.75 ]

12,250 S.F.

5,559 S.F.

 $F = 241^{\circ}$ 

P = 241'

If = [1-0.25]30/30

1f = 0.75

FIRE SPRINKLER SYSTEM BY OTHERS. 1 HOUR SEPERATION WALL

If = [1-0.25] 30/30

lf = 0.75

F = 289'

P = 289'

OCCUPANCY CLASS - R-2

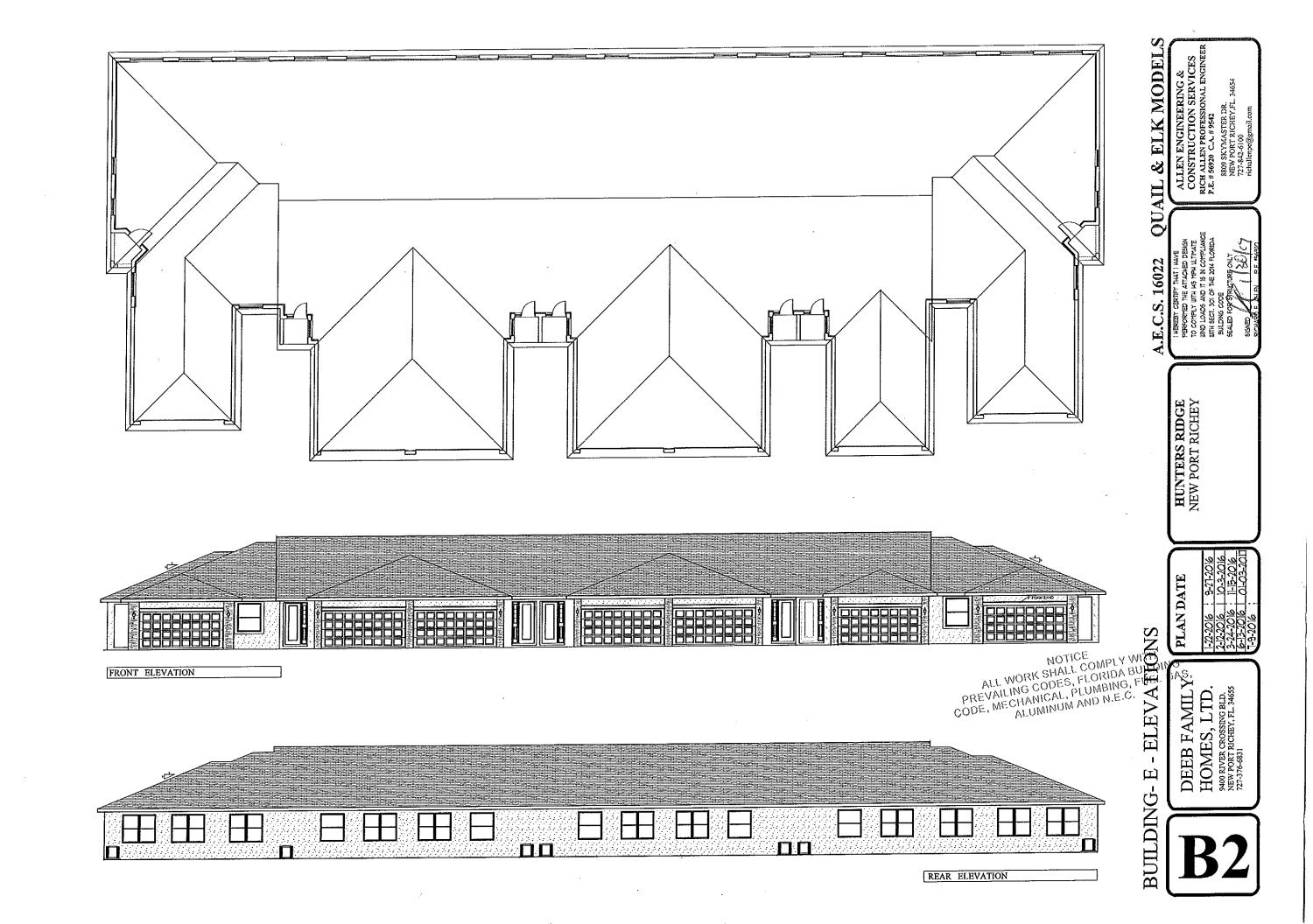
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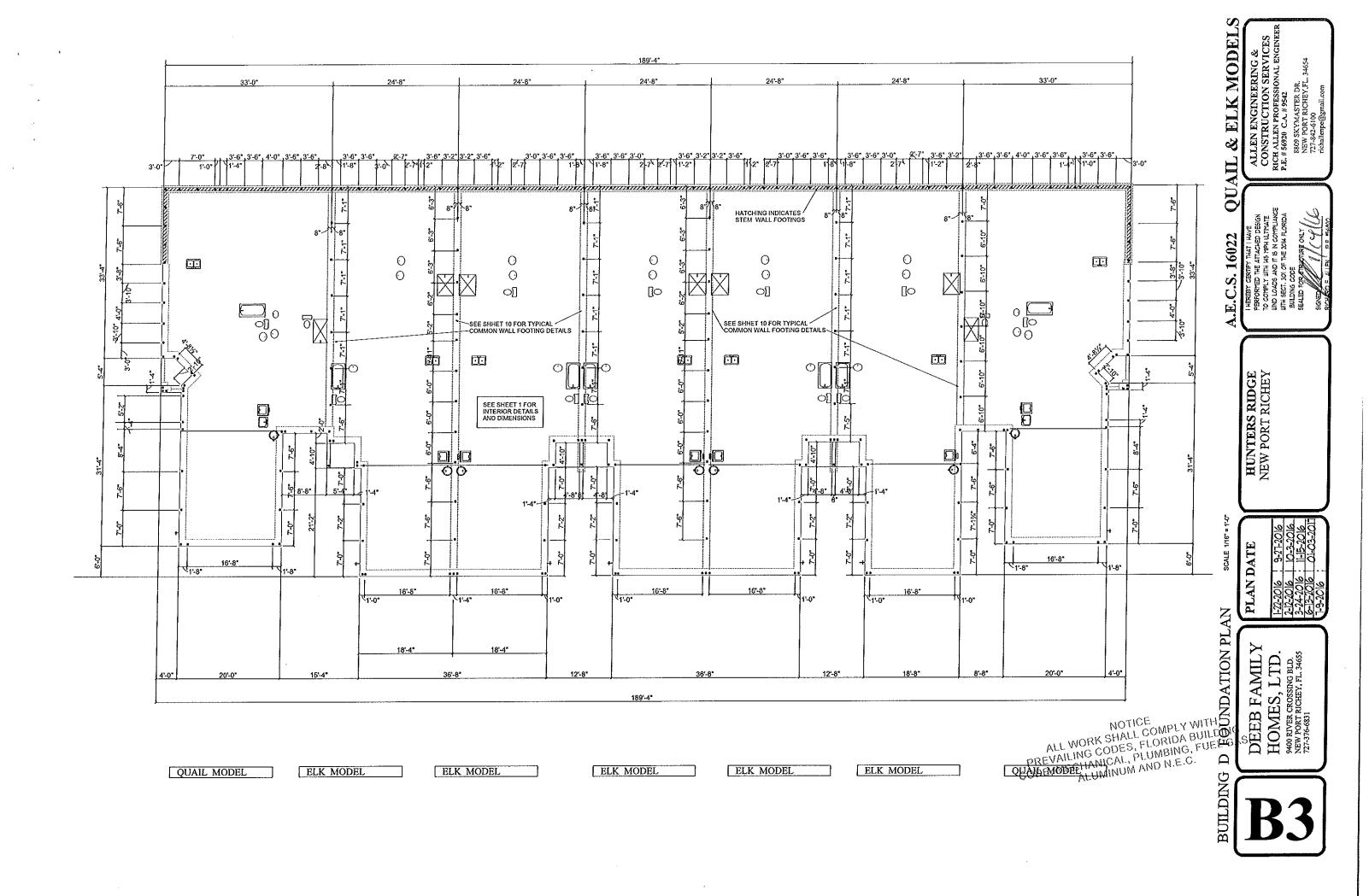
7,342 S.F.

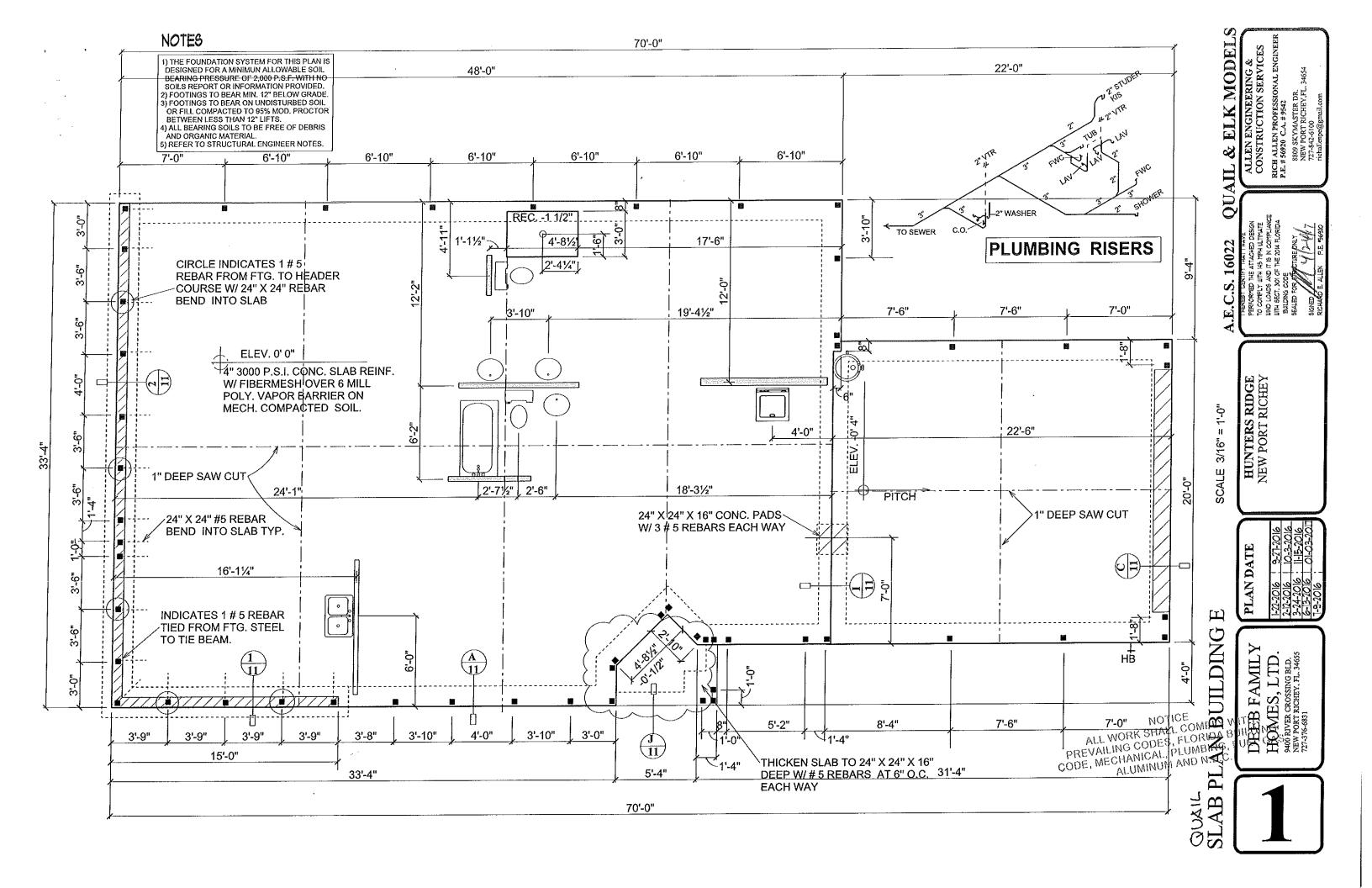
CONSTRUCTION TYPE 5B

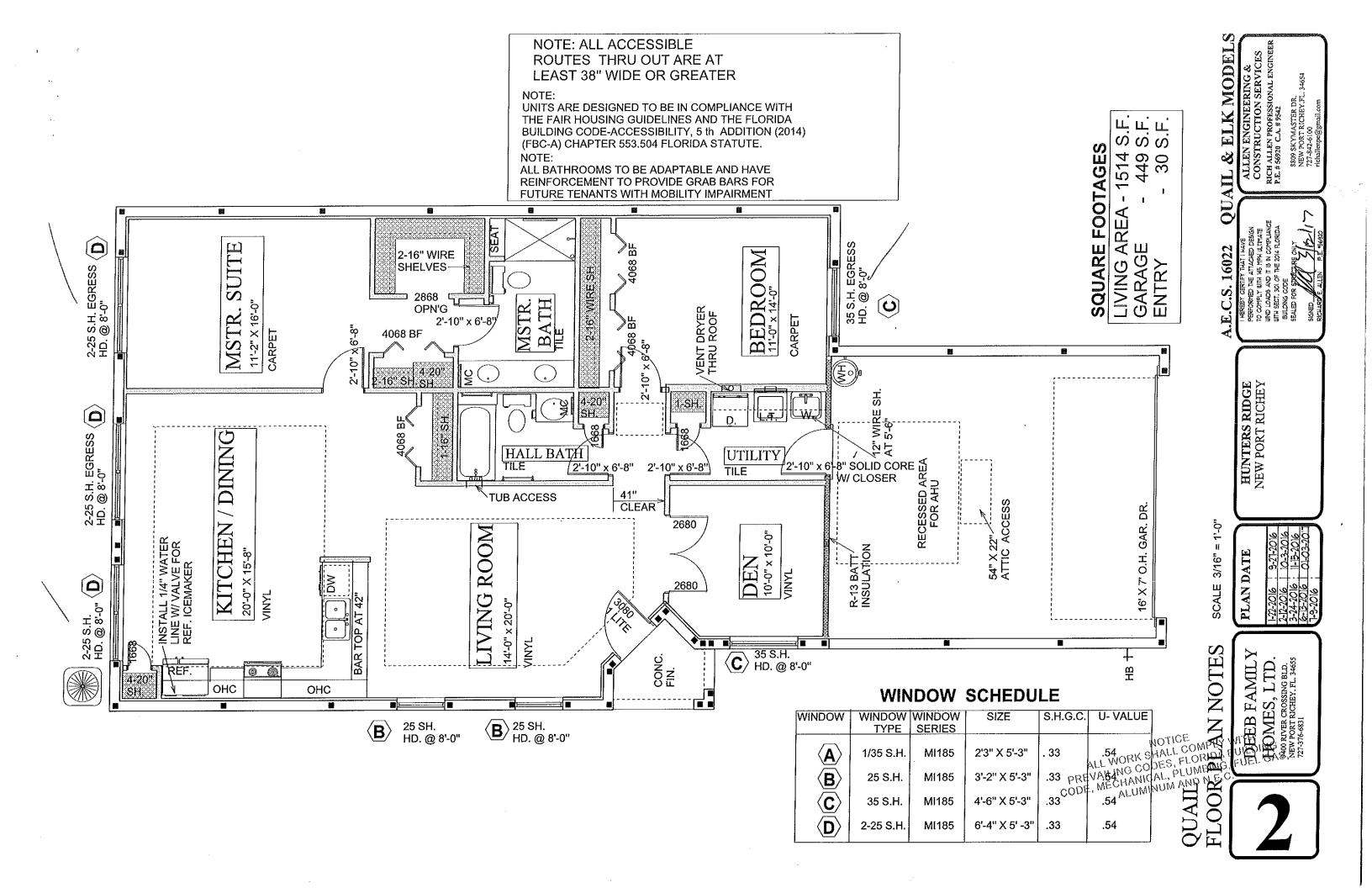
**BUILDING OCCUPANCY LOAD IS 42** (8,448 MAX SQ. FTG./200 = 42.24)

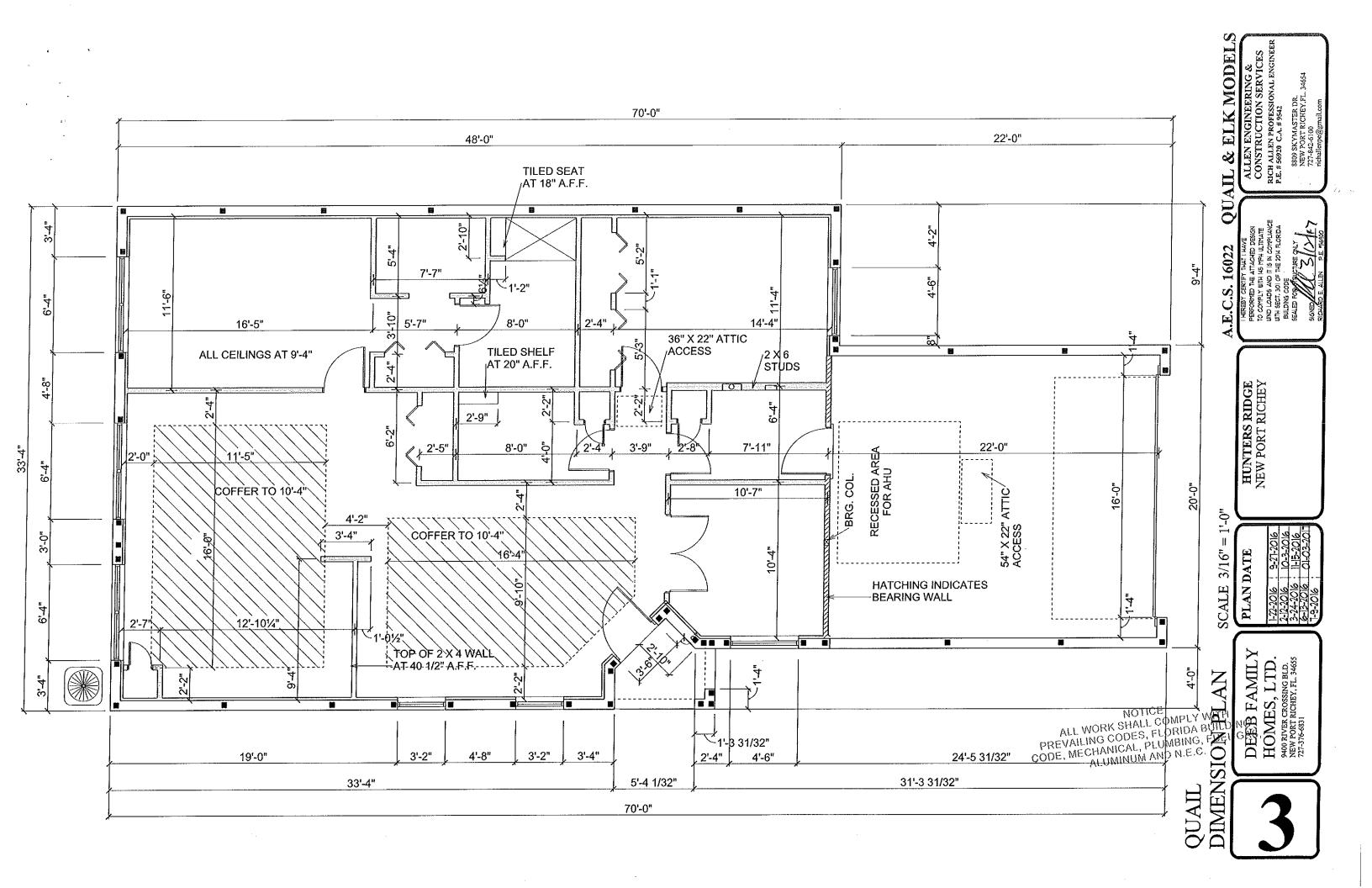
DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

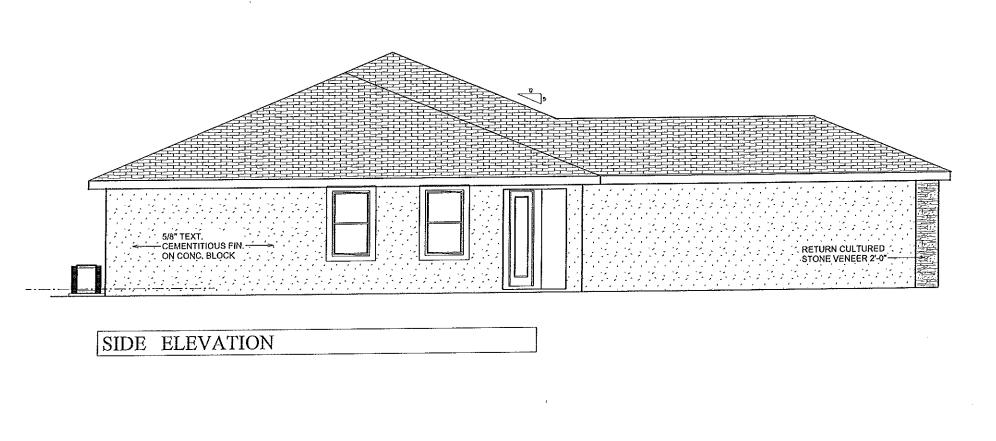


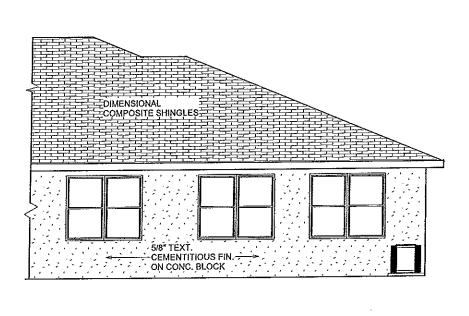




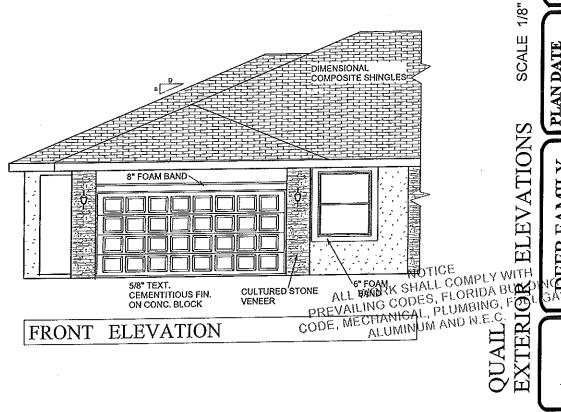








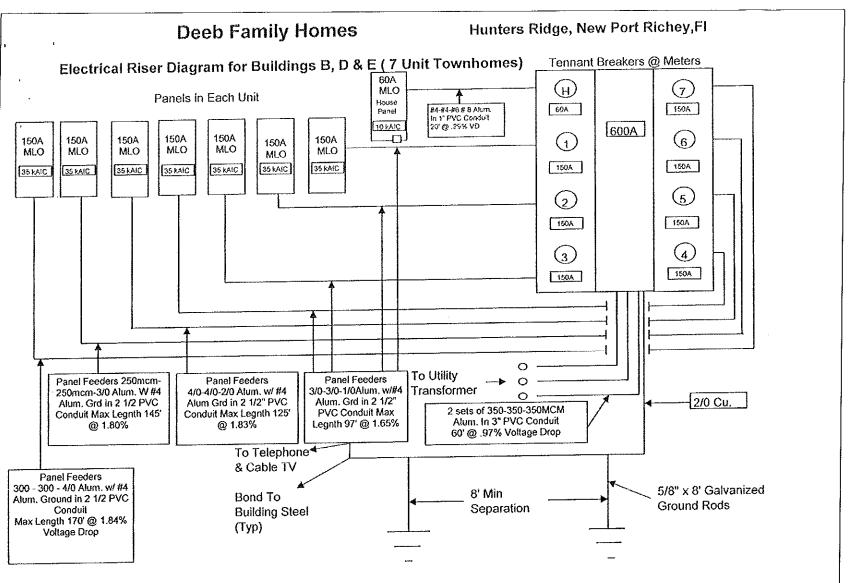
REAR ELEVATION



HUNTERS RIDGE NEW PORT RICHEY

1/8" = 1'-0"SCALE PLAN DATE

ÖÉEEB FAMILY



TYPICAL	TINU	PANE	

СКТ	1	СВ	C6	IWIRE	PH.	ASE	WIRE	СВ	CB		CKT
#	LOAD DESCRIPTION	POLES	AMPS	SIZE	Ā	В	SIZE	AMPS_	POLES	LOAD DESCRIPTION	#
<del>                                     </del>	KITCHEN APPLIANCE	1	20	#12	X		#12	20	1	WASHER	2
3	KITCHEN APPLIANCE	1	20	#12	П	X	#14	15	1	GARAGE	4
5	REFRIGERATOR	1	20	#12	X		#10	30	2	DRYER	6
7	DISPOSAL	1	20	#12	1	X		30			8
9	DISHWASHER	1	20	#12	X	Ι	#10	30	2	WATER HEATER	1 1
11	MICROWAVE	1	20	#12	T	X		30			12
13	DINING ROOM	1	20	#12	Х		#8	40	2	RANGE	14
15	BATHROOMS	1	20	#12		X		40	<u> </u>		16
17	BEORM/LIGHTING (AFI)	1	15	#14	X		#6	50 "	2_	AH-1	20
19	BEORMALIGHTING (AFI)	1	15	#14	١.,	X		60 **	2	CU-1	22
21	LIVING/LIGHTING (AFI)	1	15	#14	X	<u> </u>	#8	40 **	<u> </u>		24
23	LIVING/LIGHTING (AFI)	1_	15	#14		I.X		40 **	<del> </del>	SPARE	26
25	SPARE				Х	₩		<del>                                     </del>	├──	SPARE	28
27	SPARE			ļ.——	х	X	<b> </b>	├	<del> </del>	SPARE	30
29	SPARE			<u> </u>	<u>, ^ </u>	L		L	}		

\*NOTE; All all branch circuit wiring to meet voltage drop requirements of >2% per FBC Section C405.7.3.2

Building B, D & E		
Service Calculation		
ELK MODEL INTERIOR UNIT LOAD (CALCULATED) NUMBER OF TYPICAL UNITS	32.23 X <u>5</u> 161.15	KVA
QUAIL MODEL W/ GARAGE LOAD (CALCULATED)	33.06 X <u>2</u> 66.12	KVA KVA
SUB - TOTAL DEMAND FACTOR PER NEC	227,27 X 0.44 99,99	KVA KVA
TOTAL HOUSE LOAD @ 100%	2.4	KVA
SUB - TOTAL	102.39	KVA
@240V 1PHASE	420.0	

## HOUSE SERVICE Panel H

скт		CB					WIRE	CB	CB	LOAD DESCRIPTION	CKT #
#	LOAD DESCRIPTION	POLES	AMPS	SIZE	A_	В	SIZE	AMP\$	POLES		<del></del>
1	Fire Alarm Panel	1_1_	20	#12	×	<u> </u>	ļ	<b> </b>	<u> </u>	Spare	2
3	Spare	1	<b>1</b> .	i	L	х	L	L		Spare	4
5	Spare	1		1	×	l		<u> </u>		Spare	6
7	Spare	1	i	1	Г	Z	-	İ	<u> </u>	Spare	8
<u>'</u>	Spare	1	<u> —                                   </u>		×					Spare	1
11	Spare					x				Spare	12
	Connected Load VA			ESTIM	TEC	DEN	IAND A	#PS		FEEDER	
	PHASE A			YOLTA PHASE			240 1	) 	1	LINE CONDUCTORS - SE NUETRAL - SEE RISER	
	PHASE B									GRD CONDUCTOR - SEE CONDUIT DIA SEE RIS	
	TOTAL CONNECTED	2400									



105 Douglas Road East Oldsmar, Florida 34677-2911 813-855-6692 Fax; 813-855-4284 info@ss-electric.com

150

& ELK MODE

	Calculatio			
Project Information:	Quail Mo	ode	el w/ Gara	age
		Ri		Port Richey
Description	Qty.		Qty.	Watts
Sq. Ft. x 3 Watts	1514	x	3	4542
Small Appliance Branch		x	1500	3000
Laundry	1	X	1500	1500
Disposal	1	x	1080	1080
Dishwasher	1	X	1300	1300
Range	1	х	8000	8000
Oven		Х	9600	0
Cook Top		Х	9000	0
Jen Air		x	7680	0
Water Heater	1	х	4500	4500
Dryer	1	Х	5000	5000
Microwave	1	Х	1200	1200
Jacuzzi		x	2400	0
Pool		x	1200	0
		х	7200	0
Pool Heater		х	14400	0
Bath Fans		х	60	0
DOTT 1 CATO		x	60	0
			b Total =	30,122.00
				(10,000.00)
	5	Su	b Total =	20,122.00
			x.40%	x .40%
		Su	b Total =	8,049
				10,000.00
AC Name plate or 4 x Sq Ft				
AC # 1	1514	x	4	6056
AC # 2	1	х		0
AC # 3		x		0
AH (KW + 1000 + Fan)	<u> </u>		L	
AH # 1	4kW			5060
AH # 2	****	-		
AH#3	<del> </del>	$\vdash$		ō
ATI# 3	To	tai	   Watts =	29,165.00
	- 10		ivided by	
	To		Amps =	

www.ss-electric.com (P) 813.855.6692 - (F) 813.855.4284

\$35 Enter Co., Pr. - Econy 173, EC13035332, CAC1814111; S35 Enter Co., LLC - EC1300901; S35 Air Containing, LLC - CAC1814359

# NOTICE

Main Breaker Size

All work shall comply with prevailing codes for building, plumbing, electrical, mechanical, gas, pools and aluminum structures.

E.C.S. 16022 HUNTERS RIDGE NEW PORT RICHEY AND RISERS PLAN DATE ELECTRICAL LOAD DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

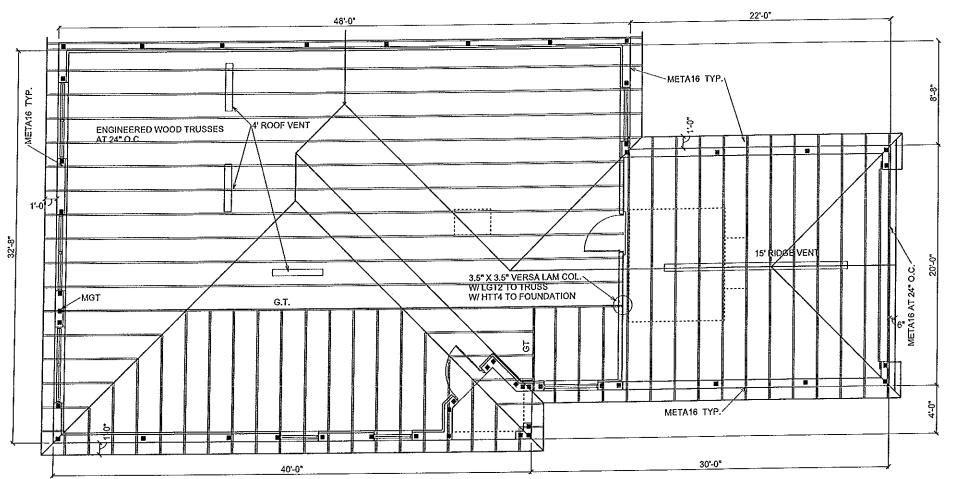
<sup>\*\*</sup> Coordinate with Mechanical shop drawings for final breaker sizes.

ALL TRUSS TO TRUSS CONNECTORS BY TRUSS SYSTEMS ENGINEER AND TO BE SPECIFIED ON INDIVIDUAL SEALED TRUSS SHEETS

> NOTE: INSTALL MOISTURE BARRIER BETWEEN MASONRY & UNTREATED WOOD

IMPORTANT NOTE:

THIS FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY, TRUSS MANUFACTURER TO PROVIDE SEPERATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER AND REVIEWED BY P.E. OF RECORD.



"O-! = "8/I SCALE

PLAN DATE

HUNTERS RIDGE NEW PORT RICHEY

ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEER P.E. # 56920 C.A. # 9542
8809 SKYMASTER DR. NEW PORT RICHEY FL. 34654
777-842-6100

& ELK MODEL

QUAIL

C.S. 16022

NOTICE

ALL WORK SHALL COMPLY WITH

PREVAILING CODES, FLORIDA BUILDING

PREVAILING CODES, PLUMBING, FUE CODE, MECHANICAL, PLUMBING, PUE CODE, PU

TOTAL NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1 TO 300 PROVIDED THAT AT LEAST 50 % AND NOT MORE THAN 80 % IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED PER SECT. R806.2

TOTAL AREA TO BE VENTILATED = 1993 S.F. 1993/300 = 6.64 S.F. OR 956.16 SQUARE INCHES.

ROOF VENTS ARE RATED AT 36 SQUARE INCHES OF OPENING PER LINEAL FT. 956.16 S.1/36 S.1. = 26.56 LINEAL FEET REQUIRED.

INSTALLATION FOR THIS ROOF IS 27' OF ROOF VENTING

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 777-376-6831 TRUSS QUAIL

All mechanical curbs, stands or other supports that require engineered anchoring must be inspected before covering.

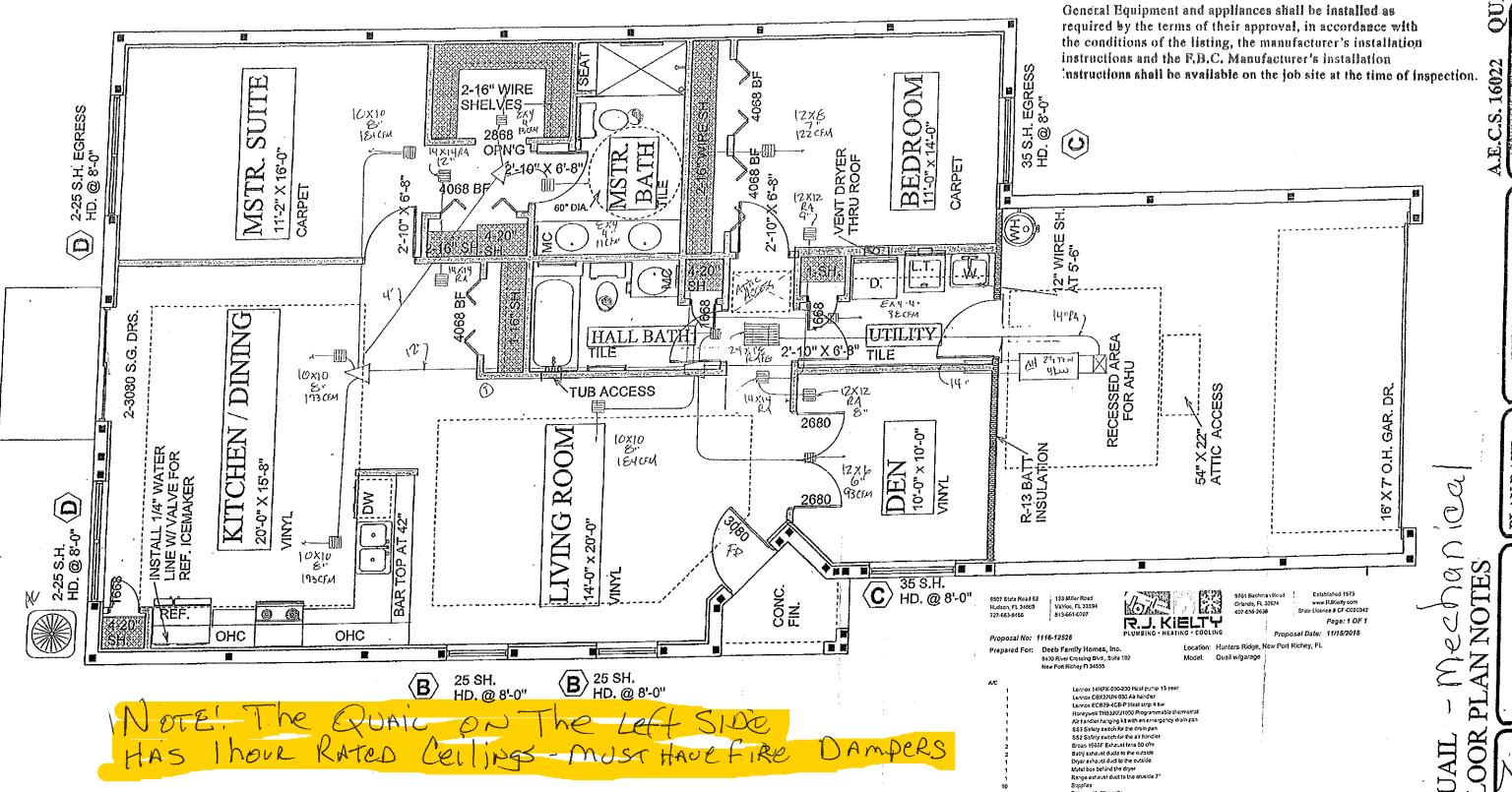
NOTICE All work shall comply with prevailing codes for building, plumbing, electrical, mechanical, gas, pools and aluminum structures.

NOTE SEPARATE PERMITS ARE REQUIRED FOR HOOD EXHAUST HOOD SUPPRESSION SPRINKLER AND COMMERCIAL GAS SYSTEMS

ANY REVISIONS TO THE APPROVED PLANS MUST BE RESUBMITTED FOR APPROVAL AND FEES PAID PRIOR TO SCHEDULING INSPECTION.

INSTALLATION

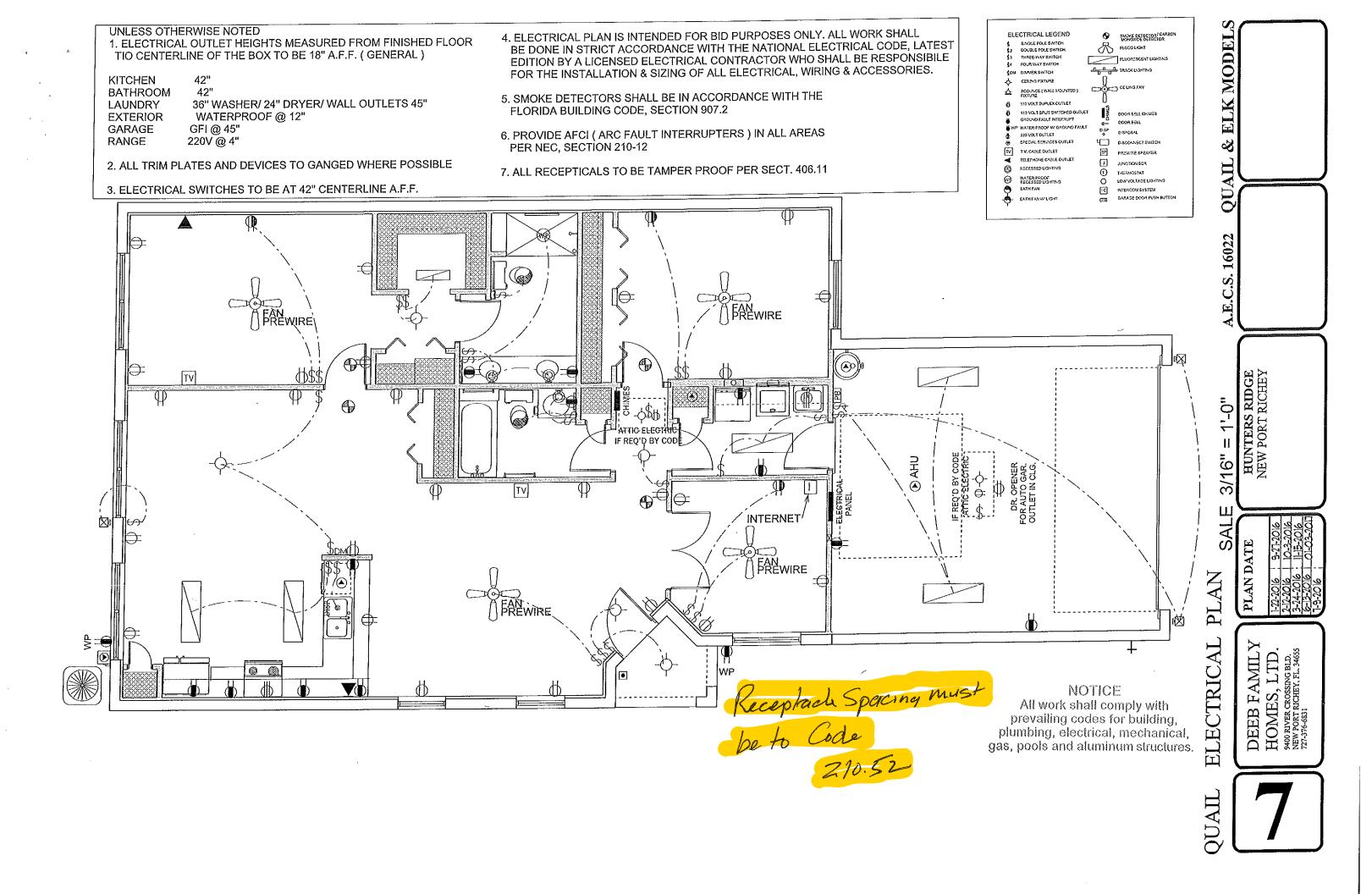
REVIEWED FOR COMPLIANCE WITH THE FLORIDA BUILDING CODE The permitted drawings shall be kept at the site of work and shall be open to inspections by the Building Official or his authorized representatives +Plass

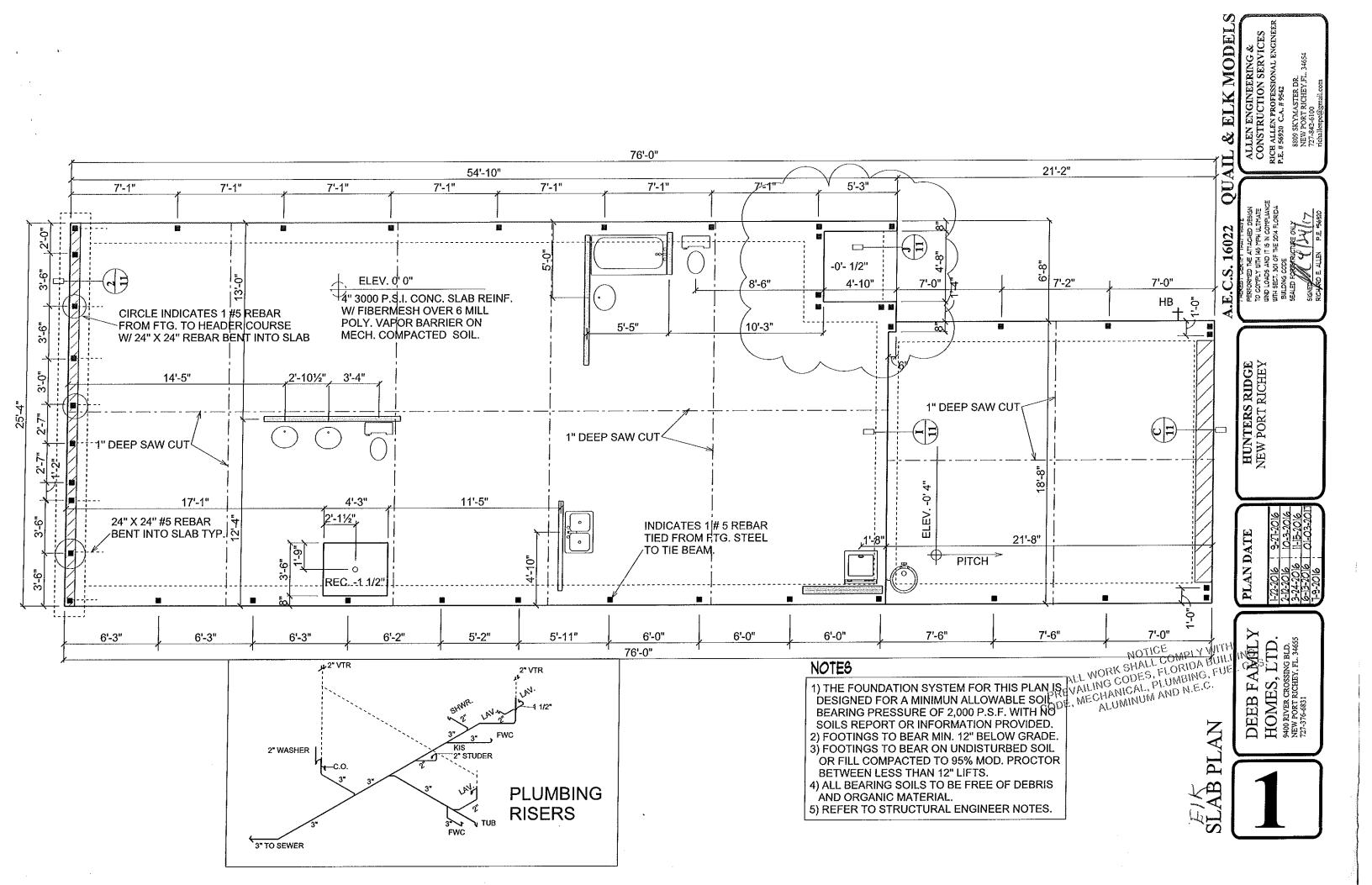


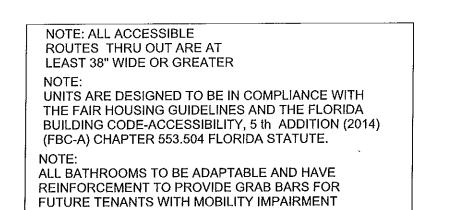
ALLEN ENGINEERING & CONSTRUCTION SERVICES

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

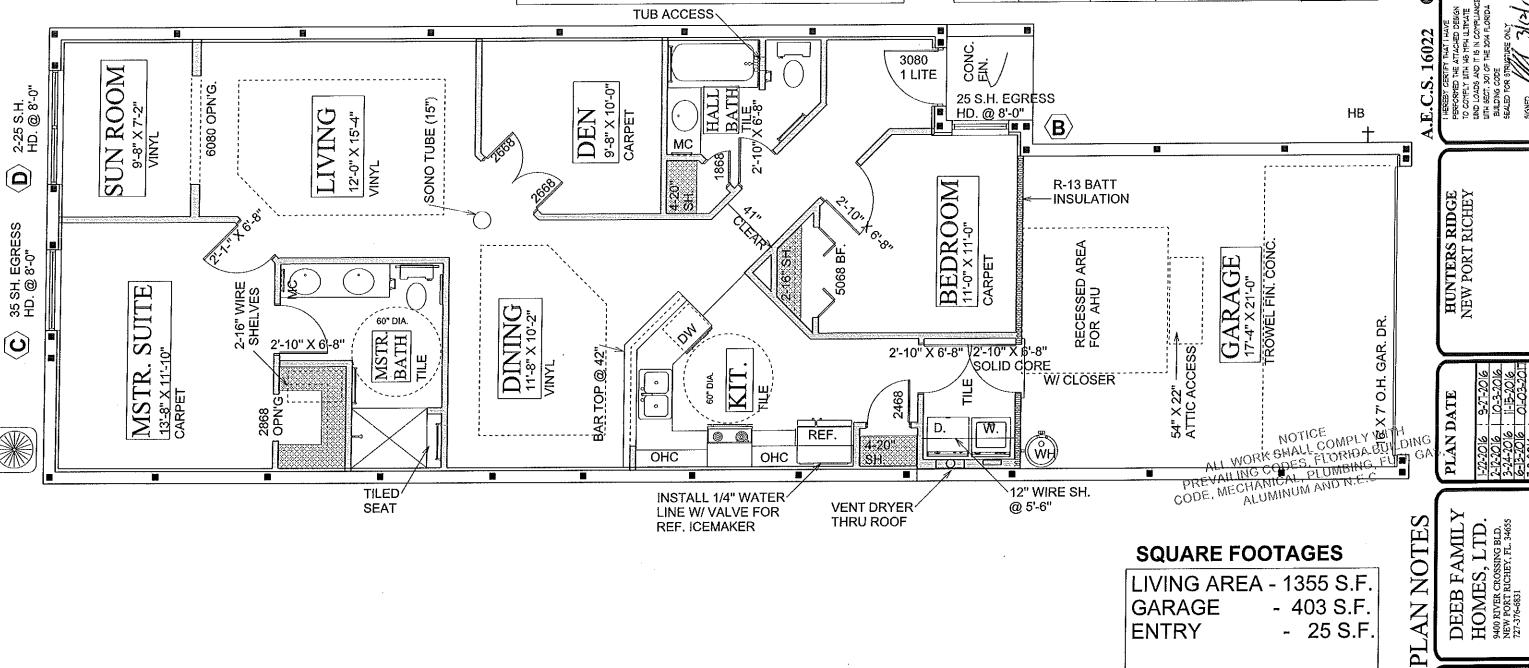






# WINDOW SCHEDULE

WINDOW	WINDOW TYPE	WINDOW SERIES	SIZE	S.H.G.C.	U- VALUE
$\langle \mathbf{A} \rangle$	1/35 S.H.	MI185	2'3" X 5'-3"	. 33	.54
$ \widetilde{\mathbf{B}}\rangle $	25 S.H.	MI185	3'-2" X 5'-3"	.33	.54
$\overline{\mathbf{c}}$	35 S.H.	MI185	4'-6" X 5'-3"	.33	.54
$ \overline{\mathbf{D}}\rangle $	2-25 S.H.	MI185	6'-4" X 5' -3"	.33	.54

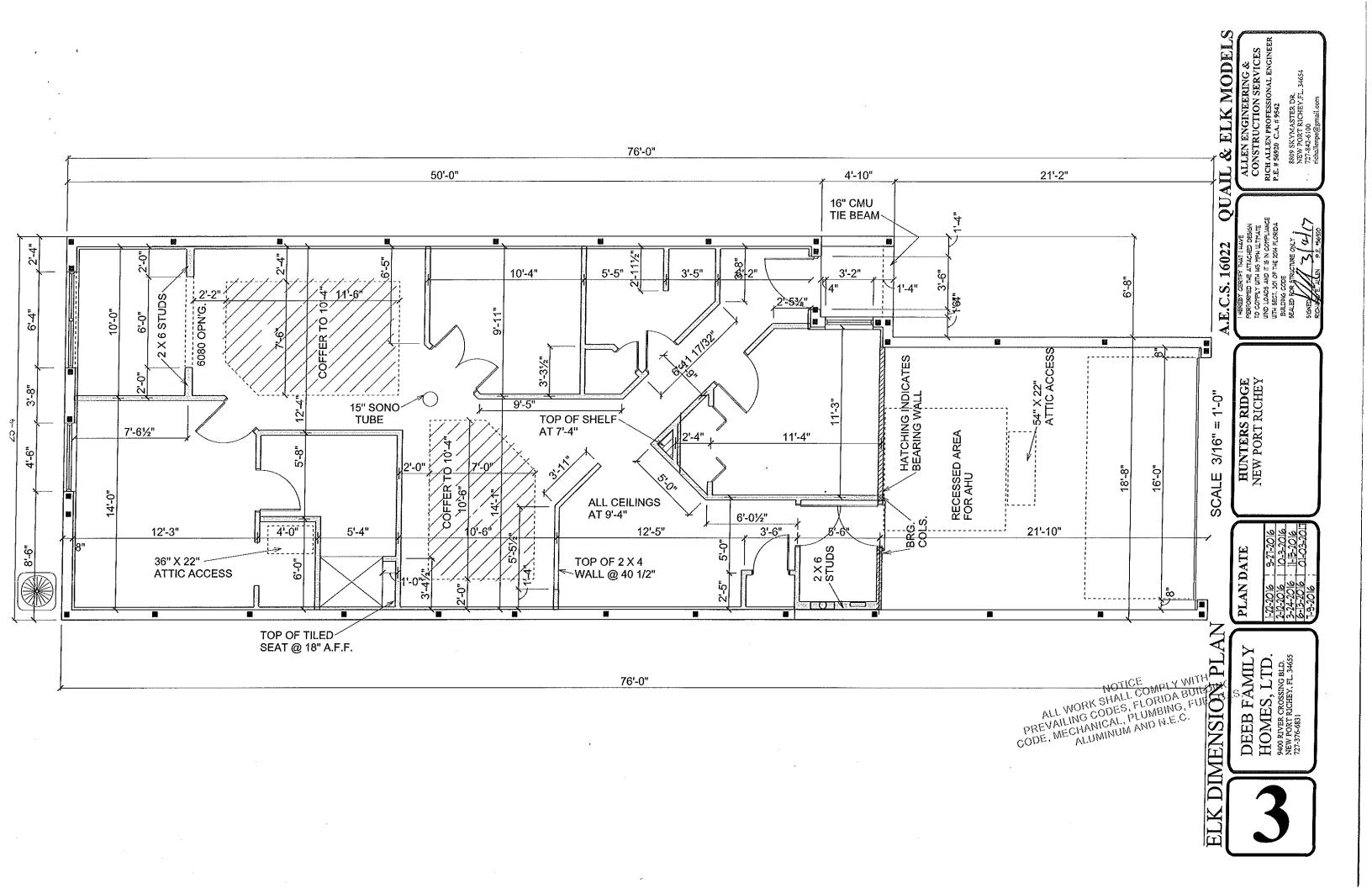


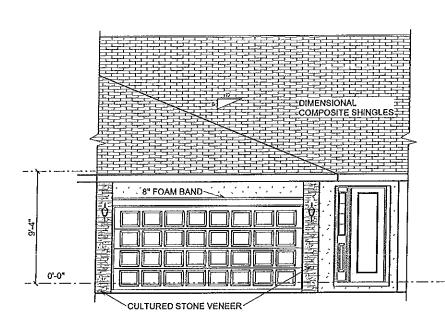
LIVING AREA - 1355 S.F. - 403 S.F. **GARAGE** - 25 S.F. **ENTRY** 

FLOOR

ELK

ALLEN ENGINEERING & CONSTRUCTION SERVICE RICH ALLEN PROFESSIONAL ENGINEE, # 85020 C.A., # 9542





FRONT ELEVATION



REAR ELEVATION

NOTICE
ALL WORK SHALL COMPLY WEN
PREVAILING CODES, FLORIDA BING, FUR
PREVAILING CODES, FLORIDA BING, FUR
ALL WORK SHALL COMPLY WEN
PREVAILING CODE, MECHANICAL, PLUMBING, FUR
ALL WINNIM AND N.E.C. ALL
ALL WORK SHALL COMPLY WEN
PREVAIL WORK SHALL COMPLY WORK SHALL COMPLY WEN
PREVAIL WORK SHALL COMPLY WORK S

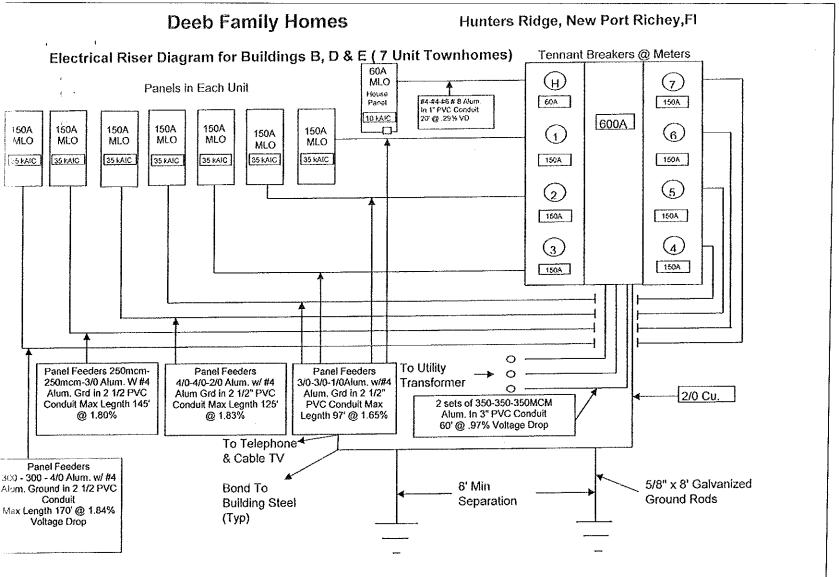
EXTERIOR

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

SCALE 1/8" = 1'-0"

DEEB; FAMILY HOMES, LTD.
9400 RIVER CROSSING BLD.
NEW PORT RICHEY, FL. 34655
727-376-6831



# TYPICAL UNIT PANEL

	CB	CB	WIRE	PH	ASE	WIRE	CB	CB		CKI
LOAD DESCRIPTION	POLES	AMPS	SIZE	Α	В	SIZE	AMPS	POLES	LOAD DESCRIPTION	#
KITCHEN APPLIANCE	1	20	#12	ŤΧ	1	#12	20	1	WASHER	2
KITCHEN APPLIANCE	1	20	#12	1	х	#14	15	1	GARAGE	4
REFRIGERATOR	1	20	#12	1 x		#10	30	2	DRYER	6
DISPOSAL	1	20	#12	1	X		30			8
DISHWASHER	1	20	#12	X	$\overline{}$	#10	30	2	WATER HEATER	1
MICROWAVE	1	20	#12	1	X		30			12
DINING ROOM	1	20	#12	X	$\overline{}$	#8	40	2	RANGE	14
BATHROOMS	1	20	#12	Τ	Х		40			16
BEDRM/LIGHTING (AFI)	1	15	#14	X		#6	50 **	2	AH-1	18
BEDRM/LIGHTING (AFI)	1	15	#14	Т	Х		50 **			20
LIVING/LIGHTING (AFI)	1	15	#14	X		#8	40 **	2	CU-I	22
LIVING/LIGHTING (AFI)	1	15	#14		X_		40 **			24
SPARE				X					SPARE	26
SPARE					Х				SPARE	28
SPARE				X					SPARE	30

E: All all branch circuit wiring to meet voltage drop requirements of >2% per FBC Section C405.7.3.2

ordinate with Mechanical shop drawings for final breaker sizes.

Building B, D & E		
Service Calculation		
ELK MODEL INTERIOR UNIT LOAD (CALCULATED) NUMBER OF TYPICAL UNITS	32.23 X <u>5</u> 161.15	KVA
QUAIL MODEL W/ GARAGE LOAD (CALCULATED)	33.06 X <u>2</u> 66.12	KVA KVA
SUB - TOTAL DEMAND FACTOR PER NEC	227.27 X <u>0.44</u> 99.99	KVA KVA
TOTAL HOUSE LOAD @ 100%	2.4	KVA
SUB - TOTAL	102.39	KVA
@240V 1PHASE	428 B	

### HOUSE SERVICE Panel H

СКТ	<del></del>	CB	CB	WIRE	PH	ASE	WIRE	CB	CB		СКТ
#	LOAD DESCRIPTION .	POLES	AMPS	SIZE	A	8	SIZE	AMPS	POLES	LOAD DESCRIPTION	#
ı	Fire Alarm Panel	1	20	#12	×	L			<u> </u>	Spare	
3	Spare	П	l	T		x	ł .		.l	Spare	4
5	Spare				×	Ι				Spare	6
7	Spare				Γ	×				Spare	8
Ø	Spare	7			x		T	1		Spare	1
11	Spare			Ι	Π	×				Spare	12
	Connected Load VA	- ""		ESTIM	ATEC	DEA	IA DKAN	MPS		<u>FEEDER</u>	
	PHASE A			VOLTA PHASE			240	) i		LINE CONDUCTORS - SE NUETRAL - SEE RISER	E RISE
	PHASE B	0		* 12-11-						GRD CONDUCTOR - SEE CONDUIT DIA SEE RIS	
	TOTAL CONNECTED	2400									



105 Douglas Road East Oldsmar, Florida 34677-2911 813-855-6692 Fax: 813-855-4284 Info@ss-electric.com

& ELK MODEL

QUAIL

A.E.C.S. 16022

	Calculatio				
Project Information	: Elk Mod	е	(Interior l	Jnit)	
Hunters Ridge, New Port Richey					
Description	Qty.	Γ	Qty.	Watts	
Sq. Ft. x 3 Watts	1355		3	4065	
Small Appliance Branch	2		1500	3000	
Laundry	1		1500	1500	
Disposal	1		1080	1080	
Dishwasher	1	х	1300	1300	
Range	1	х	8000	8000	
Oven		χ	9600	0	
Cook Top		Х	9000	0	
Jen Air		х	7680	0	
Water Heater	1	Х	4500	4500	
Dryer	1	Х	5000	5000	
Microwave	1	Х	1200	1200	
Jacuzzi		х	2400	0	
Pool		Х	1200	0	
		Х	7200	0	
Pool Heater		Х	14400	0	
Bath Fans		х	60	. 0	
		Х	60	0	
		Bul	Total =	29,645.00	
	(10,000.00)				
	19,645.00				
x .40%				x .40%	
Sub Total =				7,858	
	10,000.00				
AC Name plate or 4 x Sq Ft					
AC # 1	1355	Х	4	5420	
AC # 2		Х		0	
AC # 3		Х		0	
AH (KW + 1000 + Fan)					
AH # 1	4kW			5060	
AH # 2					
AH#3				0	
	To		Watts =	28,338.00	
	240				
	119				
	Main Br		or Clast	150	
	wan br	sa l	VEL SIZE	100	

WW.53-electric.com

NOTICE
NOT

DEEB FÄMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

AND RISERS

PLAN DATE

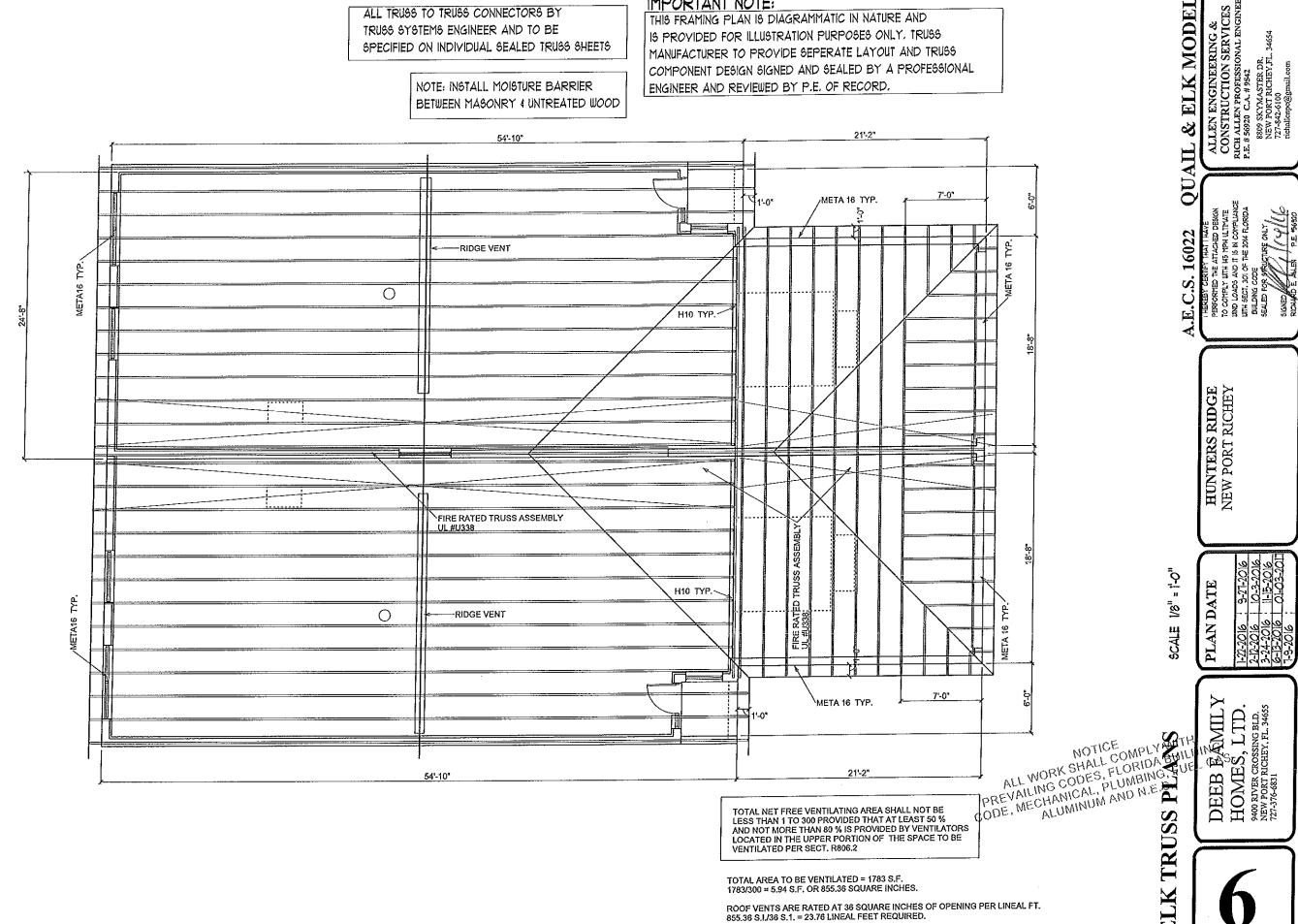
ALL TRUSS TO TRUSS CONNECTORS BY TRUSS SYSTEMS ENGINEER AND TO BE SPECIFIED ON INDIVIDUAL SEALED TRUSS SHEETS

> NOTE: INSTALL MOISTURE BARRIER BETWEEN MASONRY & UNTREATED WOOD

# IMPORTANT NOTE:

THIS FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY, TRUSS MANUFACTURER TO PROVIDE SEPERATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER AND REVIEWED BY P.E. OF RECORD.

INSTALLATION FOR THIS ROOF IS 24' OF ROOF VENTING

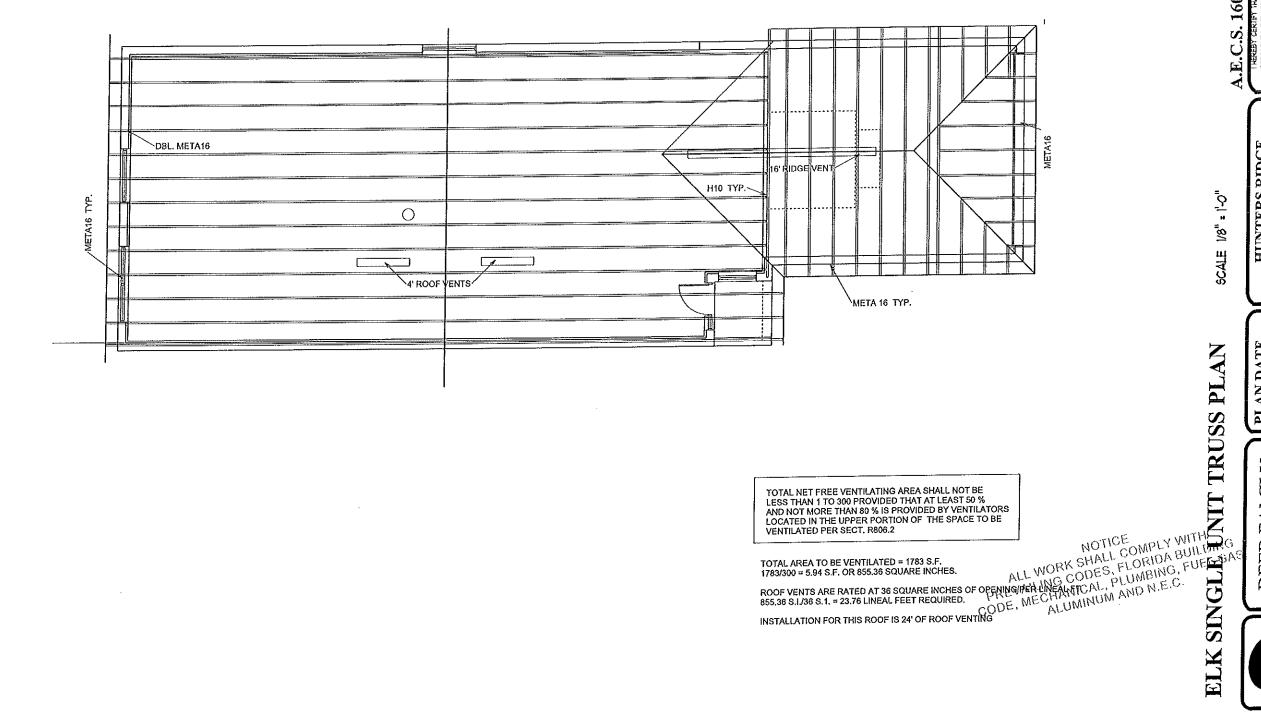


ALL TRUSS TO TRUSS CONNECTORS BY TRUSS SYSTEMS ENGINEER AND TO BE SPECIFIED ON INDIVIDUAL SEALED TRUSS SHEETS

> NOTE: INSTALL MOISTURE BARRIER BETWEEN MASONRY & UNTREATED WOOD

IMPORTANT NOTE:

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& ELK MODEL

QUAIL

HUNTERS RIDGE NEW PORT RICHEY

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

SCALE 1/8" = 1'-0"

PLAN DATE

# NOTICE

All work shall comply with prevailing codes for building, plumbing, electrical, mechanical, gas, pools and aluminum structures.

# NOTE SEPARATE PERMITS ARE REQUIRED FOR HOOD EXHAUST HOOD SUPPRESSION SPRINKLER **HEFRIGERATION** AND COMMERCIAL GAS SYSTEMS + Plans

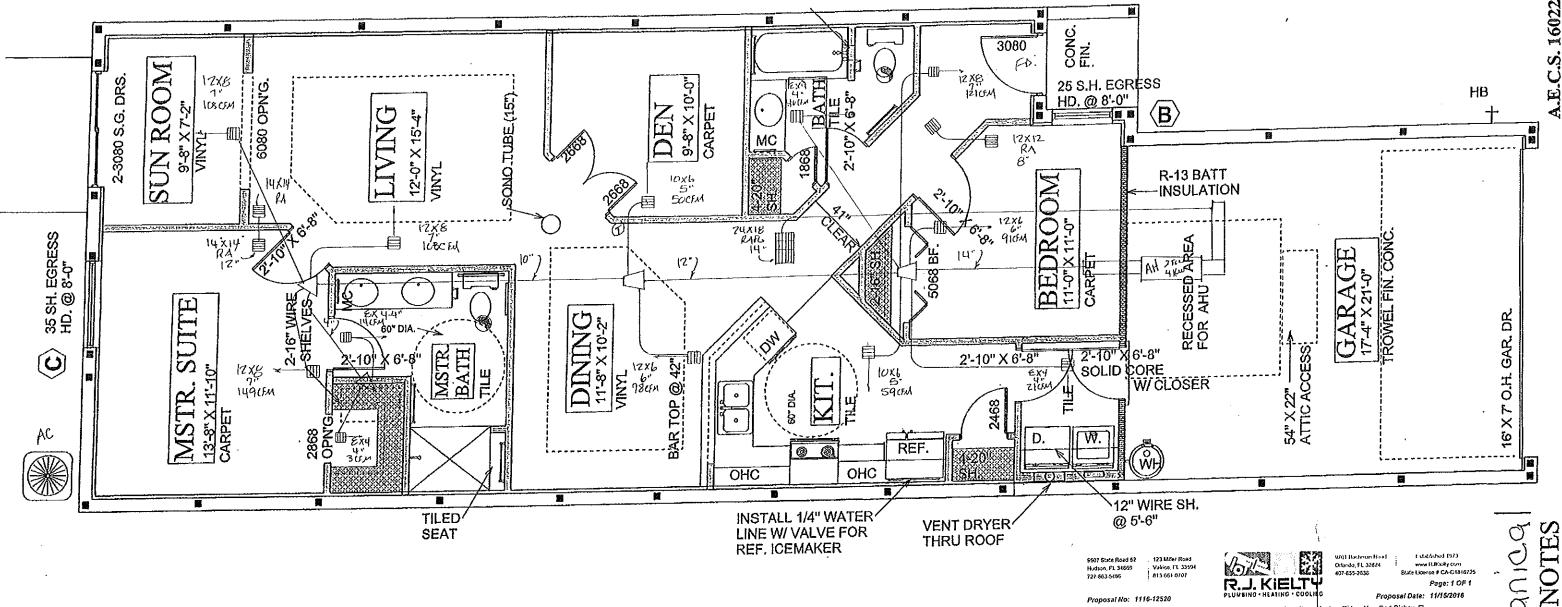
ANY REVISIONS TO THE APPROVED PLANS MUST BE RESUBMITTED FOR APPROVAL AND FEES PAID PRIOR TO SCHEDULING INSPECTION."

ELK MODEL

All mechanical curbs, stands or other supports that require engineered anchoring must be inspected before covering.

# INSTALLATION

General Equipment and appliances shall be installed as equired by the terms of their approval, in accordance with he conditions of the listing, the manufacturer's installation instructions and the F.B.C. Manufacturer's installation instructions shall be available on the job site at the time of inspection.



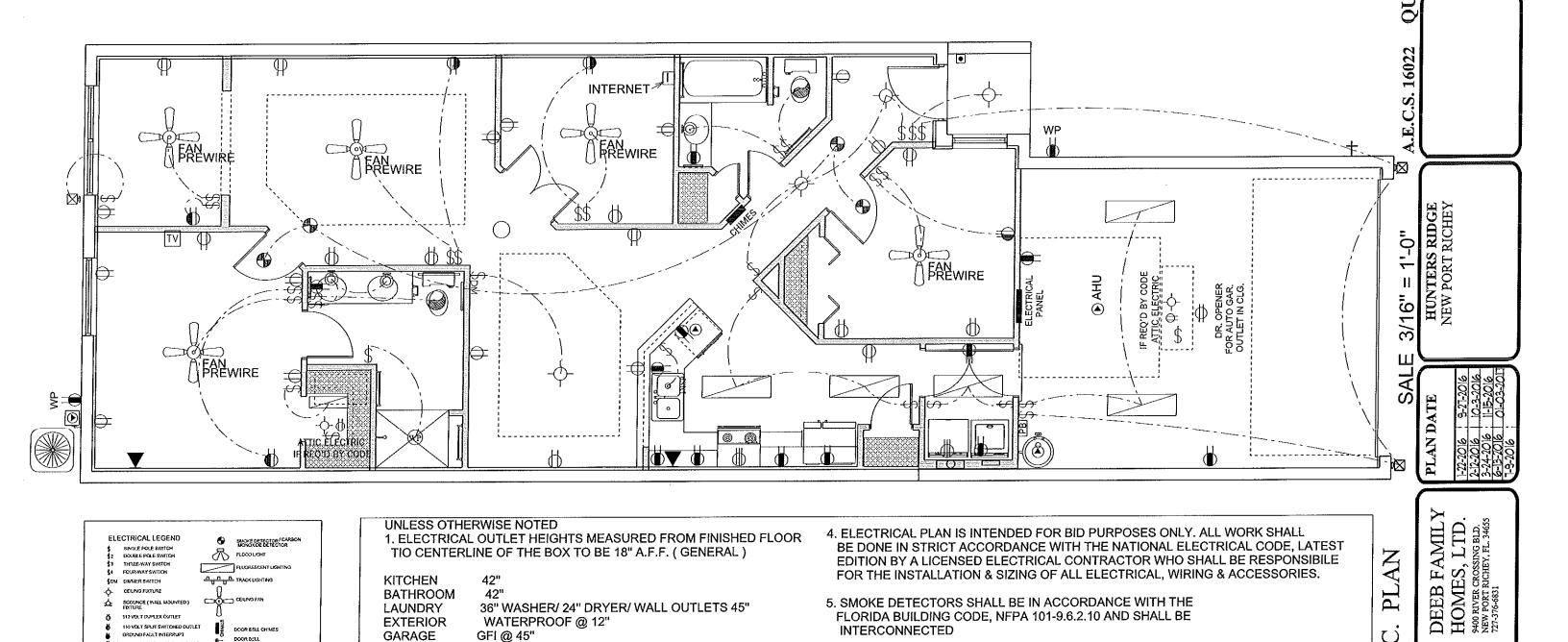
IS.J. KIELTY

Page: 1 OF 1 Proposal Date: 11/15/2016

Proposal No: 1116-12520 Prepared For: Deeb Family Homes, Inc. 940) River Crossing Blvd, Suite 10. New Port Richey IT 34655

Lennox 14189X-074-230 Heal pump 16 seen Lennox CBX27LBU 024 Av handet Lennox ECB29-4CB-P Heat ship 41w Lennox EC623-4CBP Heal ship 4 No Hanging kil with an emergency drain plan 533 Safety switch for the drain plan 533 Safety switch for the drain plan 535 Safety switch for the air handler Broan 1686F Exhaust Jans 50 cm Bath exhaust ducts to the oxisate

ELK



1. ELECTRICAL OUTLET HEIGHTS MEASURED FROM FINISHED FLOOR TIO CENTERLINE OF THE BOX TO BE 18" A.F.F. ( GENERAL )

2. ALL TRIM PLATES AND DEVICES TO GANGED WHERE POSSIBLE

3. ELECTRICAL SWITCHES TO BE AT 42" CENTERLINE A.F.F.

WATERPROOF @ 12"

36" WASHER/ 24" DRYER/ WALL OUTLETS 45"

ELECTRICAL LEGEND

SAYSLE POLE SYNTCH DOUBLE POLE SYNTCH THREE-WAY SWITCH

FOUR-WAY SWITCH

CEILING FIXTURE

220 YOUT OUTLET

TELEPHONE CASLE OUTLET

RECESSED LIGHTING

SWOKE DETECTOR / CARE
MONOX DE DETECTOR

DOOR BELL CHIMES

PREMIRE SPEAKER

LOW VOLTAGE LIGHTING

JUNCTION BOX

THERMOSTAT

DOOR BELL

KITCHEN

LAUNDRY

**EXTERIOR** 

GARAGE

RANGE

BATHROOM

42"

GFI @ 45"

220V @ 4"

忍

4. ELECTRICAL PLAN IS INTENDED FOR BID PURPOSES ONLY. ALL WORK SHALL

5. SMOKE DETECTORS SHALL BE IN ACCORDANCE WITH THE

FLORIDA BUILDING CODE, NFPA 101-9.6.2.10 AND SHALL BE

6. PROVIDE AFCI ( ARC FAULT INTERRUPTERS ) IN ALL AREAS

7. ALL RECEPTICALS TO BE TAMPER PROOF PER SECT. 406.11

INTERCONNECTED

PER NEC, SECTION 210-12

BE DONE IN STRICT ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE, LATEST

EDITION BY A LICENSED ELECTRICAL CONTRACTOR WHO SHALL BE RESPONSIBILE

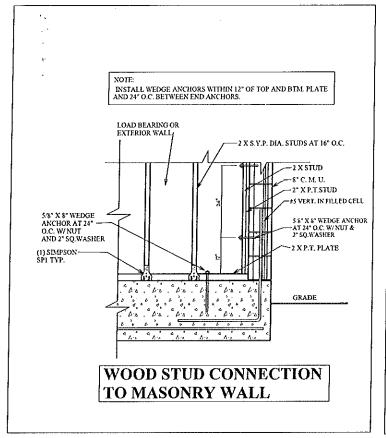
FOR THE INSTALLATION & SIZING OF ALL ELECTRICAL, WIRING & ACCESSORIES.

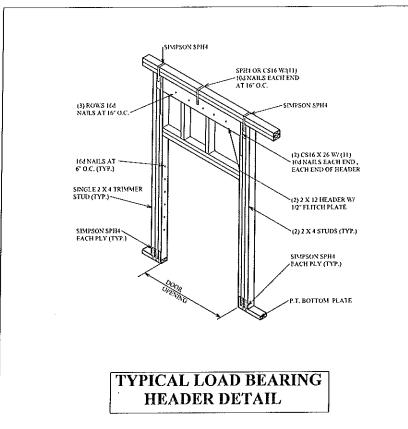
& ELK MODEL

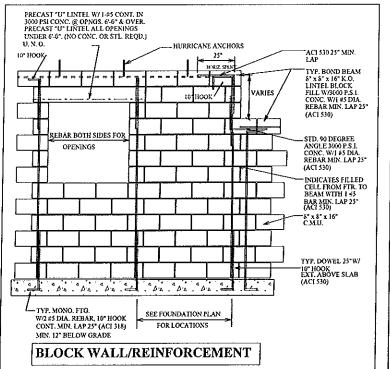
**PLAN** 

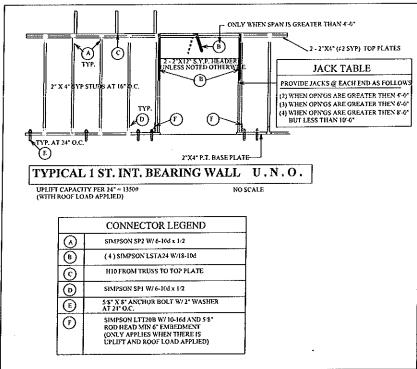
ELEC.

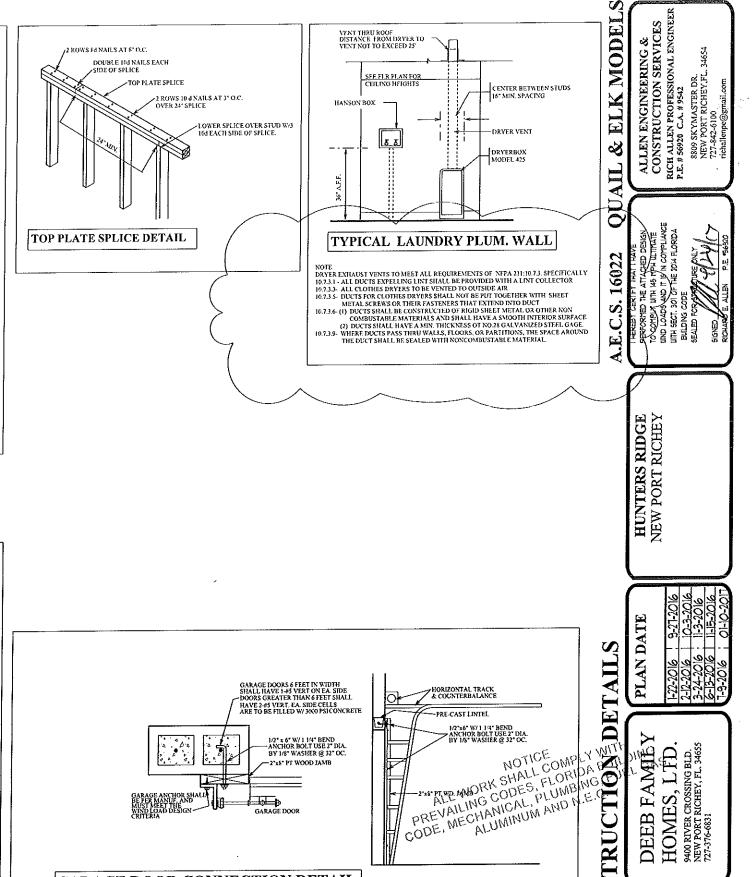
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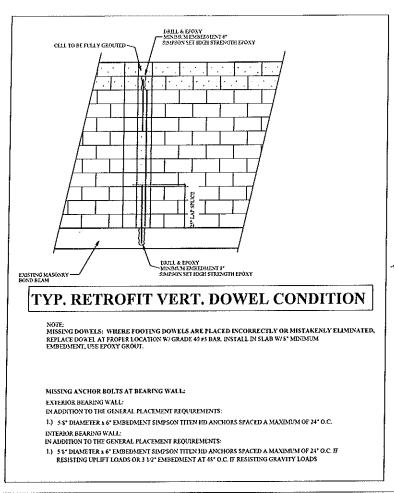


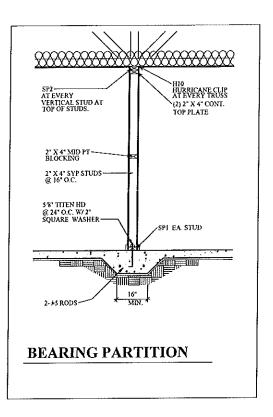


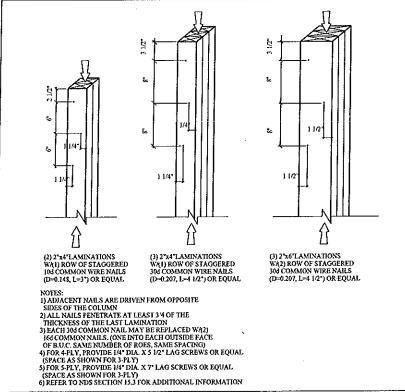


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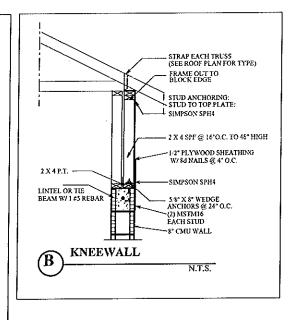
GARAGE DOOR CONNECTION DETAIL







TYP. NAILING SCHEDULE FOR BUILT-UP COLUMNS



# FIRE RESISTANCE RATINGS - ANSI/UL 263 (BXUV) **Bearing Wall Rating**

## Design No. U301

1. Nailheads - Exposed or covered with joint finisher

- 2. Joints Exposed or covered with fiber lape and joint finisher. As an alternate, nominal 3/32 in, thick gypsum veneer plaster may be applied to the entire surface of Classified veneer baseboard. Joints reinforced.
- 3. Nails 6d cement coated nails 1-7/8 in. long, 0.0915 in. shank diam, 1/4 in. diam heads, and 8d cement coated nails 2-3/8 in. long, 0.113 in. shank diam, 9/32 in. diam
- 4. Gypsum Board \* 5/8 in. thick , two layers applied either horizontally or vertically, Inner layer attached to studs with the 1-7/8 in. nails spaced 6\* o.c. Outer layer attached to study over inner layer with the 2-3/8 in, long nails spaced 8" o.c. Vertical joints located over studs. All joints in face layers staggered with joints in base layers, Joints of each

base layer offset with joints of base layer on opposite side.

When used in widths other than 48 in., gypsum board to be installed horizontally.

When Steel Framing Members' (Item 6) are used, base layer attached to furring channels with 1 in. long Type S bugle-head steel screws spaced max. 24 in. o.c.; face layer attached with 1-5/8 in, long Type S bugle-head steel screws spaced max, 12 in.

16" O.C. 16" O.C. 2x4s's FIRESTOPPED-

2 HR.

Finish Rating

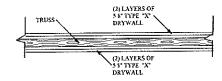
66 Min.

# **UL DESIGN U338**

# ${\bf GYPSUM\ WALLBOARD\ ,\ WOOD\ STUDS}$

BASE LAYER 5 ST TYPE X GYPSUM WALLBOARD OR GYFSUM NENEER BASE APPLIED PARALLEL OR AT RIGHT ANGLES TO EACH SIDE OF EITHER 2 X 3 OR 2 X 4 WOOD STUDS. TURNED FLATWISE, 24" O.C. WITH 64 CEMENT COATED NAILS, 17.5" LONG, 0.9315" SHANK, 14" HEADS 7" OC.FACE LAYER 58" TYPE X GYPSUM WALLBOARD OR GYPSUM VENEER BASE APPLIED PARALLEL OR AT RIGHT ANGLES TO EACH SIDE WITH 54 CEMENT COATED NAILS, 2 3 S" LONG 0.113" SHANK, 932" HEADS, 5" O.C. LOAD BEARING.

THICKNESS 41.8" APPROX. WEIGHT 12 FSF FIRE TEST UL.9-12-96



FIRE RESISTANCE RATING - 1 HOUR

# Design No. U905

March 11, 2016

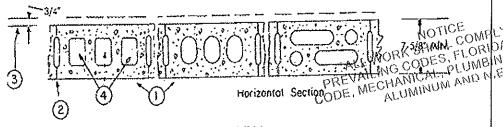
Bearing Wall Rating — 2 HR.

FIRE RESISTANCE RATING - 2 HOURS

Nonbearing Wall Rating — 2 HR

This design was evaluated using a load design method other than the Limit States Design Method (e.g., Working Stress Design Method). For jurisdictions employing the Limit States Design Method, such as Canada, a load restriction factor shall be used — See Guide BXUV or BXUV7

\* Indicates such products shall bear the UL or cUL Certification Mark for jurisdictions employing the UL or cUL Certification (such as Canada), respectively.



1. Concrete Blocks\* — Various designs. Classification D-2 (2 hr).

See Concrete Blocks category for list of eligible manufacturers.

- Mortar Blocks laid in full bed of morter, nom. 3/8 in. thick, of not less than 2-1/4 and not more than 3-1/2 parts
  of clean sharp sand to 1 part Portland cement (proportioned by volume) and not more than 50 percent hydrated lime
  (by cement volume). Vertical joints staggered.
- 3. Portland Cement Stucco or Gypsum Plaster Add 1/2 hr to classification if used. Where combustible members are framed in wall, plaster or stucco must be applied on the face opposite framing to achieve a max. Classification of 1-1/2 hr. Attached to concrete blocks (Item 1).
- 4. Loose Masonry Fill If all core spaces are filled with loose dry expanded slag, expanded day or shale (Rotary Kiln Process), water repellant vermiculite masonry fill insulation, or silicone treated perlite loose fill insulation add 2 hr to

# ELK MODEL

HUNTERS RIDGE NEW PORT RICHEY

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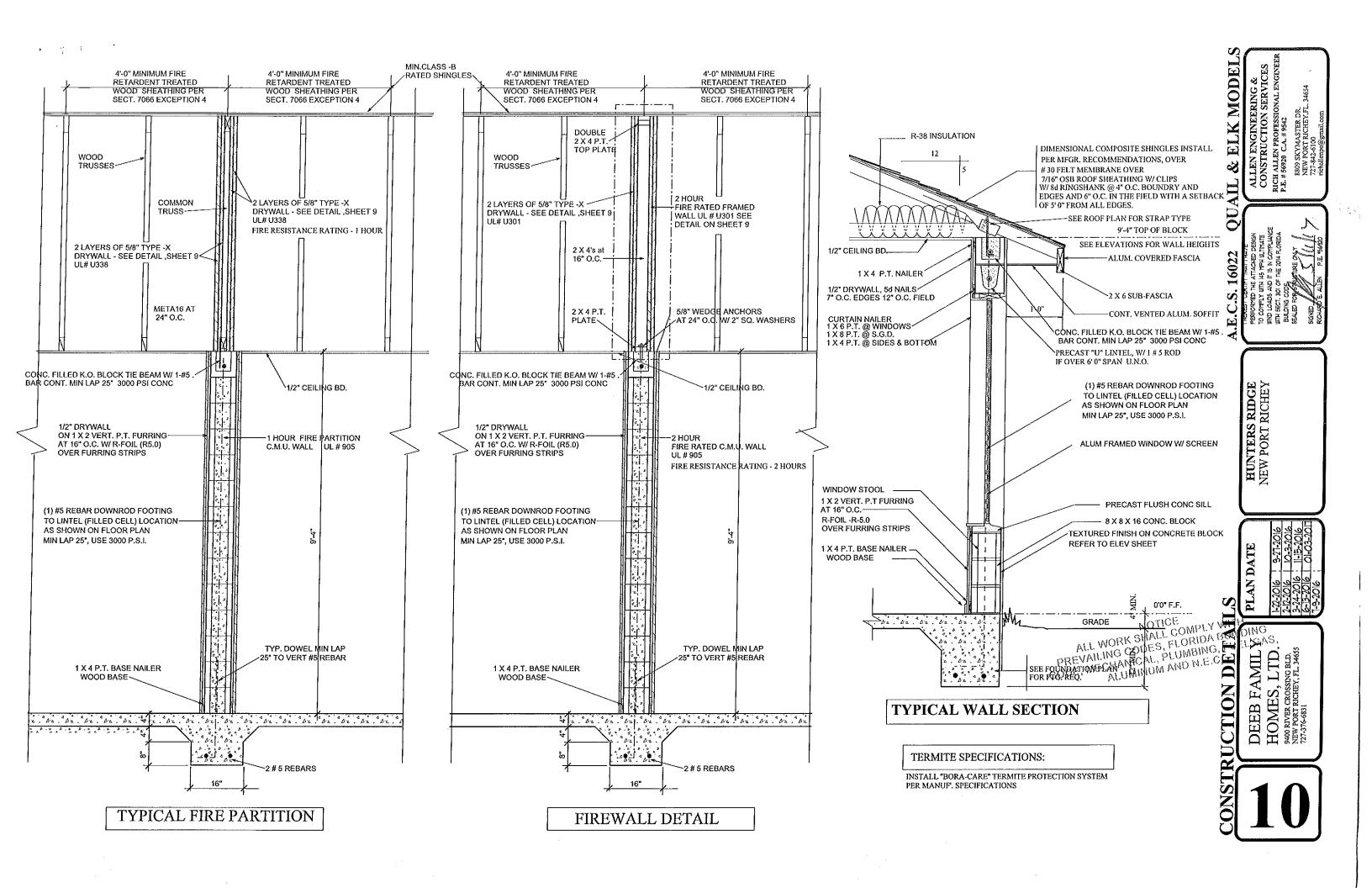
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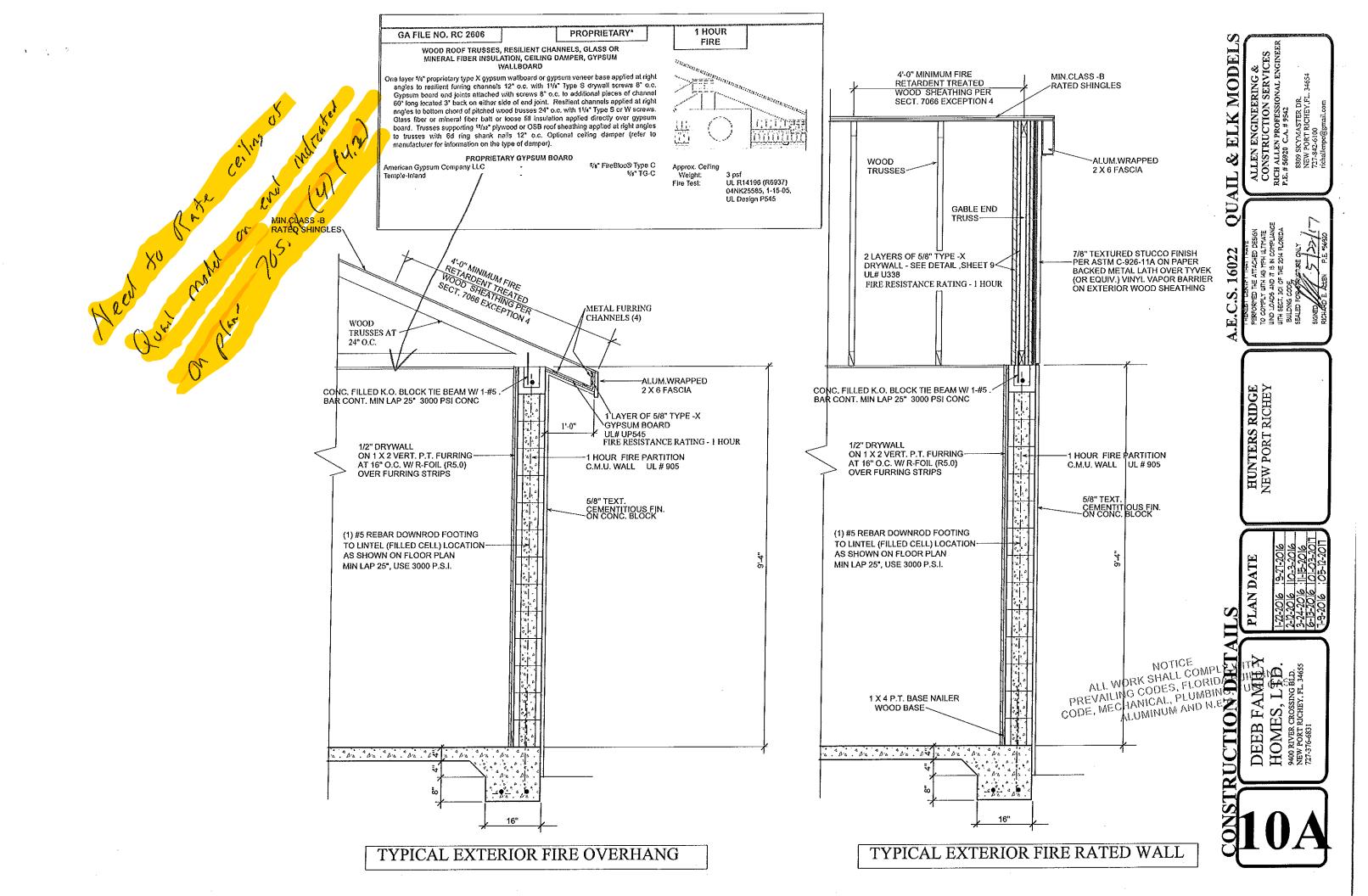
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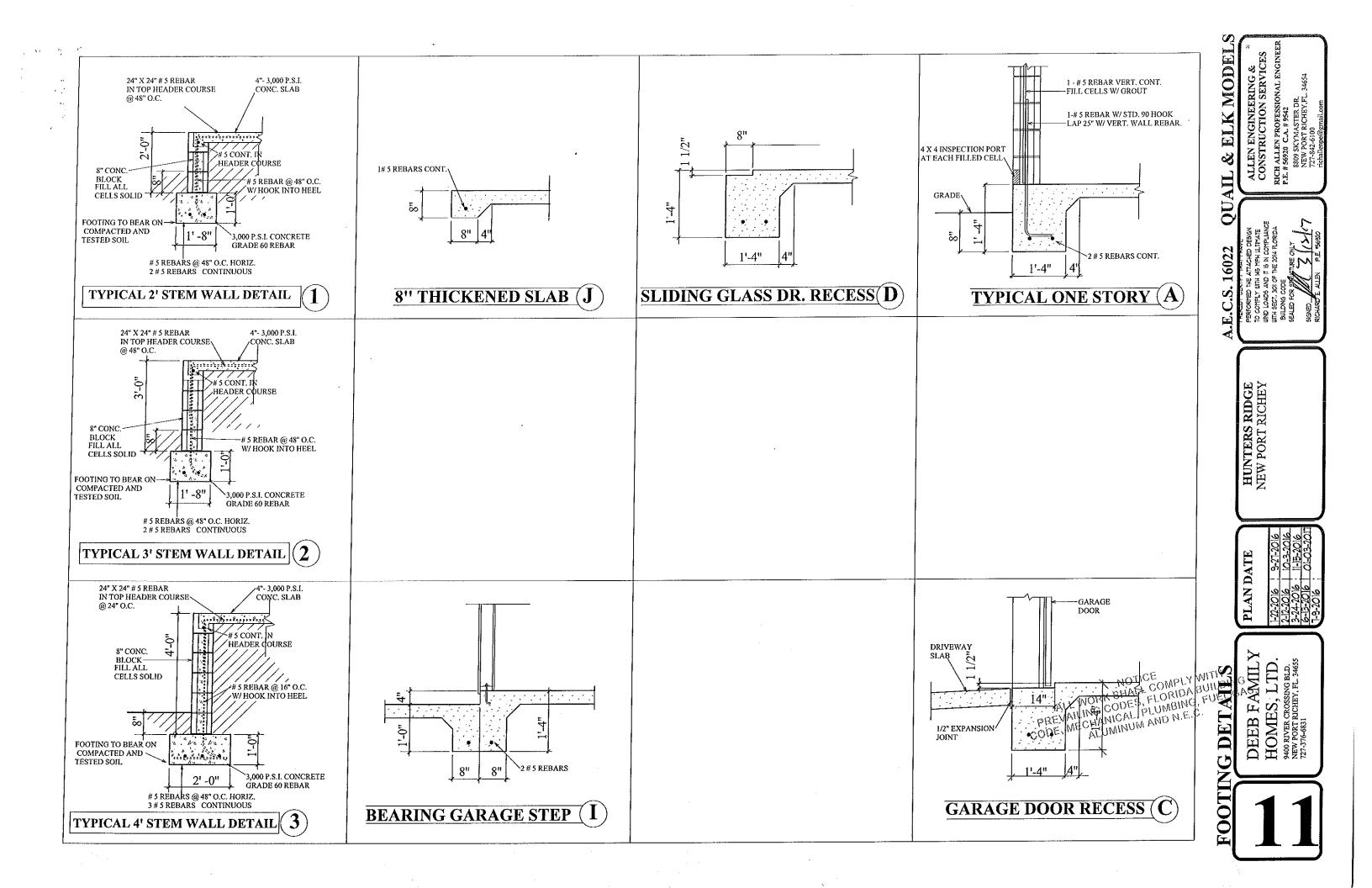
PLAN DATE

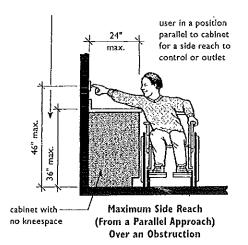
DEEB FAME Y HOMES, LTD. 940 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

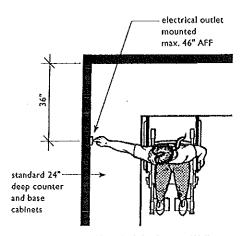




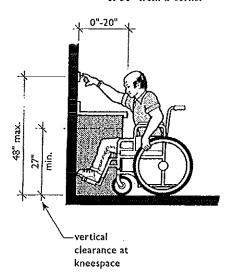


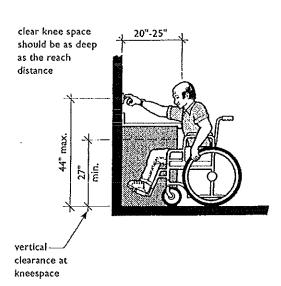


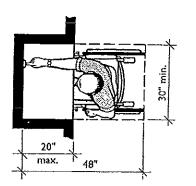


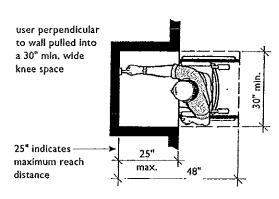


Electrical Outlets on Walls Over Cabinets Must Be a Minimum of 36" from a Corner

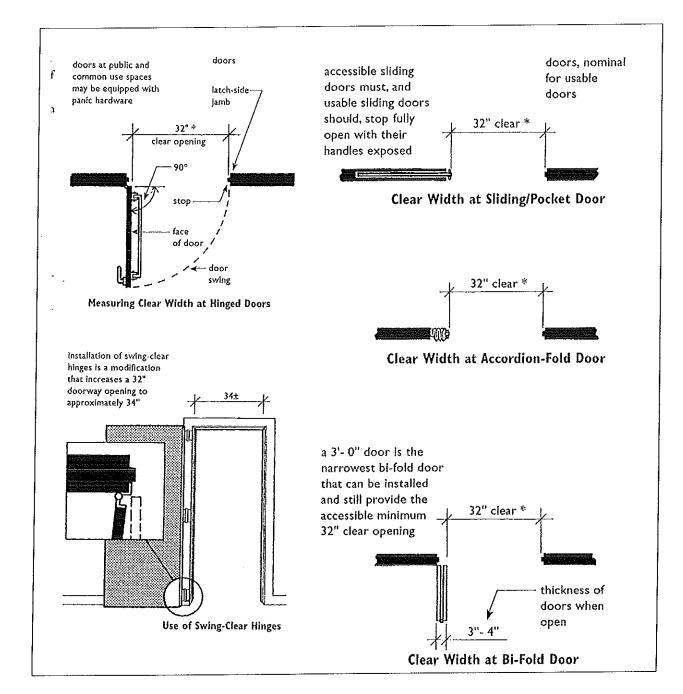








Maximum Forward Reach (From a Perpendicular Approach) over an Obstruction



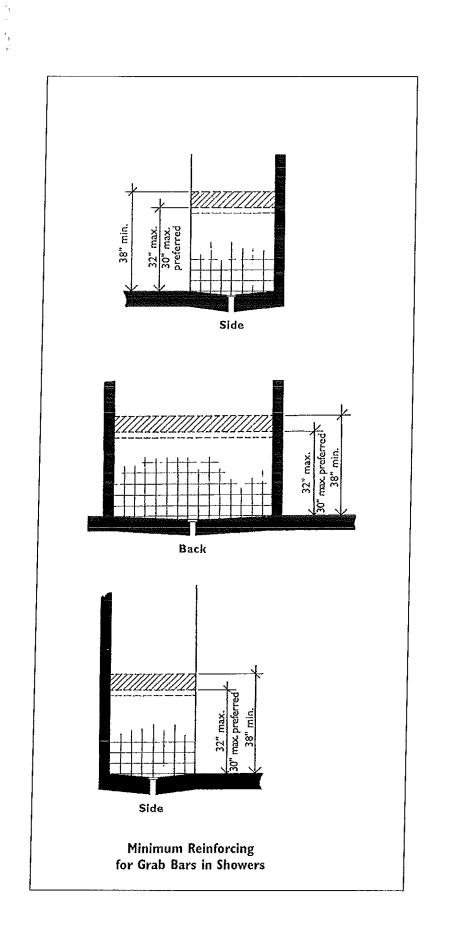
REQUIREMENTS 

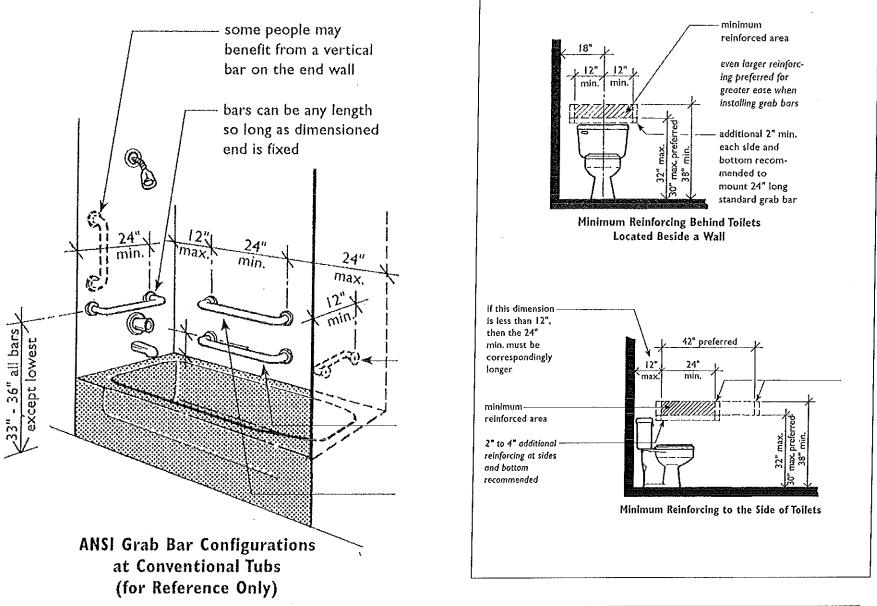
DEEB FAMILY HONES, LTD. 940 RIPECROSSING BLD. NEW POOR RICHEY, FL. 34655 727-376-6331

PLAN DATE

& ELK MODEL

E.C.S. 16022



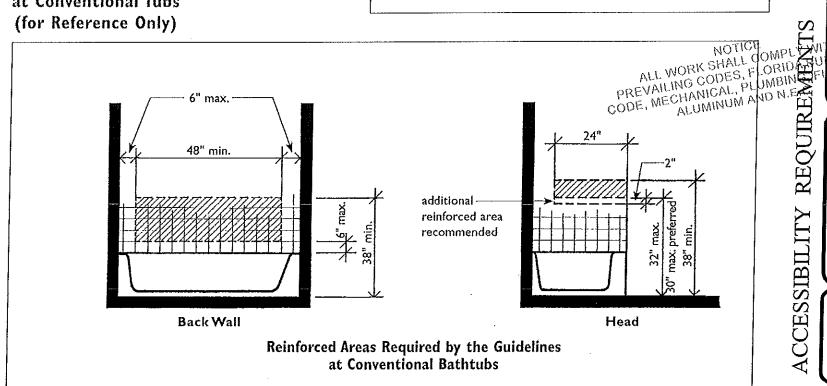


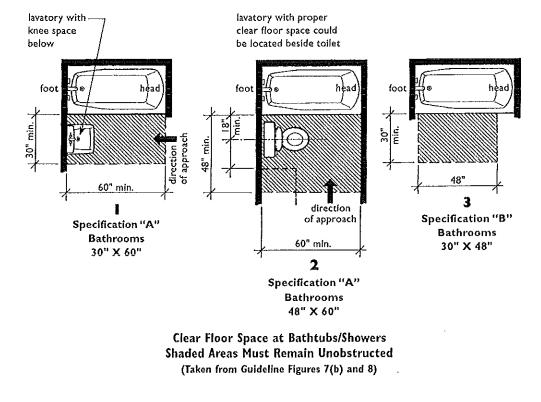
& ELK MODEI

E.C.S. 16022

DEEB FAMILY HOMES, LTD. 9400 RVER CROSSING BLD. NEW PORT RICHEY, EL. 34655

ACCESSIBILITY





. 30" min. . . 36" min.

Guideline Requirements for Clear

Floor Space at Showers

36" X 48"

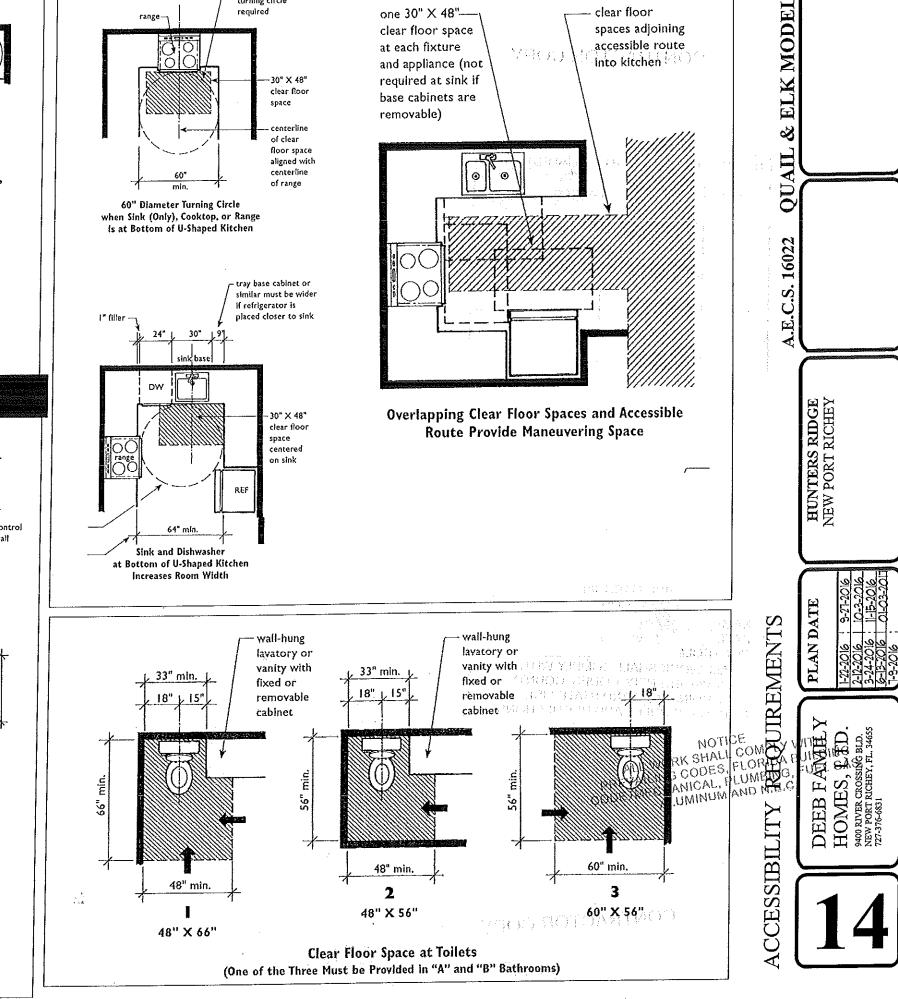
30" X 48" clear floor space flush with the control wall

30" X 48" clear floor

O space flush with the control wall

32" X 60"

Other Shower Sizes Meet the Requirements of the Guidelines



–60" diameter

turning circle

# CONTRACTOR COPY

REVIE	ved sou doi	WEGGPLIANCE
Required	Tyse in a mini contains	Approvid
	Building	RGW05 (30/17)
	Mectrical	1 5/3/17 Pos Noted Sheet 1
	Plumbing	JE 3/31/17
Market Market State Company	Mischanical	RKL4/28/17
	FireMarshall	
and the sittle territory of the con-series of sec. of		A second

PER FFPC FIFTH EDITION THANK REKETTE THE IN EVELLOS THE FOR Review and approval by the AHI shall not relieve the applicant of the responsibility of compliance with this code.

FIRE MARSHAL PASCO, COUNTY

DATE S-19-17

CONTROL#

ALL 1000 ALL WORK SHALL COMPLY WITH PREVAILING NFPA CODES, COUNTY ORDINANCES AND STATE FIRE MARSHAL'S RULES AND REGULATIONS

CONTRACTOR COPY