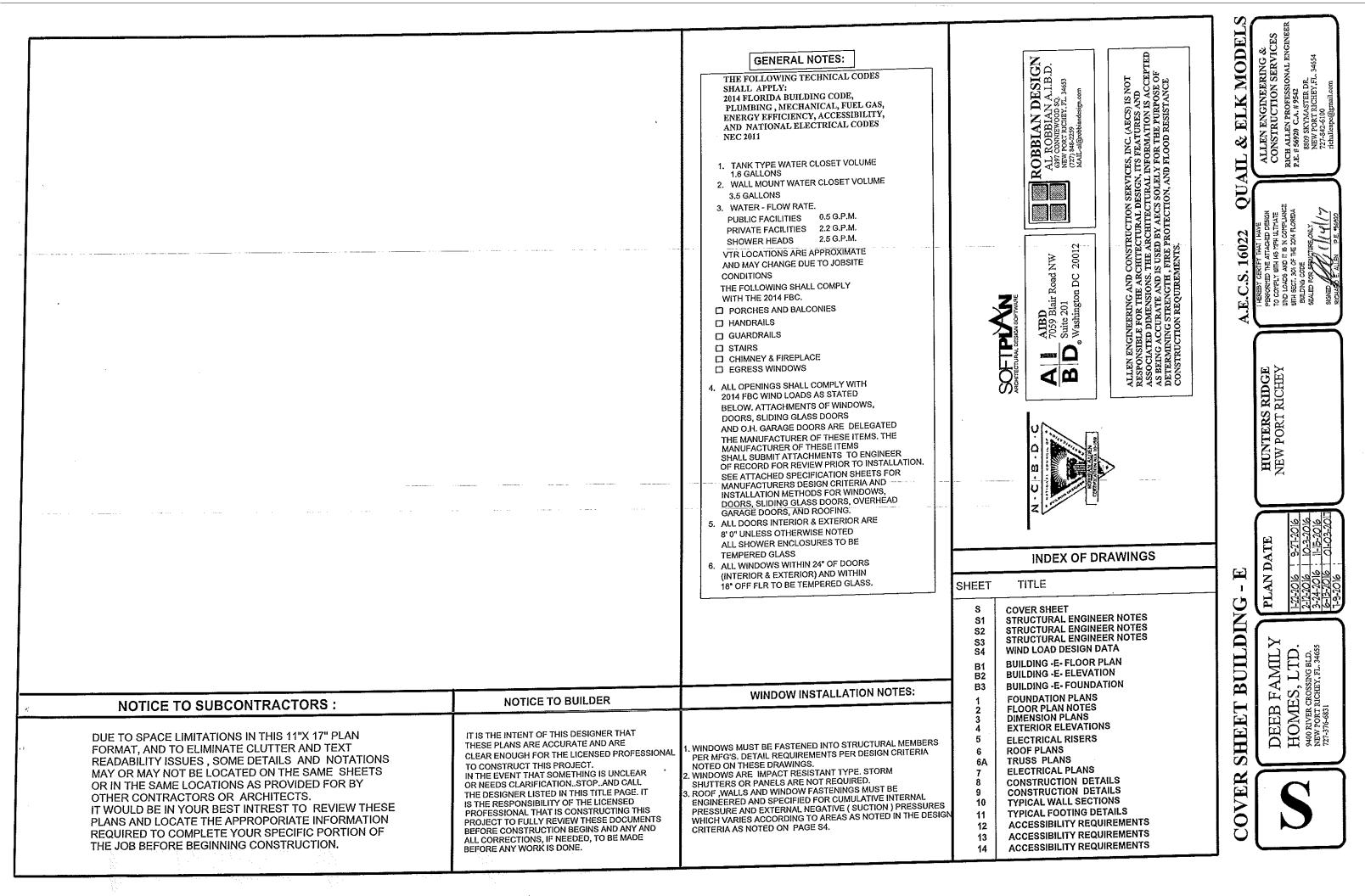
# HUNTERS RIDGE BUILDING -E



### ADMINISTRATIVE

- 1. THE ENGINEERING FIRM FOR THIS STRUCTURAL DESIGN IS ALLEN ENGINEERING AND CONSTRUCTION SERVICES, INC. HEREIN REFERRED TO AS " AECS OR " A.E.C.S ".
- 2. THE ENGINEER FOR THIS STRUCTURAL DESIGN IS RICHARD E. ALLEN, PE. HEREIN REFERRED TO AS "STRUCTURAL
- 3. THE STRUCTURAL ENGINEER DESIGN NOTES ARE PART OF THE STRUCTURAL DESIGN AND ARE TO BE TAKEN AS TYPICAL REQUIREMENTS UNLESS NOTED OTHERWISE, "UNO", IN THE STRUCTURAL PLANS AND STRUCTURAL DETAILS.
- 4. THE DESIGN SHOWN IN THESE PLANS CONFORM TO THE STRUCTURAL PROVISIONS OF THE CHAPTER 16 OF THE FLORIDA BUILDING CODE,
- 5. THE PURPOSE OF THESE PLANS IS TO OBTAIN A BUILDING 5. THE PORCUSE OF THESE PLANS IS TO SHARIN A SHEETING PERMIT AND FOR SUBSEQUENT CONSTRUCTION OF THE DESIGN AS SHOWN. THESE PLANS ARE TO BE CONSIDERED VOID IF WORK COMMENCES PRIOR TO A PERMIT BEING ISSUED, A CHANGE IN THE BUILDING CODE OCCURES PRIOR TO THE PLANS BEING SUBMITTED FOR PERMIT OR AFTER SIX MONTHS OF THE DATE THAT THESE PLANS ARE SIGNED AND SEALED WITHOUT BEING SUBMITTED FOR PERMITTING, WHICHEVER OCCURES FIRST. ONCE A BUILDING PERMIT HAS BEEN ISSUED BASED ON THESE PLANS, THE BUILDING DEPARTMENT IS NOT AUTHORIZED TO REISSUE OR TRANSFER BUILDING PERMITS WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER
- 6. CONSTRUCTION BASED ON THE STRUCTURAL DESIGN IS TO BE DONE AS SHOWN IN THE PLANS WITHOUT DEVIATION, CHANGE OR OMISSION WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. IF ADDITIONAL DETAIL INFORMATION, OR EXPLANATION IS NEEDED, IT IS TO BE OBTAINED FROM THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ADDITIONAL PARTS OF THESE PLANS, INCLUDING PROVISIONS AS STATED IN ITEM 4.
- 7. IT IS IMPORTANT TO UNDERSTAND THAT STRUCTURAL PROVISIONS OF THE BUILDING CODE ARE COMPLICATED AND THESE PLANS ARE INTENDED TO BE USED BY AN EXPERIENCED BUILDING CONTRACTOR, PROPERTY OWNERS OBTAINING OWNER-BUILDER PERMITS ARE PROCEEDING AT THEIR OWN RISK. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ERRORS OR OMISSIONS BY PROPERTY OWNERS OR THEIR AGENTS AS A RESULT OF ANY MISUNDERSTANDING OF THE PLANS THE OTHERWISE WOULD BE UNDERSTOOD BY A LICENSED CONTRACTOR. 8. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR construction means, methods, and schedule.

  9. The structural plans and any relevant design documents produced under the direct charge of the structural engineer are the property of the STRUCTURAL ENGINEER AND MAY NOR BE USED BY ANY PERSON OTHER THAN THE CONTRACTED CLIENT AND FOR ANY PURPOSE OTHER THAN THAN THAT STATED IN ITEM 5 ABOVE WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER. MOREOVER, NO OTHER ENGINEER OR ARCHITECT IS TO BE DESIGNATED A DELEGATED ENGINEER FOR ANY PURPOSE RELATED TO THESE STRUCTURAL PLANS OR CONSTRUCTION BASED ON THESE PLANS PRIOR TO THE ISSUANCE OF A CERTIFICATE OF COMPLETION OR OCCUPANCY WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

## DESIGN CRITERIA

10. LOAD COMBINATIONS: THIS DESIGN IS BASED ON AN "ALLOWABLE -STRESS" FORMULATION RELYING ON THE LOAD COMBINATIONS DEFINED IN FBC 2014 SECTION 1605.3.1 OR SECTION 1605.3.2 WHERE OMEGA EQUALS 1.3 11. FOUNDATION LOADS: SEE NOTES ON \* SITE CONDITIONS, SOILS, AND FOUNDATIONS".

13. INFORMATION CONTAINED ON A PLAN SHEET WHERE HIS SIGNATURE AND SEAL APPEAR, THAT DOES WHERE HIS SIGNATURE AND SEAL APPEAR, THAT DOES NOT PERTAIN TO THE RELEVANT STRUCTURAL PROVISIONS AS STATED IN ITEM 4, INCLUDING, BUT NOT LIMITED TO THE BUILDING OCCUPANCY, THE ARCHITECTURAL DESIGN, IT'S FEATURES, FINISHES (I.E.,DECORATIVE STUCCO, SIDING, ROOFING,SOFFITS,FLASHING, PAINTING, ETC.) AND THEIR INSTALLATION, DIMENSIONS, AND ANY DESIGN OF FIRE PROTECTION, ELECTRICAL,PLUMBING, AND MECHANICAL COMPONENTS OR SYSTEMS

COMPONENTS OR SYSTEMS.
THE ARCHITECTURAL INFORMATION, INCLUDING DIMENSIONS SHOWN IN THESE PLANS AND PROVIDED TO THE ENGINEER.

### SITE CONDITIONS

18. SITE PLAN AND TOPOGRAPHY

A. THE STRUCTURAL ENGINEER IS NOT A SUVEYOR AND IS NOT RESPONSIBLE FOR THE SITE PLAN, ESTABLISHING REQUIRED SET-BACKS, AND LOCATING THE BUILDING ON THE PROPERTY. B. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE GRADING OF THE SITE OR ITS COMPLIANCE WITH ANY DRAINAGE PLAN WHETHER INDIVIDUAL OR AS A PART OF A MASTER

C. THE FOUNDATION DESIGN IS BASED ON THESE PRESUMED CONDITIONS INCLUDING THAT DIFFERENTIAL SETTLING DOES NOT EXCEED THE SAFE LIMITS OF THE FOUNDATION DESIGN (INCLUDING STEMWALLS AND MASONRY ABOVE GRADE WALLS) AS STATED IN ITEM 19 BELOW.

D. IT IS IMPORTANT TO KNOW THAT THE FOUNDATION DESIGN BASED ON A PRESUMED ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF RELIES ON LESS THAN L/500 (E.G.,0.25 INCHES OVER 10 FEET ) OF DIFFERENTIAL SETTLEMENT. CRACKS IN MASONRY WALLS SHOULD BE EXPECTED WHERE DIFFERENTIAL SETTLEMENT EXCEEDS L/150.THIS STATEMENT SHOULD BE TAKEN AS A CAUTIONARY NOTE FOR PROCEEDING WITHOUT A SOILS ANALYSIS AND FOUNDATION RECOMMENDATION BY A GEOTECHNICAL ENGINEER FOR THE SITE.

E. COPIES OF ANY AND ALL REQUIRED COMPACTION TESTS ARE TO BE PROVIDED TO THE BUILDING DEPARTMENT FOR THEIR

# STRUCTURAL ELEMENTS

19. FOUNDATION, FOOTING AND GROUND FLOOR SLAB
A. THE FOUNDATION AND FOOTINGS ARE TO BEAR A MINIMUM
ON 12 INCHES BELOW GRADE AND ARE TO BE PLACED ON UNDISTURBED SOIL OR FILL COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR PURSUANT TO ASTM D 1557 WITH FILL LIFTS LESS THAN 12".

### COMMERCIAL

ALL LIVE LOADS PER FBC 2014 TABLE 1607.1

14. ROOF LIVE LOADS: ALL ROOF / WOOD CONSTRUCTION TYPES ARE 30 PSF.

15. DEAD LOADS: FLOOR WOOD FRAME: 35 PSF FOR TILE/MARBLE FLOOR COVERING, 15 PSF FOR ALL OTHERS.

ROOF WOOD FRAME: 25 PSF FOR SHINGLES, 35 PSF FOR TILE 16. WIND LOADS:

A. WIND LOADS ARE BASED ON THE SPECIFIC REQUIREMENTS AND DEFINITIONS OF FLORIDA BUILDING CODE 2014 EDITION ASCE-7-10.

B. THE COMPONENT AND CLADDING WIND PRESSURES ARE THE MINIMUM REQUIREMENTS FOR STRENGTH AND IMPACT PROTECTION NEEDED FOR SELECTING SATISFACTORY COMPONENTS AND CLADDING, BY OTHERS, FOR THE STRUCTURE.

ENGINEERING BY OTHERS IS PRESUMED ACCURATE AND IS RELIED UPON BY THE STRUCTURAL ENGINEER SOLEY FOR THE PURPOSE OF ACHIEVING COMPLIANCE WITH THE RELEVANT STRUCTURE

20. FOOTINGS (AND ANY ASSOCIATED MONOLITHIC FLOOR SLABS) SHALL BE CONSTRUCTED OF CONCRETE WITH A SPECIFIC COMPRESSIVE STRENGTH OF 3,000 PSI, 3 TO 5 INCH SLUMP, AND

3/8" AGGREGATE SOILS. A. IN ADDITION, THE STRUCTURAL ENGINEER IS NOT A CIVIL OR GEOTECHNICAL ENGINEER AND IS NOT RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SITE FOR CONSTRUCTION, INCLUDING ITS TOPOGRAPHY, DRAINAGE AND SUB-SURFACE CONDITIONS (INCLUDING WATER TABLE DEPTH) AND FOR INTERPRETING GEOTECHNICAL DATA CONCERNING THE SITE. B. IF SOIL CONDITIONS AT THE SITE APPEAR QUESTIONABLE AS DETERMINED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER, A SOILS ANALYSIS SHALL BE PERFORMED BY A LICENSED GEOTECHNICAL ENGINEER THAT WILL GIVE SPECIFIC RECOMMENDATIONS FOR A FOUNDATION TYPE. IF THE BUILDING CONTRACTOR OR OWNER-BUILDER DO NOT MAKE THAT DETERMINATION AND A SOILS ANALYSIS IS NOT PERFORMED, THE STRUCTURAL ENGINEER SHALL PROCEED WITH THE DESIGN BASED ON THE PRESUMPTIONS ALLOWED BY THE FBC 2012, SEC. 1804. BASED ON THE PRESUMPTIONS ALLOWED BY THE FBC 2012, SEC. 1804.

C. THE DETERMINATIONS OF THE SUITABILITY OF THE SITE FOR CONSTRUCTION (INCLUDING TOPOGRAPHICAL INFORMATION) AND THE SOIL CONDITIONS SHALL HAVE BEEN COMPLETED AND ANY RECOMMENDATIONS RESULTING FROM THAT ANALYSIS SHALL HAVE BEEN PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO THE SIGNING AND SEALING OF THE STRUCTURAL PLANS.

D. IN THE ABSENCE OF GEOTECHNICAL INFORMATION, THE SITE IS PRESUMED TO HAVE AN ALLOWABLE SOIL BEARING CAPACITY OF 2000 PSF AND THE TOPOGRAPHY AS IT RELATES TO THE STRUCTURE IS PRESUMED TO BE THAT SHOWN IN THE PLANS.

E. THE SIZE AND REQUIRED REINFORCEMENT FOR THE FOOTINGS ARE SHOWN ON THE FOUNDATION PLAN. ARE SHOWN ON THE FOUNDATION PLAN. THE GROUND FLOOR SLAB SHALL BE PLACED OVER A 6 MIL POLYETHYLENE MOISTURE RETARDER.

I. THE TRUSS SYSTEM DESIGN PROVIDED IN THIS PLAN IS FOR THE USE OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL ROOF TRUSS SYSTEM DESIGN. IT IS NOT TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL DESIGN. II. MANUFACTURED FLOOR TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.

III. THE MANUFACTURED TRUSS DESIGN SHALL INCLUDE SPECIFYING THE TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTIONS ON BITHER THE INDIVIDUAL TRUSS COMPONENT SUBJECT OF THE OFFICE AND TRUSS COMPONENT SHEETS OR THE GIRDER TRUSS COMPONENTS SHEETS AS APPLICABLE . A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION THAT A HANGER IS REQUIRED IN THE TRUSS SYSTEM.

IV. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION ON THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED UPON THE FINAL FLOOR TRUSS

F. CONVENTIONAL FRAMED JOISTS WITH A MINIMUM 6 INCH OVERLAP OF JOINTS.

G. TERMITE TREATMENT OF THE SITE SHALL BE SPECIFIED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER. H. SHRINKAGE CONTROL OF THE FLOOR SLAB SHALL BE ACCOMPLISHED BY 6 INCH BY 6 INCH. W 1.4 BY 1.4 WELDED WIRE FABRIC AS SPECIFIED BY FBC 2014 SECTION 1910.2 EXCEPTION 2 OR FIBERMESH ADMIXTURE AS SPECIFIED BY FBC 2014, SECTION 1910.2 EXCEPTION 1. THE WELDED WIRE FABRIC SHALL BE PLACED BETWEEN THE MIDDLE AND UPPER PABRIC SHALL BE PLACED BEI WEEN THE MIDDLE AND OFFER
1/3 DEPTH OF THE SLAB AND HELD IN POSITION BY APPROPIATE
SUPPORTS SPACED NOT GREATER THAN 3 FEET APART.
I. CONTRACTION JOINTS ARE TO BE PROVIDED FOR THE
PURPOSE OF CONTROLLING SHRINKAGE.ONE INCH DEEP CUTS (FOR A FOUR INCH THICK SLAB OR 25 PERCENT OF THE SLAB THICKNESS OTHERWISE) ARE TO BE PROVIDED ACROSS THE WIDTH AND LENGTH OF ANY FLOOR SLAB AT A DISTANCE OF NOT TO EXCEED 30 TIMES THE SLAB THICKNESS. FOR EXAMPLE A FOUR INCH THICK SLAB, CONTRACTION JOINTS SHALL NOT EXCEED 10 FEET ON CENTER EACH WAY.

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H ALLEN PROFESSIONAL ENGINE # 56920 C.A.# 9542 8809 SKYMASTER DR. NEW PORT RICHEY FL. 34654

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HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

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FAMILY LTD HOMES, 9400 RIVER CROSSI NEW PORT RICHEY 727-376-6831 DEEB .

A. MANUFACTURED FLOOR TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR

PLANNING TO BE USED BY THE TRUSS COMPANY.

I. FLOOR JOISTS ARE SIZED BASED ON THE SOUTHERN PINE COUNCIL SPAN TABLES FOR NO. 2 GRADE DIMENSIONAL LUMBER.

II. FLOOR JOISTS FOR EXTERIOR DECKS SHALL BE PRESSURE TREATED.

B. FOR ALL WOOD FLOORS:

I. THE TRUSS TO WALL CONNECTIONS ARE IDENTIFIED ON THE FLOOR FRAMING PLAN.

II. A STRUCTURAL BAND JOIST IS TO BE PROVIDED ON THE EXTERIOR PERIMETER OF ALL BOTTOM BEARING FLOOR TRUSSES AND JOISTS. THE STRUCTURAL BAND JOIST IS TO BE FASTENED TO EACH END OF A FLOOR TRUSS OR JOIST WITH A SIMPSON L50 BRACKET USING SIMPSON SHORT 10d COMMON NAILS.

III. FLOOR TRUSSES OR JOISTS BEARING ON WOOD WALLS ARE TO BE SET WITH A MINIMUM OF THREE 10d COMMON

NAILS.(TOE NAILED) TO THE TOP PLATE OF THE WALL.

IV. A MOISTURE BARRIER SHALL BE INSTALLED BETWEEN ANY
UNTREATED WOOD TRUSSES OR JOISTS AND CONCRETE OR ANY MASONRY.

V. LEDGERS/ NAILERS SHALL BE FASTENED TO WOOD STUDS OR BAND JOISTS (NOT SHEATHING ) WITH A MINIMUM 2 3/8" X 5 1/2" LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AT 16 INCHES ON CENTER AND SHALL CONSIST OF PRESSURE TREATED LUMBER 2 PLY 1 1/2" THICK BY A HEIGHT SHOWN IN THE PLANS, FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8" X 5 1/2" SIMPSON TITEN HEAD CONCRETE BOLTS.

VI. FLOOR BEAMS

BEAMS SUPPORTING FLOOR TRUSSES AND JOISTS ARE TO BE ATTACHED AS SPECIFIED IN THE FLOOR FRAMING PLAN.

2. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE CONTINUOUS

BETWEEN BEARING POINTS. 3. MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (I.E. GLULAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS

MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE INDIVIDUAL PLIES INTERCONNECTED AS FOLLOWS:

A. FOR TWO PLY BEAMS- ONE ROW OF 10d GALVANIZED COMMON NAILS AT 6" O.C. ON EACH SIDE OF THE BEAM

B. FOR THREE PLY BEAMS- TWO ROWS OF 16d GALVANIZED COMMON NAILS SPACED AT 6" O.C. (TOP AND BOTTOM) THRU EACH SIDE OF BEAM

EACH SIDE OF BEAM.

C. FOR FOUR PLY BEAMS OR LARGER-TWO ROWS OF 1/2" DIAMETER CARRIAGE BOLTS OR ALL THREAD ROD WITH NUTS AND WASHERS SPACED AT 12 INCHES ON CENTER, 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM. D. FLOOR SHEATHING:

I. ALL FLOOR SHEATHING IS TO BE 3/4" TONGUE AND GROOVE PLYWOOD RATED FOR FLOOR SHEATHING

II. FLOOR SHEATHING SHALL BE FASTENED TO THE FLOOR TRUSSES JOISTS WITH 10d RING SHANK NAILS AT 6" ON CENTER WITH CONSTRUCTION GRADE ADHESIVE.

III. FLOOR SHEATHING SPECIFIED FOR SEALED EXTERIOR DECKS AND ITS INSTALLATION SHALL BE THE SAME AS THAT FOR INTERIOR APPLICATION EXCEPT PRESSURE TREATED AND THE FASTENERS TO BE GALVANIZED. E. EXTERIOR DECK FLOORING:

1. DECK FLOORING SHALL BE INDIVIDUALLY SPECIFIED ON THE FLOOR FRAMING PLANS AND SHALL BE FASTENED TO THE UNDERLYING PRESSURE TREATED JOISTS WITH 3-3 INCH DECK SCREWS AE EACH FLOORING JOIST INTERSECTION.

22. WALLS:

A. MASONRY

CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI.

II. WALL CMU SHALL BE 8 INCH X 16 INCH IN SIZE OR 8 INCH X 8 INCH X 8 INCH FOR EDGE FINISHES.

III. CMU SHALL BE PLACED IN A RUNNING BOND AND THERE SHALL BE NO VERTICAL BUTT JOINTS EXCEPT AS SHOWN ON

THE FLOOR PLAN FOR CONSTRUCTION JOINTS.

IV. REINFORCED FILLED CELLS AS SHOWN ON THE PLANS SHALL
BE FILLED WITH "FINE" GRADE GROUT, HAVE A MINIMUM
COMPRESSIVE STRENGTH OF 3,000 PSI AND 8 TO 11 INCH SLUMP

COMPRESSIVE STRENGTH OF STOW FSTAND & TO THACH OBESING TO ENSURE CONSOLIDATION.

V. BOND BEAMS SHALL BE POURED WITH GROUT MONOLITHICALLY WITH THE FILLED WALL CELLS-NO COLD JOINTS.

VI. VERTICAL STEEL REINFORCEMENT SHALL BE CONTINUOUS BETWEEN THE MIDDLE AND BOTTOM 1/3 OF THE FOOTING HEIGHT AND END IN THE TOP COURSE OF THE BOND BEAM WITH A STANDARD TO THE MEND. 10 INCH 90 DEGREE BEND.

VII. HORIZONTAL REINFORCING STEEL SHALL BE CONTINUOUS, INCLUDING AROUND CORNERS.

VIII. REINFORCING STEEL SPLICES SHALL CONSIST OF WIRE LAPS NO LESS THAN 40 TIMES THE STEEL BAR DIAMETER (I.E. 25 INCHES FOR #5 REBAR, 15 INCHES FOR #3 REBAR, AND 52 INCHES FOR #7 REBAR)

B. WOOD FAME WALLS:

I. WALL STUD SIZES ARE SHOWN IN THE TYPICAL WALL SECTION.

I. WOOD STUDS IN WALLS SHALL BE SPACED 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES PER THE TOP PLATE SPLICE DETAIL. ALL LOAD BEARING STUDS TO BE SOUTHERN YELLOW PINE #2

2. LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED) IN CONTACT WITH MASONRY OR CONCRETE. SEE THE TOP PLATE SPICE DETAIL FOR TOP PLATE NAILING AND SPLICING REQUIREMENTS.

3. THE WOOD STUDS SHALL HAVE A SIMPSON SP2 AT THE TOP PLATE AND A PROPERLY SIZED SPH FOR THE BOTTOM PLATE (I.E. 4" STUD WALL = SPH4, 6" STUD WALL = SPH6)

4. 3 STUD PACK SHALL BE INSTALLED DIRECTLY BENEATH BEARING POINTS OF ALL GIRDERS AND BEAMS HAVING A GRAVITY LOAD OF UP TO 3,000 LBS. 5. STEEL TUBE COLUMNS SHALL BE INSTALLED IN THE WALL DIRECTLY BENEATH

GIRDERS AND BEAMS HAVING GRAVITY LOADS GREATER THAN 3000 LBS. 6. BASE PLATES SHALL BE FASTENED TO MONOLITHIC FOOTINGS WITH

5/8" X 8 INCH ANCHOR BOLTS OR SIMPSON TITEN HD. CONCRETE BOLTS OF THE SAME SIZE AT 24 INCHES ON CENTER. ALL CONNECTIONS SHALL BE MADE WITH 3 INCH SQUARE BY 1/8 INCH THICK WASHERS

BASE PLATES BEARING ON WOOD SHALL BE FASTENED WITH 16d COMMON NAILS AT 8" O.C. THROUGH ANY FLOOR SHEATHING AND TO UNDERLYING LUMBER (NOT SHEATHING ONLY ) AND USE BLOCKING AS NEEDED TO MAINTAIN NAILING SPACING REQUIREMENTS.

8. FOR EXTERIOR LOAD BEARING WALLS, EACH STUD ABOVE THE BASE PLATE SHALL BE FASTENED TO THE UNDERLYING BAND JOIST OR BEAM WITH A SIMPSON LSTAIR STRAP.FOR THIS SITUATION THE SIMPSON SPH BRACKET

SIMPSON LSTA18 STRAP.FOR THIS SITUATION THE SIMPSON SPH BRACKET TO THE BASE PLAN MAY BE OMITTED.

9. FOR INTERIOR LOAD BEARING WALLS, 1/2 INCH ALL THREAD ROD SHALL BE INSTALLED AT 32" O.C. FROM THE BASE PLATE THROUGH THE SHEATHING AND TOP PLATE OF UNDERLYING SUPPORTING WALL. ALL CONNECTIONS SHALL INCLUDE A STANDARD 3 INCH SQUARE WASHER.

10. HEADER BEAMS SHALL BE SIZED ACCORDING TO THE ENCLOSED HEADER SCHEDULE AND FASTENED WITH A MINIMUM OF TWO SIMPSON LSTA36 STRAPS OVER EACH END TO THE JACK STUDS BELOW. IN ADDITION, THE HEADER BEAMS SHALL BE FASTENED WITH A MINIMUM OF 3-10d COMMON NAILS (TOE NAILED ON FACH FACE SIDE AT EACH END TO THE ABUTTING NAILS (TOE NAILED ON EACH FACE SIDE AT EACH END TO THE ABUTTING FULL LENGTH STUDS.

III. NON LOAD BEARING WALLS:

1. WOOD STUDS IN WALLS SHALL BE SPACED AT 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES WITH A MINIMUM OF THREE 10d COMMON NAILS. NAILS INSTALLED IN PRESSURE TREATED WOOD SHALL BE GALVANIZED.

2. INCIDENTAL, NON STRUCTURAL FRAMING ITEMS SUCH AS KNEE WALLS, DROP CEILINGS, BUILT IN SHELVING, NICHES, ETC. MAY BE CONSTRUCTED WITH 2 X 4 'S AT 24" O.C. AT THE DISCRETION OF THE BUILDER

2. NON LOAD BEARING WALLS SHALL HAVE A SINGLE
BOTTOM PLATE (PRESSURE TREATED AGAINST MASONRY
AND CONCRETE) AND A SINGLE TOP PLATE.

3. BASE PLATES SHALL BE FASTENED TO CONCRETE SLABS
WITH 1/4 INCH BY 3 1/2 INCH TAPCON SCREWS AT 12 "ON
CENTER

4. BASE PLATES ON WOOD SHALL BE FASTENED WITH 16d COMMON NAILS AT 8" ON CENTER.

L PLYWOOD SHEATHING.

1. EXTERIOR WALL SHEATHING COVERED BY AN ARCHITECTURAL FINISH SHALL BE MINIMUM 7/16 INCH THICK (NOMINAL) 4 PLY PLYWOOD MANUFACTURED WITH EXTERIOR GLUE.

2. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE WALL STUDS.

PERFENDICULAR TO THE WALL STODS.

3. FASTEN TO STUDS AND BLOCKING WITH 8d RING SHANK NAILS AT 4 INCHES ON CENTER ALL LOCATIONS.

4. IN ADDITION TO THE REGULAR FASTENING, A SECOND ROW SHALL BE INSTALLED AT THE DOUBLE TOP PLATE AND TO THE LOWEST HORIZONTAL WOOD MEMBER ON AN EXTERIOR WALL.

(LE. SILL PLATE, BAND JOIST)

5. FOR PLYWOOD SHEATHING COVERED WITH A CEMENTITIOUS FINISH ALL BUTT JOINTS NOT ON WALL STUDS SHALL BE BLOCKED WITH 2 X BLOCKING, TOE NAILED AT EACH END TO THE WALL STUDS WITH 3-8d COMMON NAILS.

II. PARTICLE BOARD

 PARTICLE BOARD IS NOT TO BE USED WITHOUT THE EXPRESS,
 WRITTEN CONSENT OF THE STRUCTURAL ENGINEER AND THE PROPERTY OWNER.

III. ARCHITECTURAL FINISHES

1. ARCHITECTURAL WALL FINISHES, SUCH AS STUCCO, CEMENTITIOUS COATING, SIDING OR PAINT ARE MENTIONED HERE ONLY FOR THE PURPOSE OF UNDERSTANDING THAT THEIR INSTALLATION AND ASSOCIATED DETAILS ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

2. CONCRETE / MASONRY COLUMNS
1. MASONRY COLUMNS SHALL BE CONSTRUCTED OF PILASTER CONCRETE
BLOCK OR FORMED AND POURED. WALL BLOCK SHALL NOT BE USED

FOR MASONRY COLUMNS.

II. REINFORCING STEEL SHALL BE GRADE 60 AND HELD IN PLACE BY STIRUPS SPACED AT 12 INCHES ON CENTER VERTICALLY.

III. PILASTER BLOCK COLUMNS SHALL BE FILLED THAT A FINE GROUT THE PROPERTY OF A 1000 PSI

HAVING A MINIMUM OF COMPRESSIVE STRENGTH OF 3,000 PSI
IV. FORMED AND POURED COLUMNS SHALL CONSIST OF A MINIMUM
OF 3,000 PSI CONCRETE, OR IN AREAS OF HIGH CHLORIDES, SUCH AS NEAR THE COAST OR BODIES OF SALT WATER, THE MINIMUM SHALL BE 5,000 PSI

V. ALL MASONRY COLUMNS SHALL BEGIN AT THE FOUNDATION OR AT A MONOLITHIC FOOTING, IN NO CASE SHALL THERE BE A BREAK OR A COLD JOINT IN THE GROUT OF A COLUMN EXCEPT AT 1 FOOT FROM THE TOP IN PREPARATION FOR INSTALLATION OF A CONCRETE LINTEL.
VI. METAL CONNECTORS AT THE TOP OF THE COLUMN FOR HOLDING

WOOD BEAMS OR GIRDERS SHALL BE INSTALLED WITH THE MINIMUM EMBEDMENT OF THE ASSOCIATED FASTENERS FOR THE CONNECTOR AS SHOWN ON THE PLANS.

B. WOOD COLUMNS:

B. WOOD COLUMNS:

1. ALL LOAD BEARING WOOD COLUMNS SHALL BE A MINIMUM OF #2
GRADE PRESSURE TREATED WOOD.

1I. DIMENSIONAL WOOD COLUMNS OF 4 INCHES BY 4 INCHES IN CROSS
SECTION SHALL ONLY BE USED FOR SUPPORTING OPEN WOOD DECKS
WHERE THE FLOOR HEIGHT ABOVE THE FLOOR BELOW IS 8 FEET OR LESS.
ALL OTHER DIMENSIONAL WOOD COLUMNS SHALL HAVE A MINIMUM OF

III. METAL CONNECTORS AT THE BASE AND THE TOP OF WOOD COLUMNS SHALL BE OF THE TYPE THAT RESISTS LATERAL LOADS AS WELL AS UPLIFT AND GRAVITY LOADS. IN NO CASE SHALL FLAT STRAPS BE USED UNLESS SPECIFICALLY SHOWN IN THE PLANS OR CROSS SECTION DETAILS.

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ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEE! P.E. # 56920 CA. # 9542

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

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- 1. A COMPOSITE COLUMN HERE IS DEFINED AS A HOLLOW COLUMN CONSISTING OF ANY MATERIAL SPECIFICALLY DESIGNED BY ITS MANUFACTURER TO BE LOAD BEARING. ANY OTHER TYPE OF HOLLOW COLUMN IS CONSIDERED AN ARCHITECTURAL FINISH INTENDED TO FIT OVER A STRUCTURAL COLUMN AND ITS USE AND DETAILS OF INSTALLATION ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER
- STRUCTURAL ENGINEER.

  II. LOAD BEARING COMPOSITE COLUMNS ARE A MANUFACTURED PRODUCT SUBJECT TO THE DESIGN AND LOAD BEARING CAPACITY AS DETERMINED BY THE MANUFACTURER. A SHOP DRAWING OR A LETTER FOR THE INSTALLATION OF THE COLUMN SHALL BE PROVIDED BY THE STRUCTURAL ENGINEER TO SUPPLEMENT THE CONSTRUCTION PLANS AFTER THE SPECIFIC COLUMN AND MANUFACTURER HAVE BEEN IDENTIFIED.

  III.IN ALL CASES, THE COLUMN MANUFACTURES INFORMATION SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT FOR REVIEW PRIOR TO ITS ACCEPTANCE FOR THE STRUCTURAL DESIGN THE INFORMATION SHALL INCLIEDE THE LATERAL AS WELL AS UPLIFT
- DESIGN. THE INFORMATION SHALL INCLUDE THE LATERAL AS WELL AS UPLIFT AND GRAVITY LOAD BEARING CAPACITIES.
- D. STERL TUBE COLUMNS:
- 1. LOAD BEARING STEEL TUBE COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS OF 1/4 INCH AND BE MADE OF STEEL WITH A DESIGN YIELD STRENGTH OF 46 PSI UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN
- II. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE STEEL TUBE COLUMN IS TO BE INSTALLED.
- I. LOAD BEARING ALUMINUM COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS
- II. ALL FASTENERS AND CONNECTORS FOR ALUMINUM COLUMNS SHALL BE STAINLESS STEEL OR MONEL TO AVOID CORROSION DUE TO DISSIMILAR METALS BEING IN CONTACT.
- III. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE ALUMINUM COLUMN IS TO BE INSTALLED.

- A. MANUFACTURED WOOD TRUSSES

  1. THE MANUFACTURED ROOF TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL SYSTEM DESIGN. IT IS NOT INTENDED TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL
- II. MANUFACTURED ROOF TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE, THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.
- III. THE TRUSS PLAN " SIGNED AND SEALED" BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND PRIOR TO CONSTRUCTION OF THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED ON THE FINAL FLOOR TRUSS SYSTEM.
- CHANGES BASED ON THE FINAL FLOOR TRUSS SYSTEM.

  VI. THE TRUSS MANUFACTURER SHALL PROVIDE ALL LATERAL BRACING
  REQUIREMENTS TO THE BUILDING CONTRACTOR. IF NOT, THE BUILDING
  CONTRACTOR IS TO NOTIFY THE STRUCTURAL ENGINEER FOR GUIDANCE.

  V. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE
  ORIGINAL PLANS, EACH TRUSS IS TO BE SET ON WOOD FRAME BEARING WALLS
  OR SILL PLATES WITH 10d COMMON NAILS (TOE-NAILED)

  VI. A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND
  CONCRETE / MASONRY
- CONCRETE/MASONRY
- 23.2 CONVENTIONAL FRAME
- I. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH RAFTER IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 3- 10d COMMON NAILS (TOE-NAILED)
- II. ANY WOOD COMING IN CONTACT WITH MASONRY OR CONCRETE IS TO BE PRESSURE TREATED OR A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE OR MASONRY.

- III. COLLAR TIES ARE TO BE INSTALLED BETWEEN RAFTERS AT 23 OF THE RIDGE HEIGHT FROM WHERE THE RAFTERS BEAR ON WALLS. THE COLLAR TIES ARE TO BE FASTENED WITH A MINIMUM OF 4-104 16 COMMON NAILS (CLINCHED) AT EACH LAP JOINT. EACH RAFTER IS TO BE ATTACHED TO THE RIDGE BEAM WITH A LIGHT ANGLE HANGER AS SHOWN IN THE FRAMING PLAN. IN ADDITION, A FLAT METAL STRAP SHALL BE INSTALLED ACROSS THE RIDGE BEAM TO TWO OPPOSING RAFTER. TO BE REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO THE UNDERLYING STRUCTURE" CONNECTIONS.
- IV. AS PART OF THE REVIEW, THE STRUCTURAL ENGINEER WILL
  DETERMINE WHETHER THE TRUSS TO WALL / BEAM METAL
  CONNECTORS SHOWN IN THE ORIGINAL PLANS ARE ACCEPTABLE
  OR WHETHER THEY NEED TO BE CHANGED OR SUPPLEMENTED
  TO ACCOMM TO ACCOMMODATE THE LOADS SHOWN IN THE TRUSS COMPONENT
- SHEETS.

  V. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR VERIFYING THE DIMENSIONAL, ARCHITECTURAL, OR FORM ASPECTS OF THE OF THE TRUSS MANUFACTURERS PLAN WITH THE ORGINAL PLANS.

  VI. THE MINIMUM LIVE LOADS FOR THE ROOF TRUSS DESIGN IS TO BE ON FBC 2014 SECTION 1607 FOR ROOF TYPE AND ROOFING MATERIAL.

  VII. THE DEAD LOADS ARE LASTED IN ITEM 16 ABOVE.

  VIII. ALL TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTORS ARE TO BE SPECIFIED BY THE TRUSS MANUFACTURER, INCLUDING CONNECTORS FOR TRUSS TO MANUFACTURER BEAM (I.E. GLUFLAM.
- CONNECTORS FOR TRUSS TO MANUFACTURED BEAM (I.E. GLUELAM, OR MICROLAM ) SPECIFIED BY THE TRUSS MANUFACTURER. A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION, A HANGER IS REQUIRED IN THE TRUSS SYSTEM.
- IX. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORGINALPLAN AND FOR ANY CHANGES TO THE TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN
- MUST BE PROVIDED TO THE STRUCTURAL ENGINEER.

  X. A RIDGE BEAM TERMINATING AT A GABLE END SHALL BE SUPPORTED BY A MINIMUM 3 STUD PACK COLUMN BEARING ON THE UNDERLYING
- WALL OR BEAM.

  XI. TREATED LUMBER-DOUBLE 1 1/2 INCH BY A HEIGHT SHOWN ON THE PLANS. FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8 INCH BY 5 1/2 INCH SIMPSON TITEN HD CONCRETE BOLTS.

  XII. SLEEPERS SHALL BE FASTENED TO UNDERLYING ROOF TRUSSES OR RAFTERS (NOT SHEATHING) WITH A MINIMUM OF 2-3/8 INCH BY 3 1/2 INCH LAG BOLTS AND WASHERS AT EACH TRUSS OR RAFTER INTERSECTION AND NO GREATER THAN 24 INCHES ON CENTER AND SHALL CONSIST OF DIMENSIONAL LUMBER 1 1/2 INCH THICK AND SHALL CONSIST OF DIMENSIONAL LUMBER 1 1/2 INCH THICK BY A WIDTH SHOWN IN THE PLANS.
- XIII. USE 2 INCH BY 4 INCH BLOCKING ATTACHED BETWEEN UNDERLYING STUDS, TRUSSES OR RAFTERS WITH A MINIMUM OF 3-10d NAILS AT EACH IN ORDER TO SATISFY THE ON CENTER SPACING FOR THE LEDGERS OR SLEEPERS.
- XIV BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS ARE TO BE ATTACHED AS SPECIFIED IN THE ROOF FRAMING PLANS.
- 24. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM, THE PLIES ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.
- A. LEDGERS/ SLEEPERS

  I. LEDGERS / NAILERS SHALL BE FASTENED TO WOOD STUDS (NOT SHEATHING) WITH A MINIMUM OF 2-3/8 INCH BY 5 1/2 INCH LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AND NO GREATER THAN 16 INCHES ON CENTER AND SHALL CONSIST ON PRESSURE TREATED WOOD.

  II. MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (I.E. GLUELAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS SPECIFICATIONS.

- III. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS FOLLOWS:
- I. FOR TWO PLY BEAMS ONE ROW OF 10d GALVANIZED COMMON NAILS AT 6 INCHES ON CENTER ON EACH SIDE OF BEAM.

  II. FOR THREE PLY BEAMS TWO ROWS OF 16d GALVANIZED

  COMMONNAIS AT COMMONNER TOP AND POTTON OF
- COMMON NAILS AT 6" ON CENTER (TOP AND BOTTOM)
- THRU EACH SIDE OF THE BEAM. III.FOR FOUR PLY BEAMS AND LARGER-TWO ROWS OF 1/2 INCH
  DIAMETER CARRIAGE BOLTS OR ALL THREAD RODS WITH NUTS
  AND WASHERS SPACED AT 12" ON CENTER 2 INCHES FROM THE
  TOP AND BOTTOM EDGES OF THE BEAM.
- B. SHEATHING:
- L ROOF SHEATHING COVERED BY COMPOSITE ROOFING SHALL BB A MINIMUM OF 15/32 INCH THICK (NOMINAL.) O.S.B. MANUFACTURED WITH EXTERIOR GLUE.
- II. ROOF SHEATHING COVERED BY TILE SHALL BE A MINIMUM OF 5/8 INCH THICK (NOMINAL ) MANUFACTURED WITH EXTERIOR
- III. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE ROOF TRUSS SYSTEM.

  IV. FASTENING SHALL BE 8d RING SHANK NAILS AT 4 INCHES ON
- CENTER AT BOUNDARY AND EDGES AND 6 INCHES ON CENTER IN THE FIELD WITH A SETBACK OF 5 '-0" FROM ALL EDGES.

  V. METAL "H" CLIPS OR SOLID WOOD BLOCKING SHALL BE USED AT ALL UNSUPPORTED BUTT JOINTS BETWEEN TRUSSES OR RAFTERS.
- 25. PRECAST CONCRETE LINTELS
- 25. PRECAST CONCRETE LINTELS

  A. PRECAST AND PRESTRESSED CONCRETE LINTELS SHALL BE
  MANUFACTURED BY CASTCRETE AND INSTALLED PER MANUFACTURES
  SPECIFICATIONS AND INSTRUCTIONS.

  B. THE SIZE OF THE LINTELS SHALL BE BASED ON THE SPAN AND LOAD.
  REFER TO THE ATTACHED SCHEDULE UNLESS OTHERWISE SHOWN IN
  THE STRUCTURAL DESIGN FOR THE SPECIFIED LINTEL
  C. LINTEL SCHEDULE U.N.O. ON PLANS:
  I. SPAN UP TO 3'- 8F8-0B
- I. SPAN UP TO 3'- 8F8-0B II, SPAN UP TO 3' TO < 6' - 8F8-OB 111. SPAN 6' TO > 14' - 8F16- 1B/1T
- D. THE MINIMUM SPECIFIED GROUT COMPRESSIVE STRENGTH TO BE USED
- FOR LINTELS IS 3,000 PSI. E. THE REINFORCING STEEL SHALL BE ASTM GRADE 60
- 26. FASTENERS/METAL CONNECTORS.
- A. ALL FASTENERS AND METAL CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE AND INSTALLED PER THE MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS.
- B. THESE FASTENERS DO NOT INCLUDE TYPICAL NAILS AND SCREWS WHICH MAY BE MANUFACTURED BY OTHERS.
- C. FOLLOW ALL MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS FOR ALL FASTENERS, METAL CONNECTIONS, SCREWS, NAILS, ETC. THAT ARE IN CONTACT WITH PRESSURE TREATED LUMBER.
- 27. DIMENSIONAL LUMBER:
- A. ALL LOAD BEARING WALLS SHALL BE SOUTHERN YELLOW PINE #2 OR BETTER GRADED AND STAMPED BY THE CERTIFYING AGENCY. IN ADDITION, ALL WOOD SHALL BE PRESSURE TREATED FOR EXTERIOR USE WHERE EXPOSED TO MOISTURE, PLACED WITHIN 12 INCHES OF SOUR OR IN CONTRACT WITH CONTRACT OF THE PROPERTY OF SOIL OR IN CONTACT WITH CONCRETE OR MASONRY.
- 28. STRUCTURAL SHEATHING:
  A. ALL SHEATHING USED FOR EXTERIOR APPLICATIONS SHALL BE EXTERIOR GRADE AND ADA STAMPED AND VERIFYING ITS RATING.
- A. CONCRETE MASONRY UNITS SHALL CONFORM WITH AMERICAN MASONRY INSTITUTE STANDARD 530
- B. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI
- C. MORTAR SHALL BE OF TYPE M OR S GRAY MORTAR.
- A. ALL GROUT SHALL BE A FINE TYPE HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI UNLESS SPECIFICALLY SHOWN OTHERWISE BY A MANUFACTURER PURSUANT TO GROUT USE WITH ITS PRODUCTS.
- 31. REINFORCING STEEL:
- A. ALL REINFORCING STEEL SHALL BE ASTM GRADE 40 EXCEPT GRADE 60 SHALL BE USED FOR GRADE BEAMS, ALL LINTEL TYPES (I.E. PRECAST AND FIELD PREFORMED) COLUMNS UNLESS OTHERWISE SHOWN IN THE STRUCTURAL PLANS.

ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEER P.E. # 56920 C.A. # 9542 8593 SKYMASTER DR. NEW PORT RICHEY FL. 34654 727-842-6100

NOTE PLAN DATE ENGINEER

RUCTURAL

- 32. STRUCTURAL STEEL AND CONNECTION ACCESSORY MATERIAL:
  A. 1-BEAMS, FORMED STRUCTURAL STEEL, FLAT BAR OR PLATE
  SHALL BE ASTM GRADE A36 UNLESS STATED OTHERWISE.
- B. ALL STRUCTURAL STEEL SHALL HAVE A MINIMUM OF TWO COATS OF PRIMER AND TWO COATS OF EPOXY AS A CORROSION PREVENTIVE. THE BUILDING CONTRACTOR MAY VARY FROM THIS SPECIFICATION WITH THE APPROVAL OF THE STRUCTURAL ENGINEER IF IT CAN BE DEMONSTRATED ANOTHER MEANS OF CORROSION CONTROL IS EQUALLY EFFECTIVE.
- C. ALL WELDING OF STRUCTURAL STEEL SHALL BE MADE WITH E60/70 TYPE ELECTRODES. THE DEPTH AND LENGTH FOR THE WELD SHALL BE SPECIFIED IN THE STRUCTURAL DESIGN FOR THE
- A. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR DETERMINING VENTILATION REQUIREMENTS OF CRAWL SPACES, FLOORS AND ATTICS NOR THE MEANS AND METHODS FOR IMPLEMENTING THESE
- 34. WATERPROOFING:
  A. ANY RENDERING OF NOTES OF WATERPROOFING MEASURES FOR
  BASEMENTS OR HALF BASEMENTS SHOWN IN THESE PLANS WHERE
  A SPECIFIC CONSTRUCTION DETAIL IS NOT SHOWN IN THE STRUCTURAL
  DESIGN IS AN ARCHITECTURAL ILLUSTRATION ONLY AND IS NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- B. CRICKETS ARE ASSOCIATED WITH THE ARCHITECTURAL FINISHES AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- 35. FIRE RESISTANT DESIGN:
- A. FIRE RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON UNDERWRITERS LABORATORY OR GYPSUM ASSOCIATION DESIGN FOR FIRE RATED FLOOR, WALL AND ROOF ASSEMBLIES.
- 36. FLOOD RESISTANT DESIGN:
- A. FLOOD RESISTANT DESIGN.

  A. FLOOD RESISTANT DESIGN OF FLOOD RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DEIGN AND SHALL BE BASED ON THE REQUIREMENTS STATED IN TITLE 44 CFR SECTIONS 59 AND 60, AND ON THOSE OF THE INDIVIDUAL COMMUNITY RATING AGENCIES FOR THE GOVERNMENTAL JURISDICTION WHERE THE CONSTRUCTION IS TO BE DONE
- RATING AGENCIES FOR THE GOVERNMENTAL JURISDICTION WHERE THE CONSTRUCTION IS TO BE DONE.

  B. HOWEVER, THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR IDENTIFYING AND SHOWING ON THE PLANS THE FLOOD ZONE CATEGORY, BASE FLOOD ELEVATION, AND THE FLOOR AND STORY HEIGHTS OF THE BUILDING IN RELATION TO THE BASE FLOOD ELEVATION. THIS INFORMATION IS CONSIDERED ARCHITECTURAL AND SITE RELATED AND SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT.
- 37. SPECIAL CONSTRUCTION:

  L ALUMINUM STRUCTURAL COLUMNS.
- A. ANY ALUMINUM STRUCTURES SHOWN IN THESE PLANS SUCH AS PORCH AND POOL ENCLOSURES OR GUARDRAILS AND HANDRAILS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- B. WHERE THE ALUMINUM STRUCTURE ATTACHES TO THE MAIN STRUCTURE OR IS INCORPORATED IN THE MAIN STRUCTURE, SHOP DRAWINGS FOR THESE STRUCTURES SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER TO DETERMINE THEIR EFFECT ON THE MAIN STRUCTURE.
- A. ANY SWIMMING POOL OR HOT TUBS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL DESIGN.
- III. FENCES AND RETAINING WALLS:

  A. ANY RENDERING OF FENCES, RETAINING WALLS OR EXTERIOR PLANTERS WHERE A SPECIFIC STRUCTURAL DETAIL IS NOT SHOWN FOR THEIR CONSTRUCTION ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- IV. DRIVEWAYS AND WALKWAYS:
- A. ANY DRIVEWAYS OR WALKWAYS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION PURPOSES ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

Hunters Ridge

Floor and Roof Live Loads									
Attics:	20 psf w/ storage, 10 psf w/o storage								
Habitable Attics, Bedroom:	30 psf								
All Other Rooms:	40 psf								
Garage:	40 psf								
Roofs:	20 psf								

Wind D	esign Data	
Ultimate Wind Speed:		145 mph
Nominal Wind Speed:	-	112 mph
Risk Category:		II
Wind Exposure:		В
Enclosure Classification:		Enclosed
Internal Pressure Coefficient:		0.18 +/-
Components and Cladding Design Pro	essures:	
Roofing Zone 1:	+16.0 psf max.,	-20.7 psf min.
Roofing Zone 2:	+16.0 psf max.,	-36.0 psf min.
Roofing Zone 3:		-53.2 psf min.
Roofing at Zor	ne 2 Overhangs:	-42.1 psf min.
Roofing at Zor	ne 3 Overhangs:	-70.9 psf min.
Stucco, Cladding, Doors & Windows	4	
Zone 4:	+22.6 psf max.,	-24.5 psf min.
Zone 5:	+22.6 psf max.,	-30.2 psf min.
End Zone Width:		4.00 ft.
The Nominal Wind Speed was used	d to determine the	above Component

All exterior glazed openings shall be protected from wind-borne debris as per Section 1609.1.2 of the 2014 FBC.

The site of this building is not subject to special topographic wind effects as per Section 1609.1.1.1 of the 2014 FBC.

Design Soil Load-Bearing Capacity:	2,000 psf
Flood Design Da	ta

This table was created using Windload Calculator Plus software (2014 Florida Building Code Edition) available from WindCalcs.com

# 0.6 ALLOWABLE STRESS DESIGN USED

AUTOMATIC FIRE SPRINKLER SYSTEM PER FBC 903.3 SHALL BE PROVIDED , DESIGNED AND ENGINEERED BY OTHERS

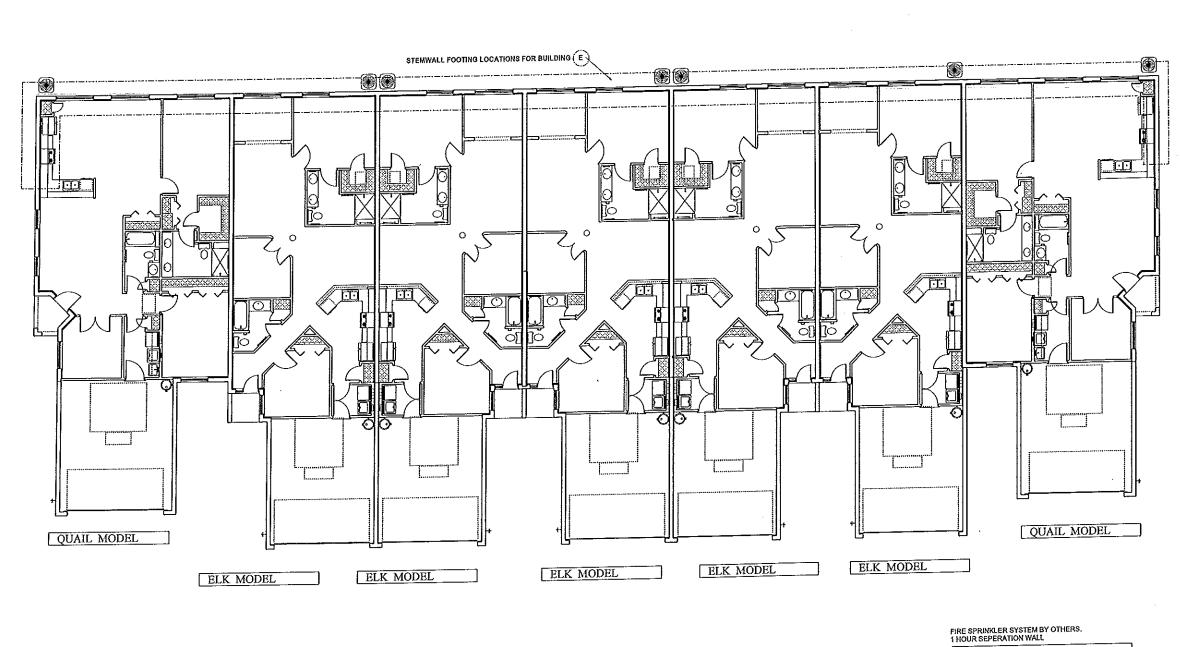
DAT LOAD DESIGN

DEEB

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

FAMILY LID HOMES,



OCCUPANCY CLASS - R-2

CONSTRUCTION TYPE 5A

OCCUPANCY LOAD - QUAIL -7.5

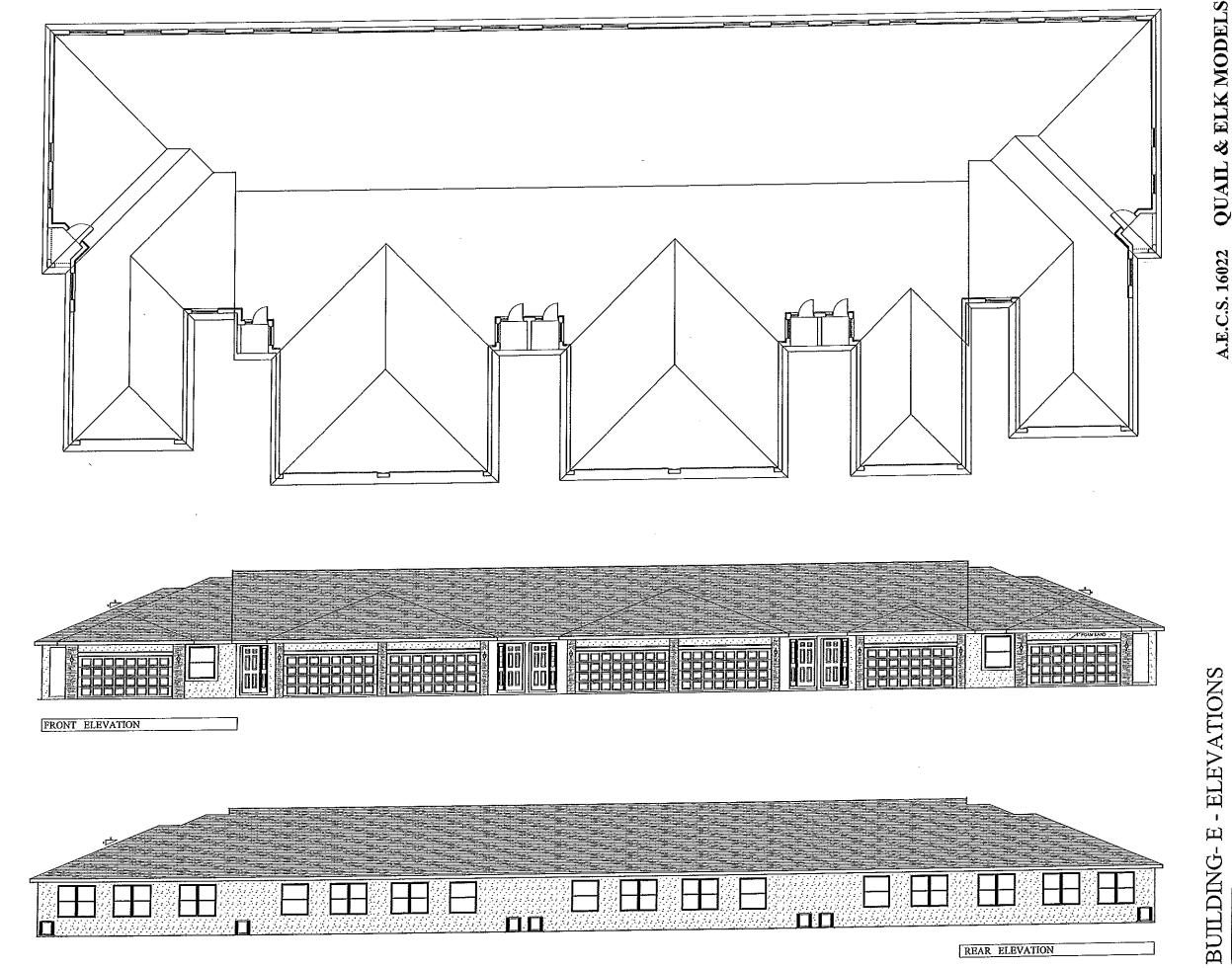
OCCUPANCY LOAD - ELK- 6.77

FLOOR PLANS 中 BUILDINGS

& ELK MODEL

HUNTERS RIDGE NEW PORT RICHEY

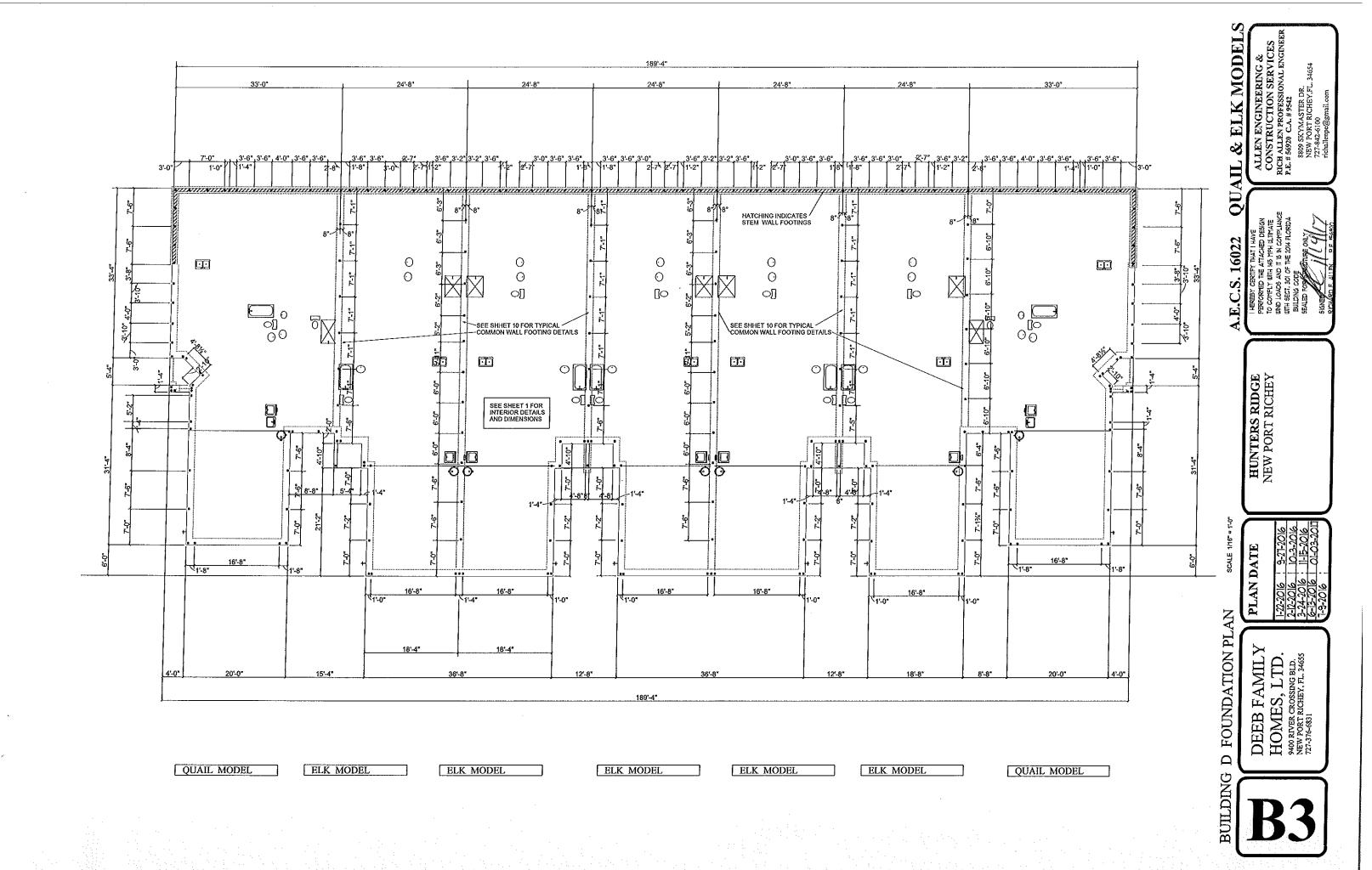
PLAN DATE

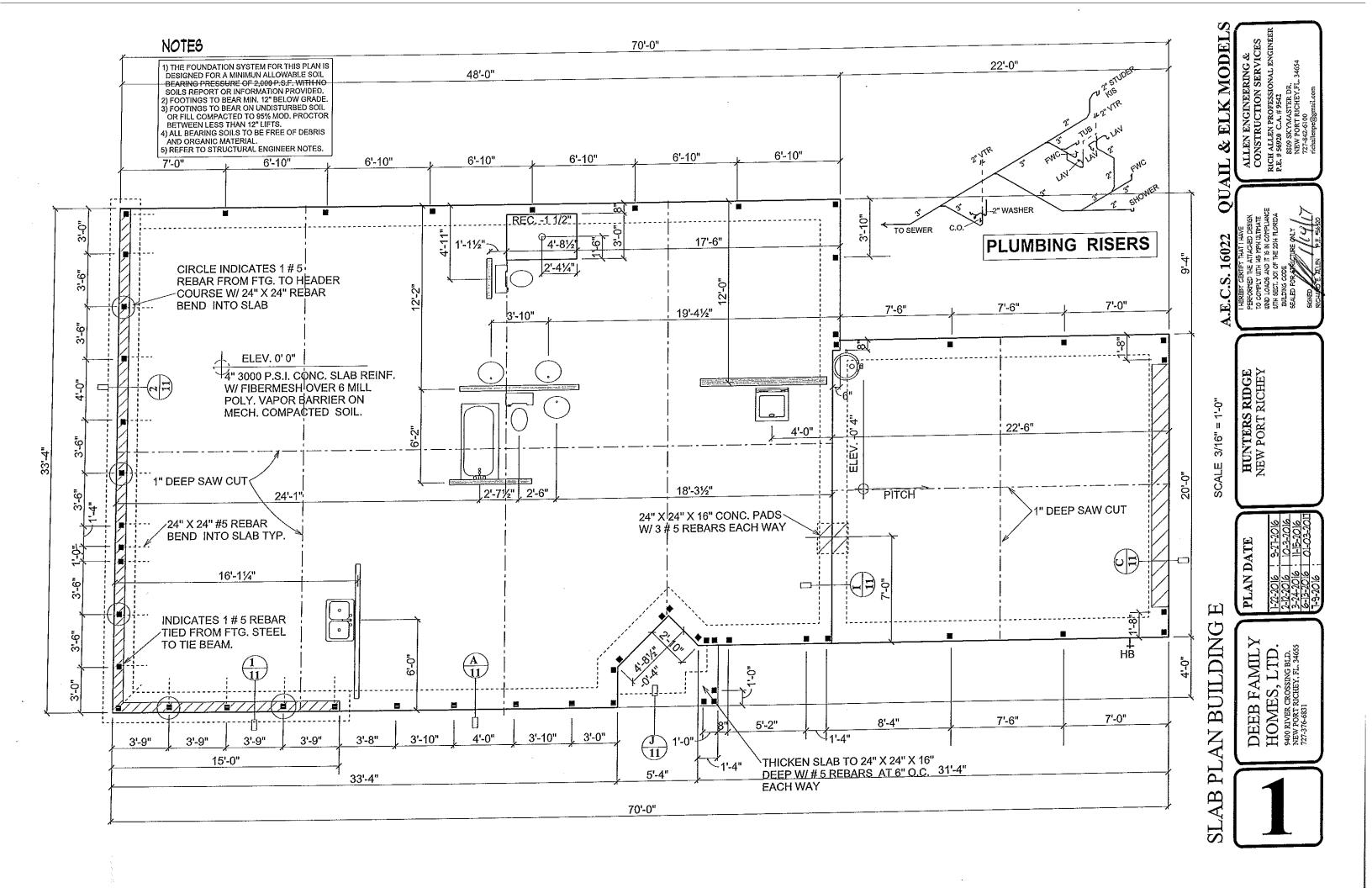


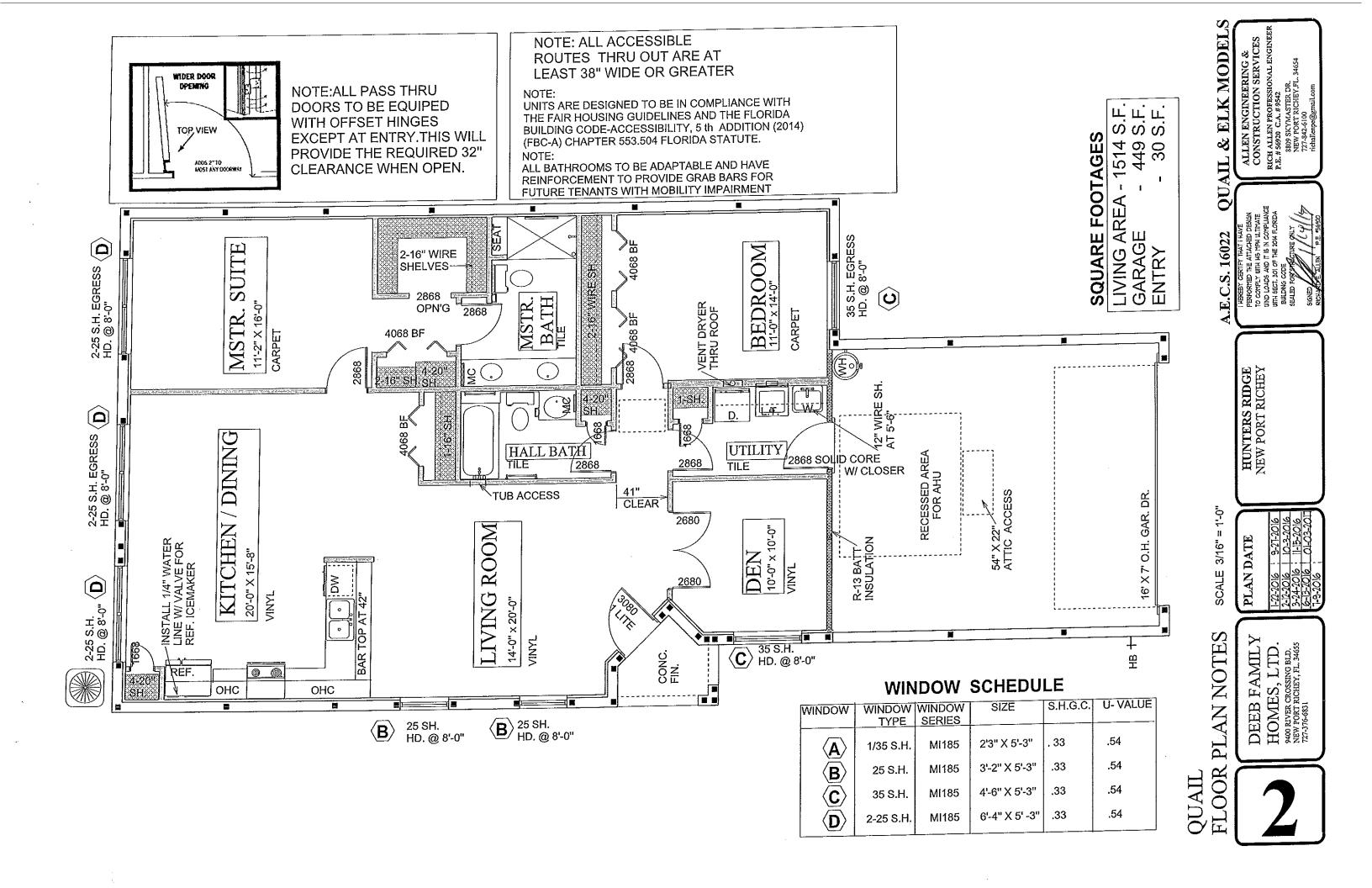
PLAN DATE DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

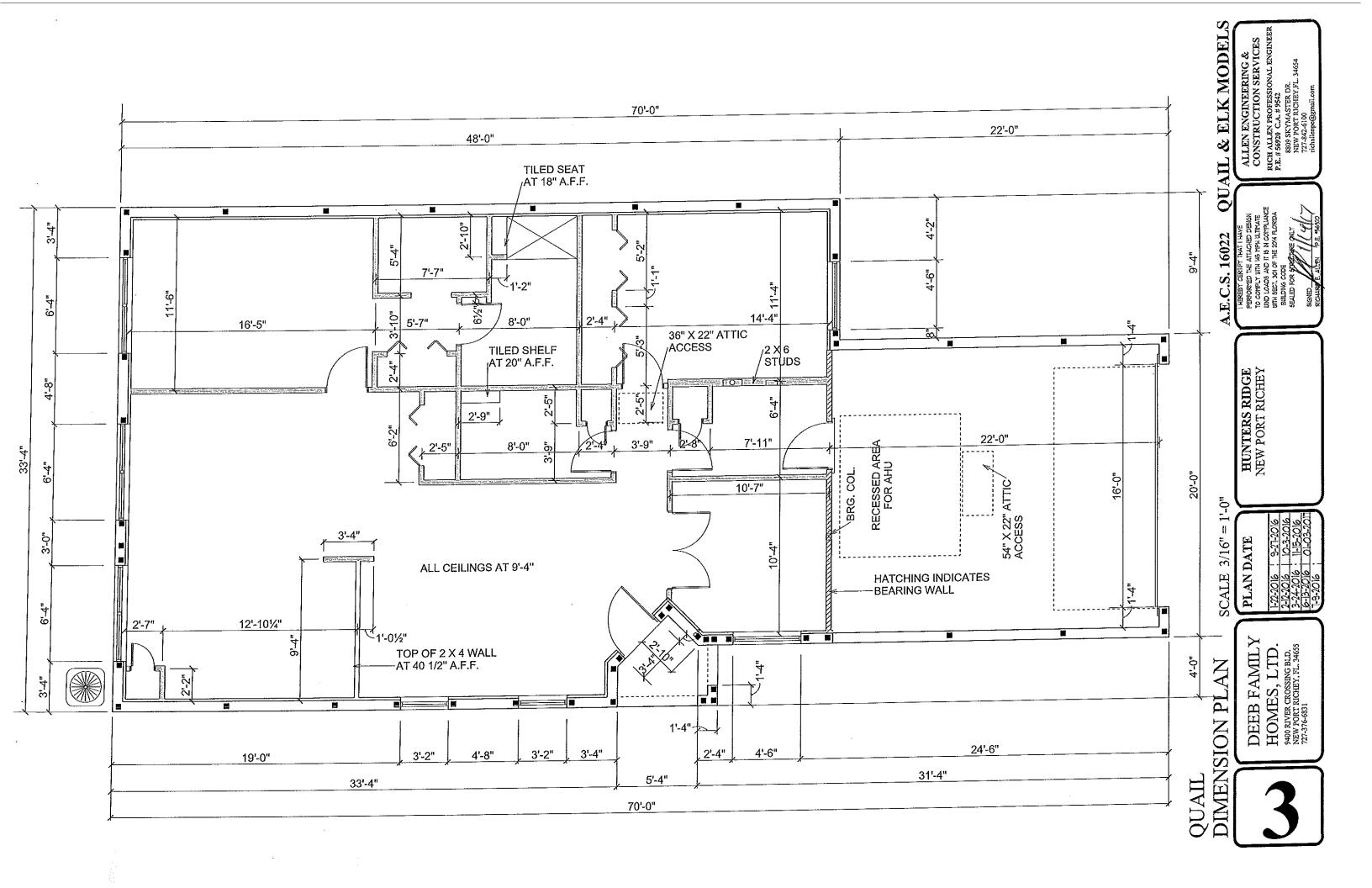
HUNTERS RIDGE NEW PORT RICHEY

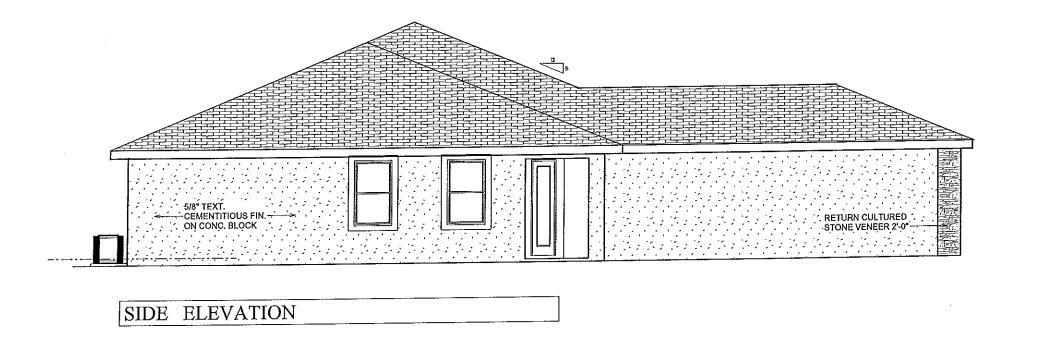
ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEER P.E. # 55920 C.A. # 9542

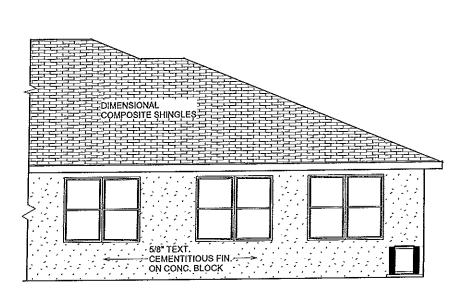




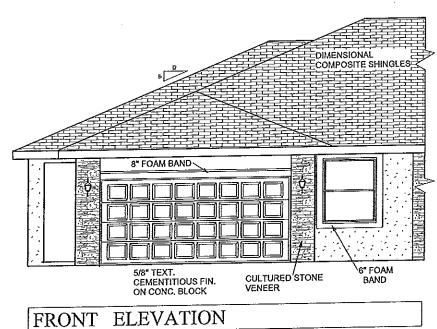




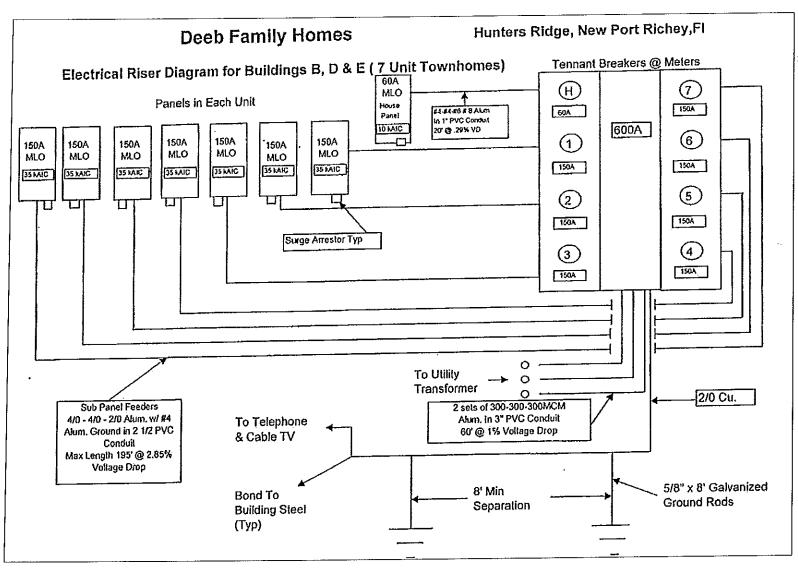




REAR ELEVATION



HUNTERS RIDGE NEW PORT RICHEY 1.0.1 1/8" SCALE PLAN DATE ELEVATIONS DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831 EXTERIOR QUAIL



# TYPICAL UNIT PANEL

CKT		CB	CB	WIRE	PH	ASE:	WIRE	CB	СВ		CKT
#	LOAD DESCRIPTION	POLES	AMPS	SIZE	A	]B	SIZE	AMPS	POLES	LOAD DESCRIPTION	#
1-7	KITCHEN APPLIANCE	1	20	#12	İχ	1	#12	20	1_1_	WASHER	1 2
3	KITCHEN APPLIANCE	1	20	#12	1	x	#14	15	1	GARAGE	4_
	REFRIGERATOR	<del>  -                                   </del>	20	#12	X	<del>  ``</del>	#10	30	2	DRYER	6
5		<del>                                     </del>	20	#12	<del>                                     </del>	ĺχ		30			<b>]</b> 8_
7	DISPOSAL			#12	x	┝╌	#10	30	2	WATER HEATER	1 1
9	DISHWASHER	1	20		<del>  ^</del>	<del>  ∪</del>	#10	30	┝╼		12
11	MICROWAVE	1	20	#12	<del>↓</del>	X	#8	40	<del>  2</del>	RANGE	14
13	DINING ROOM	1	20	#12	1~	<del> </del>	#0	40	<del> </del> -	100101	16
15	BATHROOMS	11	20	#12	ļ.,.	X	- 12	50 **-	1 2	AH-1	18
17	BEDRM/LIGHTING (AFI)	1	15	#14	X	l	#6		<del> </del> _	747-	20
19	BEDRM/LIGHTING (AFI)	1	15	#14	١	X		50 **	2	CU-1	22
21	LIVING/LIGHTING (AFI)	1	15	#14	X	ļ	#8	40 **	<u> </u>		24
23	LIVING/LIGHTING (AFI)	1	15	#14	<u> </u>	X_		40 **		SPARE	26
25	SPARE			l	X	<u> </u>		L	ļ	SPARE	28
27	SPARE			<u> </u>		X		ļ	<u> </u>	SPARE	30
29	SPARE			Ι	X	L				OFARE .	1 30

\*NOTE: All all branch circuit wiring to meet voltage drop requirements of >2% per FBC Section C405.7.3.2

Building B, D & E		
Service Calculation		
ELK MODEL INTERIOR UNIT LOAD (CALCULATED) NUMBER OF TYPICAL UNITS	32.23 X <u>5</u> 161,15	KV/
QUAIL MODEL W/ GARAGE LOAD (CALCULATED)	33.06 X <u>2</u> 66.12	KV/
SUB - TOTAL DEMAND FACTOR PER NEC	227.27 X <u>0.44</u> 99.99	KV/
TOTAL HOUSE LOAD @ 160%	2.4	KV
SUB - TOTAL	102.39	KV
@240V 1PHASE	428 S	

# HOUSE SERVICE Panel H

CKT	ı ————————————————————————————————————	C8	СВ		PH.		WIRE	C8	CB	LOAD DESCRIPTION	CKI
#	LOAD DESCRIPTION	POLES	AMPS	SIZE	Ā	В	SIZE	AMPS	PULES		2
	Fire Alarm Panel	1_1_	20	#12	×	<u> </u>	<u> </u>	<b> </b>	<b>-</b>	Spare	4
3	Spare	<u> </u>	<u> </u>		<u> </u>	L×.	<u> </u>	Ļ	<b>↓</b>	Spare	1 1
5	Spare	1	l	İ	<u> </u> ×	<u>L</u> .		.	<u> </u>	Spare	6
7	Spare			I	<u> </u> _	×	<u> </u>		ļ	Spare	8
9	Spare	i		Г	×		<u> </u>	<u> </u>	<u> </u>	Spare	1 1
11	Spare	1				×		<u>L</u>		Spare	12
	Connected Load VA			ESTIMA	VTE0	DE!	A GYAL	MPS		FEEDER	
	PHASE A 240		VOLTAGE PHASE					) 	LINE CONDUCTORS - SEE RIS MUETRAL - SEE RISER GRD CONDUCTOR - SEE RISE		
	PHASE B		1							CONDUIT DIA - SEE RIS	ER
	TOTAL CONNECTED	2400									



105 Douglas Road East Oldsmar, Florida 34677-2911 813-855-6692 Fax: 813-855-4284 Info@ss-electric.com MODEL

ELK

&

QUAIL

A.E.C.S. 16022

Load	Calculatio	n		
Project Information:	Quail Mo	ode	el w/ Gar	age
i Tojoot anomation	Hunters	Ri	dge, Nev	/ Port Richey
Description	Qty.		Qty.	Watts
Sq. Ft. x 3 Watts	1514	x	3	4542
Small Appliance Branch		×	1500	3000
Laundry	I	x	1500	1500
Disposal		x	1080	1080
Dishwasher		x	1300	1300
Range		x	8000	8000
Oven		x	9600	0
Cook Top		x	9000	0
Jen Alr		x	7680	0
Water Heater	1	х	4500	4500
Dryer		X	5000	
Microwave	1	x	1200	
Jacuzzi	†	X	2400	0
Pool		×	1200	0
FUUI	<del>                                     </del>	x	7200	0
Pool Heater	1	x	14400	0
Bath Fans		×	60	0
Dalli Falls		x	60	
	<u> </u>		b Total =	30,122.00
				(10,000.00)
		Su	b Total =	
			x .40%	x .40%
		Su	b Total =	
				10,000.00
AC Name plate or 4 x Sq Ft		-		
	1514	Īχ	4	6056
AC#1	'	X	<del></del>	O
AC#2 AC#3	<del> </del>	Ιχ		0
AH (KW + 1000 + Fan)	<u>.l.,</u>	1,,	<u></u>	
	8kW	1		8960
AH#1	J.(17	1-		
AH#2	<del> </del>	t		C
AH#3	J. To	ı ha	Watts =	33,065.00
			ivided by	
	To		Amps =	

www.ss-electric.com (P) 813.855.6692 - (F) 813.855.4284

Main Breaker Size

SES Electric Co. Inc. - ECOXXIII. ECISCOSIVIZ CACININIII SES Electric Co. LLC - ECISCOSIVIZ, SAS AIR CONSISSIONING, LLC - CACINISSIV

HUNTERS RIDGE NEW PORT RICHEY AND RISERS PLAN DATE ELECTRICAL LOAD DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

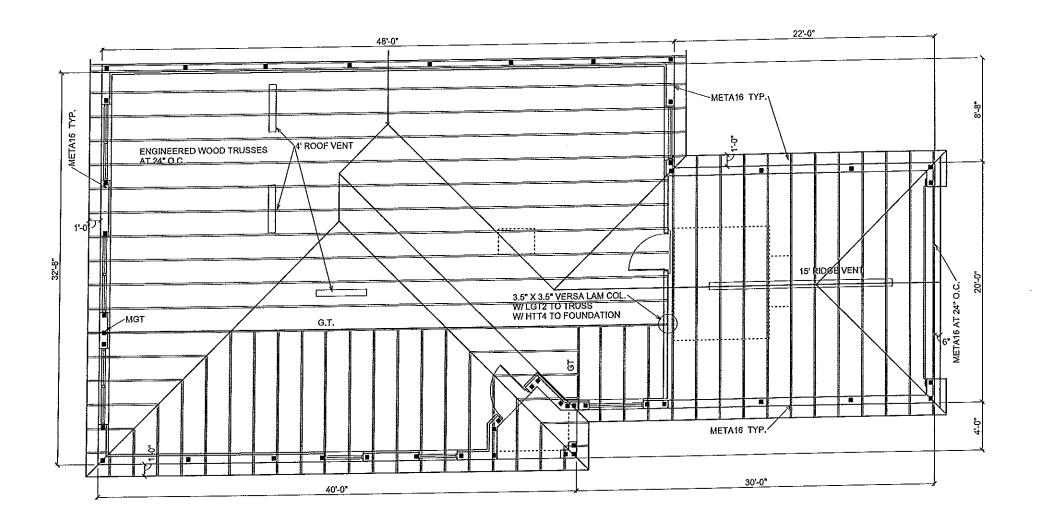
150

<sup>\*\*</sup> Coordinate with Mechanical shop drawings for final breaker sizes.

ALL TRUSS TO TRUSS CONNECTORS BY
TRUSS SYSTEMS ENGINEER AND TO BE
SPECIFIED ON INDIVIDUAL SEALED TRUSS SHEETS

NOTE: INSTALL MOISTURE BARRIER BETWEEN MASONRY & UNTREATED WOOD IMPORTANT NOTE:

THIS FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY. TRUSS MANUFACTURER TO PROVIDE SEPERATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER AND REVIEWED BY P.E. OF RECORD.

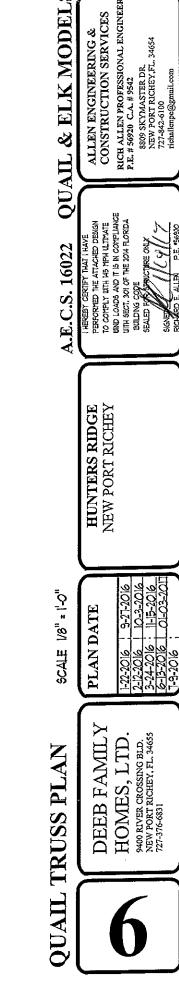


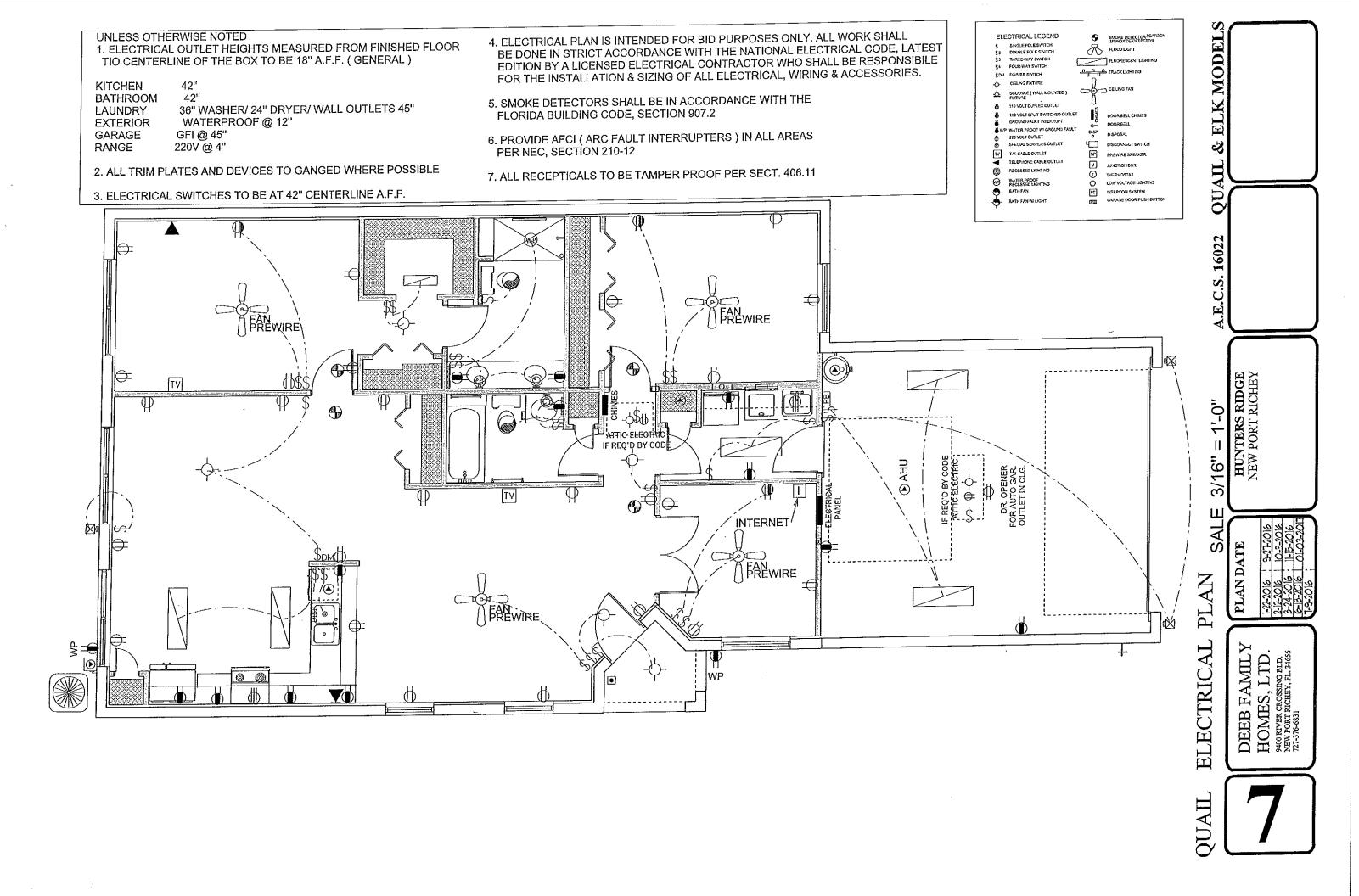
TOTAL NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1 TO 300 PROVIDED THAT AT LEAST 50 % AND NOT MORE THAN 80 % IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED PER SECT. R806.2

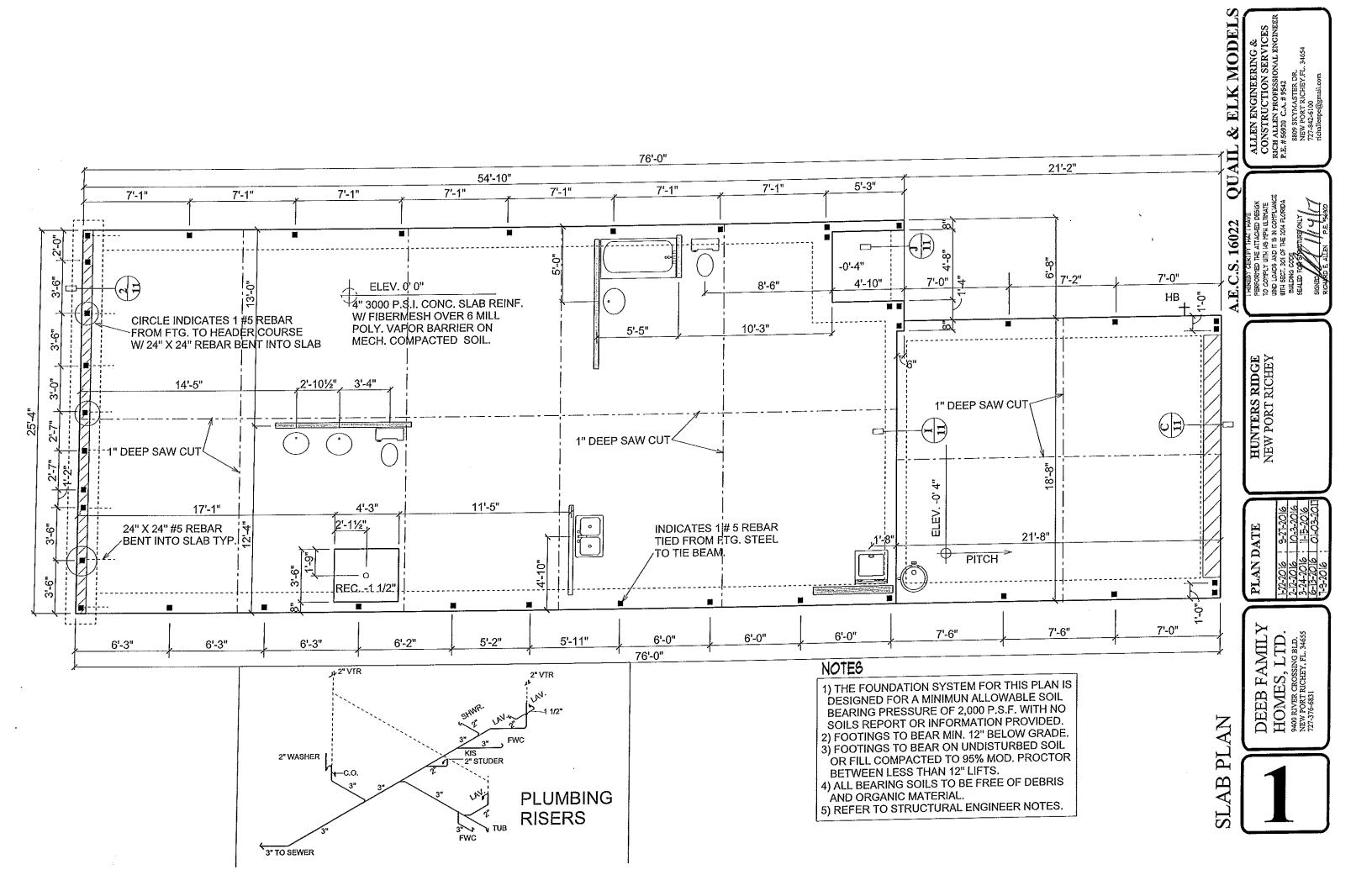
TOTAL AREA TO BE VENTILATED = 1993 S.F. 1993/300 = 6,64 S.F. OR 956.16 SQUARE INCHES.

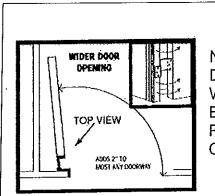
ROOF VENTS ARE RATED AT 36 SQUARE INCHES OF OPENING PER LINEAL FT. 956.16 S.I./36 S.1.  $\pm$  26.56 LINEAL FEET REQUIRED.

INSTALLATION FOR THIS ROOF IS 27' OF ROOF VENTING









NOTE:ALL PASS THRU DOORS TO BE EQUIPED WITH OFFSET HINGES EXCEPT AT ENTRY. THIS WILL PROVIDE THE REQUIRED 32" CLEARANCE WHEN OPEN.

NOTE: ALL ACCESSIBLE ROUTES THRU OUT ARE AT LEAST 38" WIDE OR GREATER

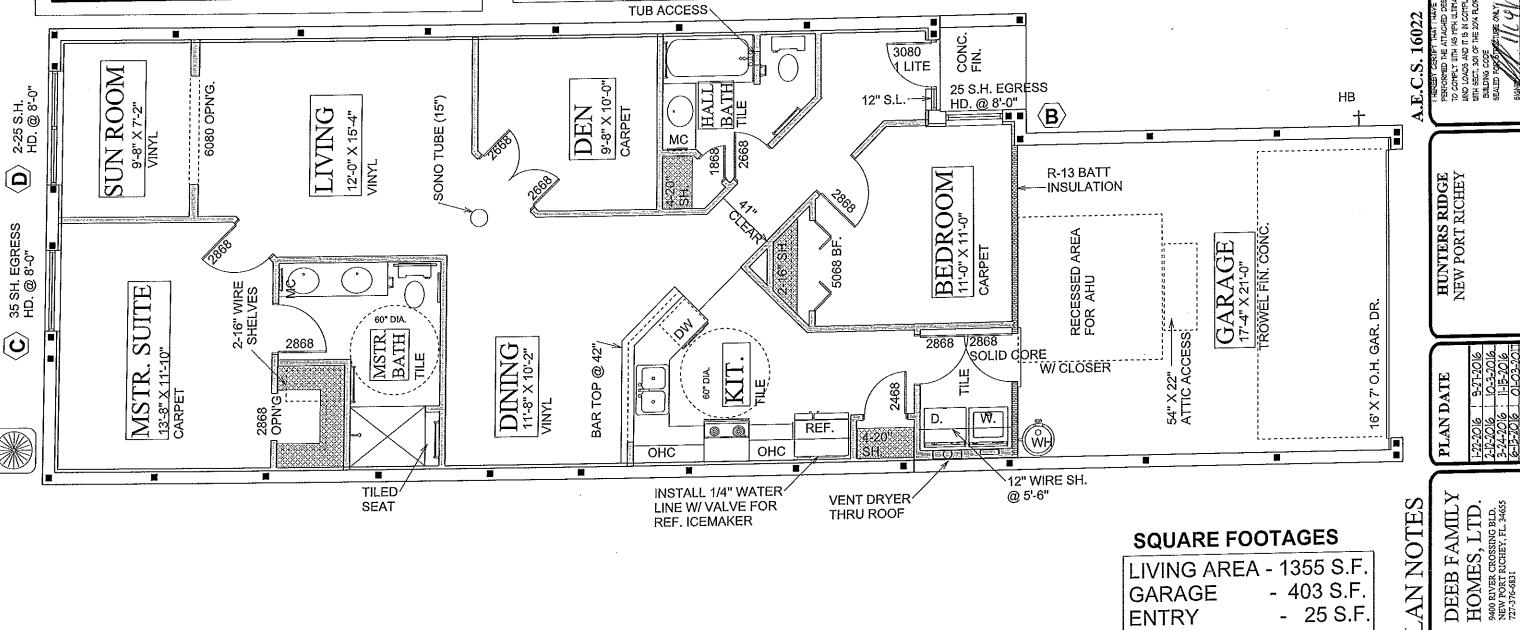
UNITS ARE DESIGNED TO BE IN COMPLIANCE WITH THE FAIR HOUSING GUIDELINES AND THE FLORIDA BUILDING CODE-ACCESSIBILITY, 5 th ADDITION (2014) (FBC-A) CHAPTER 553.504 FLORIDA STATUTE.

NOTE:

ALL BATHROOMS TO BE ADAPTABLE AND HAVE REINFORCEMENT TO PROVIDE GRAB BARS FOR FUTURE TENANTS WITH MOBILITY IMPAIRMENT

# WINDOW SCHEDULE

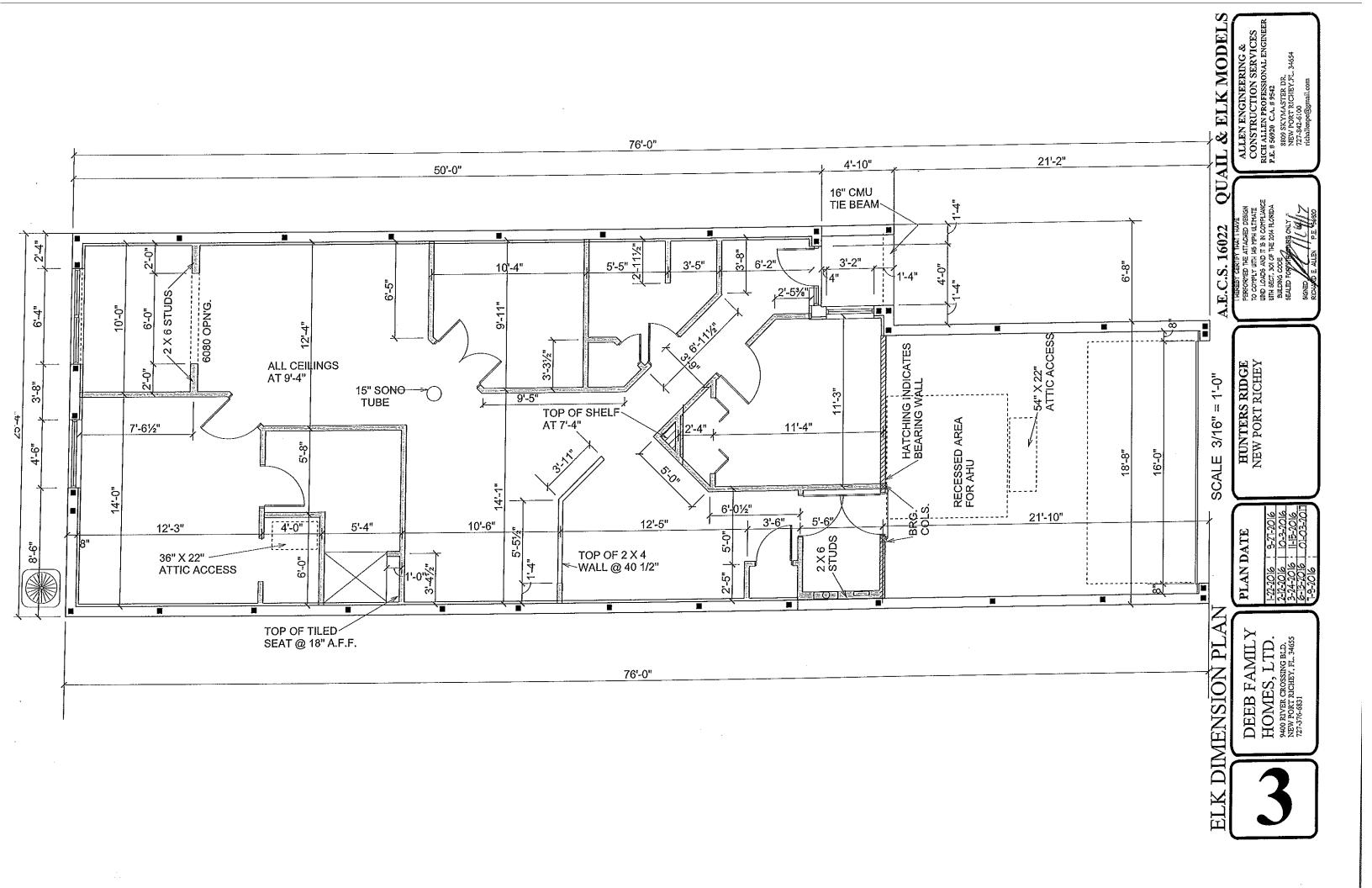
WINDOW TYPE	WINDOW SERIES	SIZE	S.H.G.C.	U- VALUE
1/35 S.H.	MI185	2'3" X 5'-3"	. 33	.54
25 S.H.	MI185	3'-2" X 5'-3"	.33	.54
35 S.H.	MI185	4'-6" X 5'-3"	.33	.54
2-25 S.H.	Ml185	6'-4" X 5' -3"	.33	.54
	TYPE  1/35 S.H.  25 S.H.  35 S.H.	TYPE SERIES  1/35 S.H. MI185  25 S.H. MI185  35 S.H. MI185	TYPE SERIES  1/35 S.H. MI185 2'3" X 5'-3"  25 S.H. MI185 3'-2" X 5'-3"  35 S.H. MI185 4'-6" X 5'-3"	TYPE         SERIES           1/35 S.H.         MI185         2'3" X 5'-3"         . 33           25 S.H.         MI185         3'-2" X 5'-3"         .33           35 S.H.         MI185         4'-6" X 5'-3"         .33

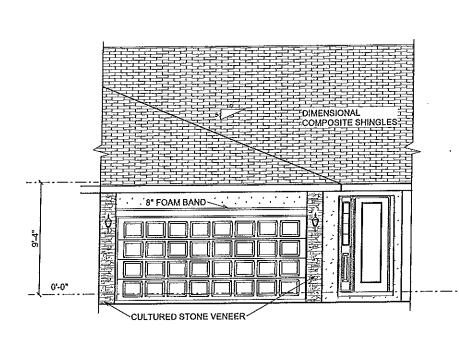


**SQUARE FOOTAGES** 

LIVING AREA - 1355 S.F. **GARAGE** - 403 S.F. - 25 S.F. **ENTRY** 

PL



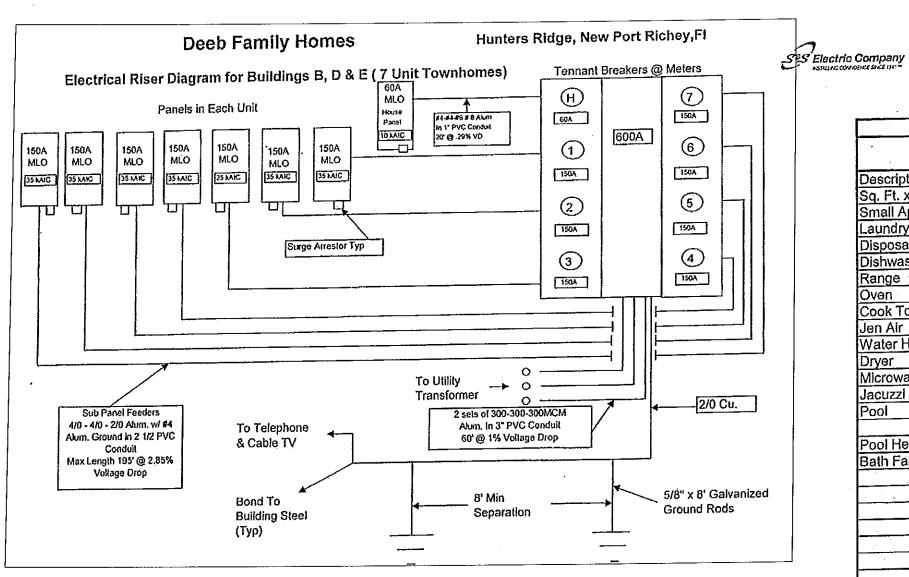


FRONT ELEVATION



REAR ELEVATION

ELK EXTERIOR ELEVATIONS



# TYPICAL UNIT PANEL

Oler	T	TC8	CB	WIRE	PН	ASE	WIRE	ICB	IC8		CKT
CKT #	LOAD DESCRIPTION	POLES	3	SIZE			SIZE	AMPS		LOAD DESCRIPTION	#
-#-	KITCHEN APPLIANCE	1	20	#12	ĺχ	<del> </del>	#12	20	1	WASHER	2_
	KITCHEN APPLIANCE		20	#12	1^	lх	#14	15	1	GARAGE	4
<u>3</u>	REFRIGERATOR		20	#12	1x	<del>  ~</del>	#10	30	2	DRYER	6
- 5	DISPOSAL	<del>                                     </del>	20	#12	1^	<del>l x</del>		30	<u> </u>		8
9	DISHWASHER	1	20	#12	Ιx		#10	30	2	WATER HEATER	1
11	MICROWAVE	1	20	#12	+	x		30	1		12
13	DINING ROOM	1	20	#12	ТX		#8	40	2	RANGE	14
15	BATHROOMS	1	20	#12		Х		40			16
17	BEDRM/LIGHTING (AFI)	1	15	#14	Х		#6	50 **	2_	AH-1	18
19	BEDRM/LIGHTING (AFI)	1	15	#14		X		50 **	ļ		20
21	LIVING/LIGHTING (AFI)	1	15	#14	X		#8	40 **	2	CU-1	24
23	LIVING/LIGHTING (AFI)	1	15	#14		X		40 **	<b> </b> -	SPARE	26
25	SPARE			<u> </u>	X	Ļ.,_		<del> </del>		SPARE	28
27	SPARE			<b> </b>	I.,	X		<del> </del>	<del> </del>	SPARE	30
29	SPARE			I	X			1	1	\$ 74 KL	

'NOTE: All all branch circuit wiring to meet voltage drop requirements of >2% per FBC Section C405.7.3.2

Building B, C	) & E
Service Calcu	ılation
ELK MODEL INTERIOR UNIT LOAD (CAL NUMBER OF TYPICAL UNITS	CULATED) 32.23 X <u>5</u> 161.15
QUAIL MODEL W/ GARAGE LOAD (CALC	33.06 X 2 65.12
SUB - TOTAL DEMAND FACTOR PER NEC	227.27 X 0.44 99.99
TOTAL HOUSE LOAD @ 100%	2.4
SUB - TOTAL	102.39
@240V 1PHASE	A Se s.

Н	ΙΟU	SE	SER	VICE	Panel	п

CKT	LOAD DESCRIPTION	CB POLES		WIRE	PH.	ASE B	WIRE	C8 AVPS	CB POLES	LOAD DESCRIPTION	CKT #
<del>-</del>	Fire Alarm Panel	1	20	#12	×	1	1 -			Spare	2
		<del> </del> -		1	1	×		1		Spare	4
_3_	Spare	<del> </del>	├──		×	<del>  ~</del>	<del>                                     </del>	1	1	Spare	6
5_	Spare	<del> </del> -	ļ	<del>                                     </del>	†^	<del>-</del> -	<del> </del>	<del> </del>		Spare	8.
7	Spare		<b>├</b> ┈─	<del> </del>	ł٠	X	<del> </del>	╂──	<del>                                     </del>	Spare	1
9	Spare	.ļ	ļ <u> </u>	ļ	×	-		┼─	<del> </del>	Spere	12
11	Spare	ل	<u> </u>	<u> </u>	١	X	<u> </u>	٠	├		1
	Connected Load VA PHASE A PHASE B	2400 0		ESTIMATED DEMAND A VOLTAGE 2: PHASE						FEEDER LINE CONDUCTORS - SE NUETRAL - SEE RISER GRD CONDUCTOR - SEE CONDUIT DIA - SEE RISE	RISER
	TOTAL CONNECTED	2400							<u> </u>		

105 Douglas Road East Oldsmar, Florida 34677-2911 813-855-6692 Fax: 813-855-4284 Info@ss-electric.com & ELK MODEL

QUAIL

Load (	Calculatio	n		
. Project Information:	Elk Mode	el (	Interior (	Jnit)
	Hunters	Ri	dge, Nev	Port Richey
Description	Qty.		Qly.	Watts
Sq. Ft. x 3 Watts	1355		3	4065
Small Appliance Branch		X	1500	
Laundry		X	1500	
Disposal	1		1080	
Dishwasher	1	_	1300	
Range	1	_	8000	
Oven		Х	9600	
Cook Top		х	9000	
Jen Air		X	7680	
Water Heater	1	Х	4500	
Dryer	1	-	5000	
Microwave	1	<u></u>	1200	
Jacuzzi		Х	2400	
Pool		<u>x</u>	1200	
7 001		X	7200	
Pool Heater		X	14400	
Bath Fans		X	60	
Dati i dio		x	60	
Sub Total =				
				(10,000.00)
Sub Total =				
x .40%				
Sub Total ≈				7,858
				10,000.00
AC Name plate or 4 x Sq Ft		-		
AC#1	1355	x	4	5420
AC#2		x		C
AC#3		X		C
AH (KW + 1000 + Fan)	<u> </u>	حنيا		
AH #1	8kW	Γ		8960
		Γ		
AH#2		T	<del></del>	C
AH#3	To	ta	Watts =	32,238.00
Divided by				
Total Amps =				
		, , ,	. ,	
	Main Br	62	ker Size	150

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\$15 Electric Co., Inc., + EC0002719, EC13003932, CAC181417, S85 Electric Co., LLC - EC13003993, E85 Ar Conditioning, LLC - CAC1815399

HUNTERS RIDGE NEW PORT RICHEY AND RISERS PLAN DATE DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RIGHEY, FL. 34655 727-376-6831 ELK ELECTRICAL LOAD

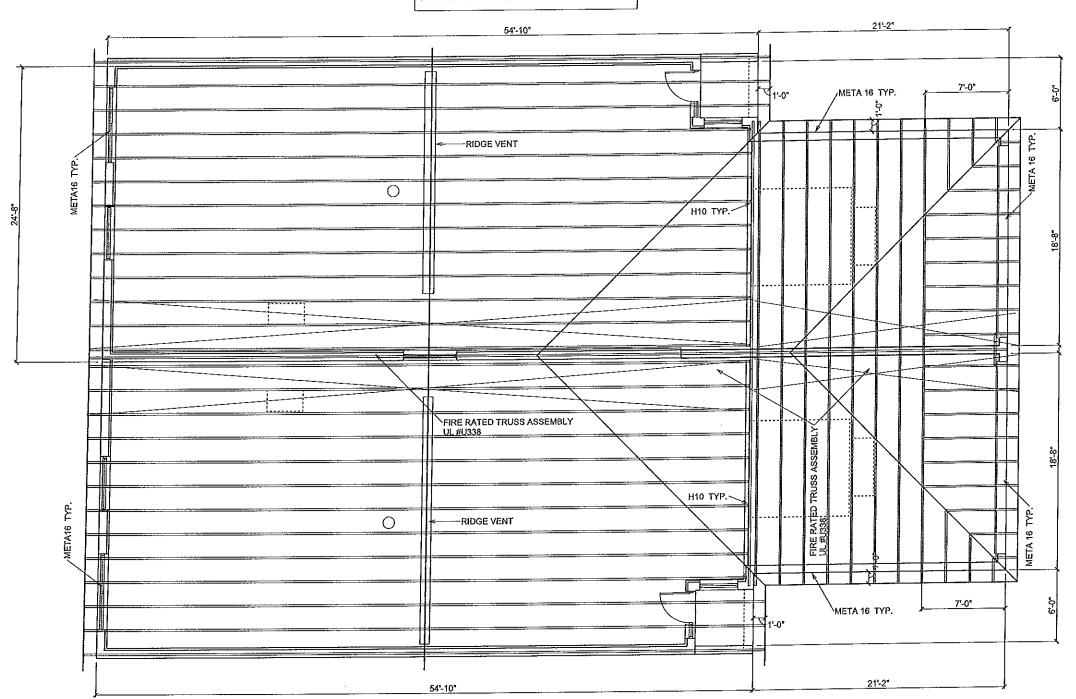
Coordinate with Mechanical shop drawings for final breaker sizes.

ALL TRUSS TO TRUSS CONNECTORS BY TRUSS SYSTEMS ENGINEER AND TO BE SPECIFIED ON INDIVIDUAL SEALED TRUSS SHEETS

> NOTE: INSTALL MOISTURE BARRIER BETWEEN MASONRY & UNTREATED WOOD

IMPORTANT NOTE:

THIS FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY, TRUSS MANUFACTURER TO PROVIDE SEPERATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER AND REVIEWED BY P.E. OF RECORD.



TOTAL NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1 TO 300 PROVIDED THAT AT LEAST 50 % AND NOT MORE THAN 80 % IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED PER SECT. R806.2

TOTAL AREA TO BE VENTILATED = 1783 S.F. 1783/300 = 5.94 S.F. OR 855.36 SQUARE INCHES.

ROOF VENTS ARE RATED AT 36 SQUARE INCHES OF OPENING PER LINEAL FT. 855.38 S.I./36 S.1.  $\pm$  23.76 LINEAL FEET REQUIRED.

INSTALLATION FOR THIS ROOF IS 24' OF ROOF VENTING

HUNTERS RIDGE NEW PORT RICHEY

& ELK MODEL

16022

PLAN DATE

5CALE 1/8" = 1'-0'

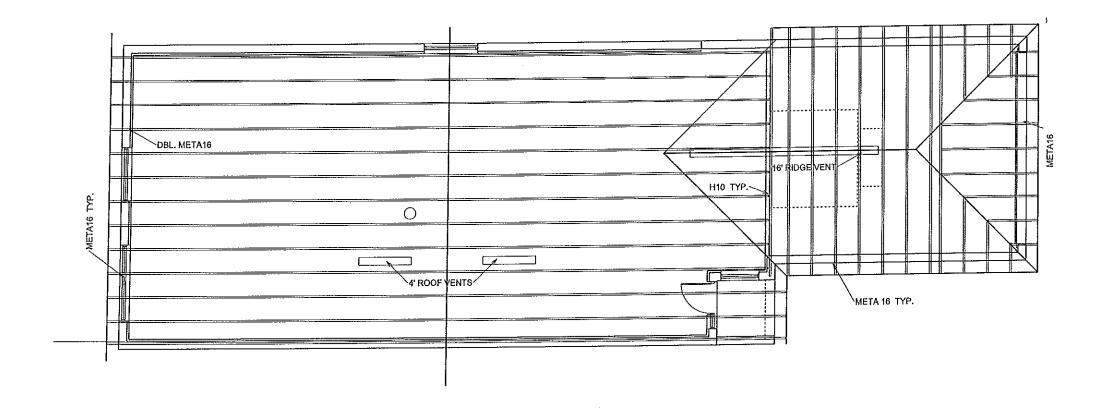
TRUSS PLANS

ALL TRUSS TO TRUSS CONNECTORS BY TRUSS SYSTEMS ENGINEER AND TO BE SPECIFIED ON INDIVIDUAL SEALED TRUSS SHEETS

> NOTE: INSTALL MOISTURE BARRIER BETWEEN MASONRY & UNTREATED WOOD

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TOTAL AREA TO BE VENTILATED = 1783 S.F. 1783/300 = 5.94 S.F. OR 855.36 SQUARE INCHES.

ROOF VENTS ARE RATED AT 36 SQUARE INCHES OF OPENING PER LINEAL FT. 855.36 S.I/36 S.1. = 23.76 LINEAL FEET REQUIRED.

INSTALLATION FOR THIS ROOF IS 24' OF ROOF VENTING

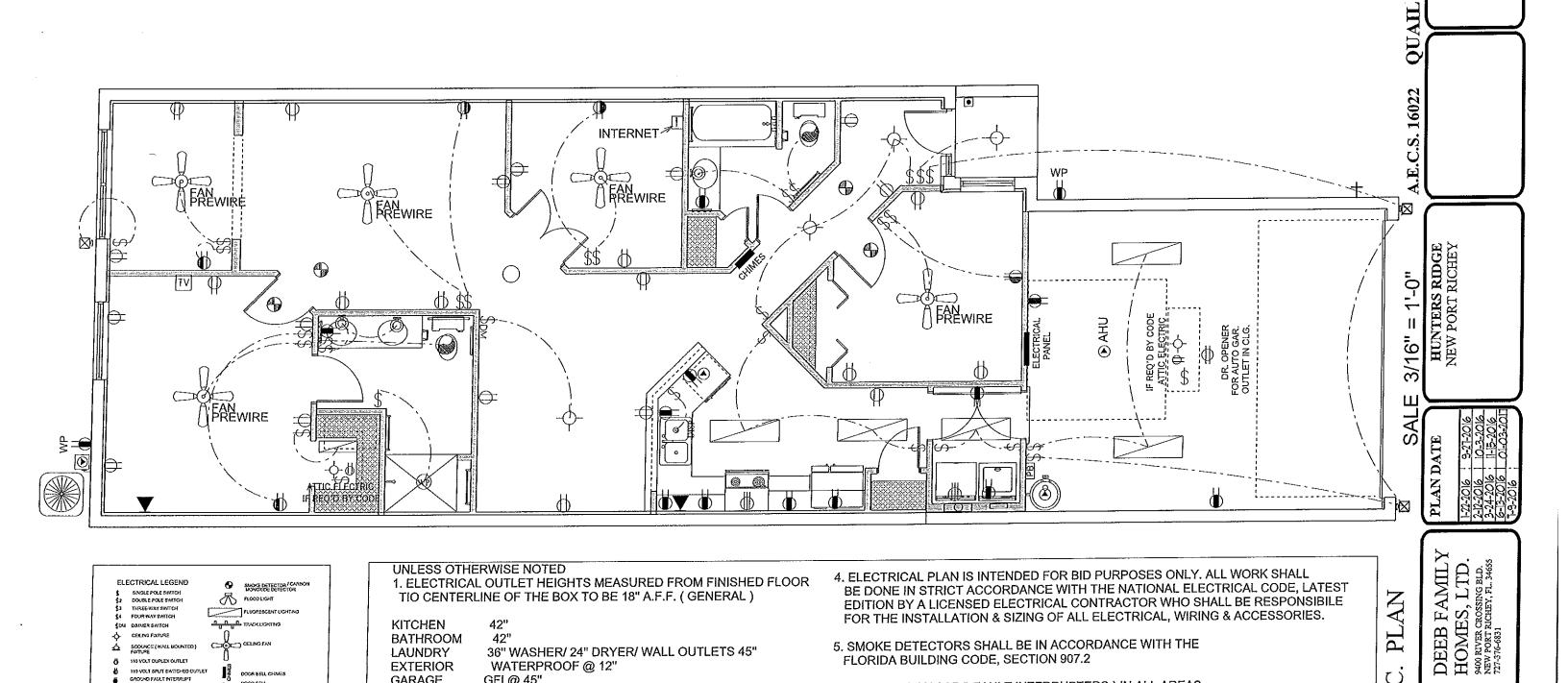
# ELK SINGLE UNIT TRUSS PLAN

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

SCALE 1/8" = 1-0"





5. SMOKE DETECTORS SHALL BE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, SECTION 907.2

6. PROVIDE AFCI ( ARC FAULT INTERRUPTERS ) IN ALL AREAS PER NEC, SECTION 210-12

7. ALL RECEPTICALS TO BE TAMPER PROOF PER SECT. 406.11

THREE-WAY SWITCH FOUR-WAY SWITCH

SCOUNCE (WALL MOUNTED)
FATURE

DOOR BELL CHIVES

DOOR BELL

JUNCTION BOX THERMOSTAT

LOW VOLTAGE LIGHT

Ш

1

SHO VOLT DUPLEX GUTLET

GROUND FAULT INTERRUPT

TELEPHOAS CASLE OUTLE

222 VOLT CUTLET

\$DM DAMER SWITCH CEILING FOOTURE KITCHEN

LAUNDRY

**EXTERIOR** 

**GARAGE** 

RANGE

**BATHROOM** 

36" WASHER/ 24" DRYER/ WALL OUTLETS 45"

WATERPROOF @ 12"

2. ALL TRIM PLATES AND DEVICES TO GANGED WHERE POSSIBLE

3. ELECTRICAL SWITCHES TO BE AT 42" CENTERLINE A.F.F.

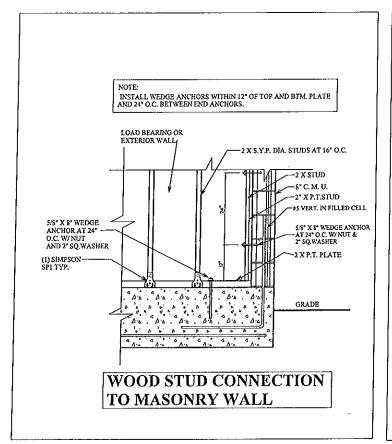
GFI @ 45" 220V @ 4"

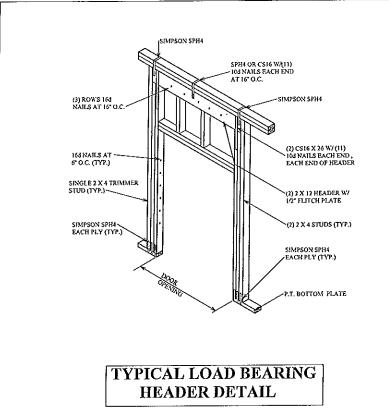
& ELK MODEL

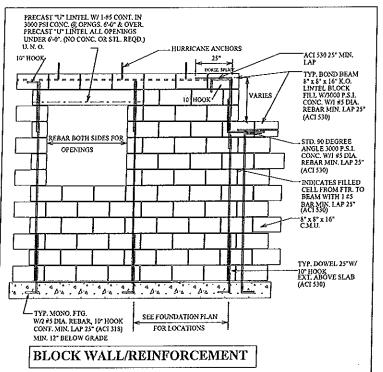
EC.

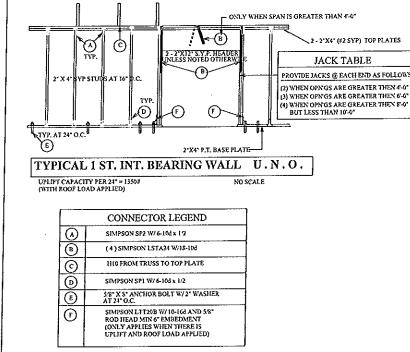
EL

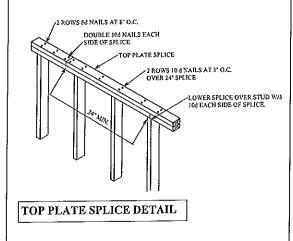
ELK

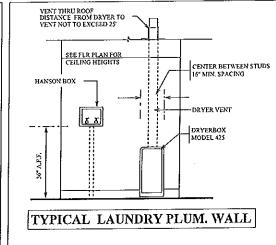


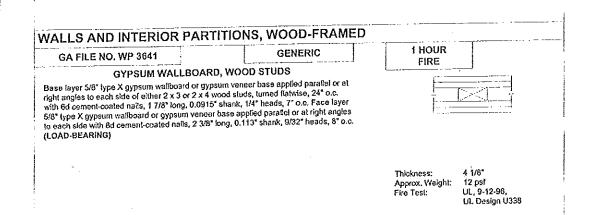


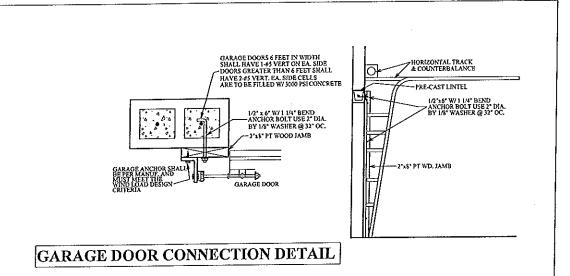














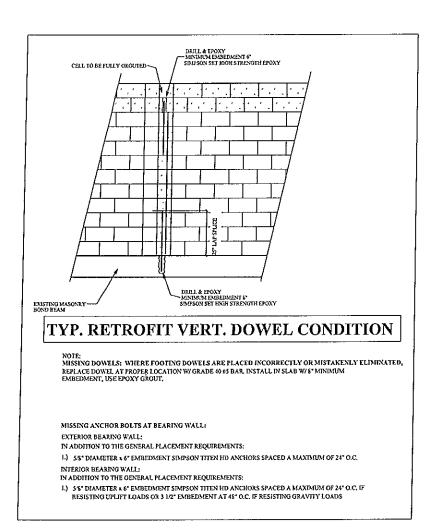
ALLEN ENGINEERING & CONSTRUCTION SERVIK RICH ALLEN PROFESSIONAL ENG 8899 SKYMASTER DR. NEW PORT RICHEY, FL. 34654 777-840-6100

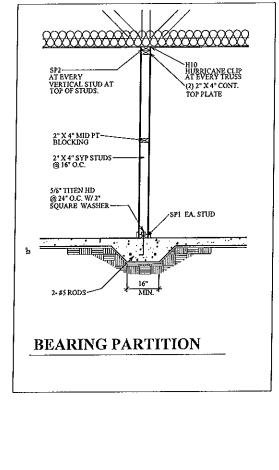
8

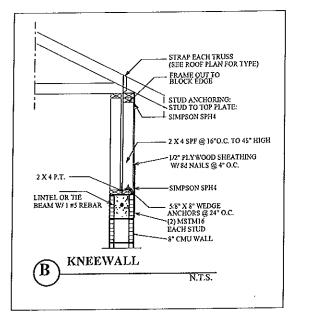
16022

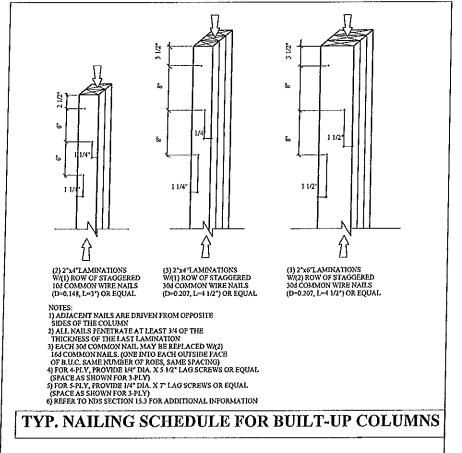
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HUNTERS RIDGE NEW PORT RICHEY









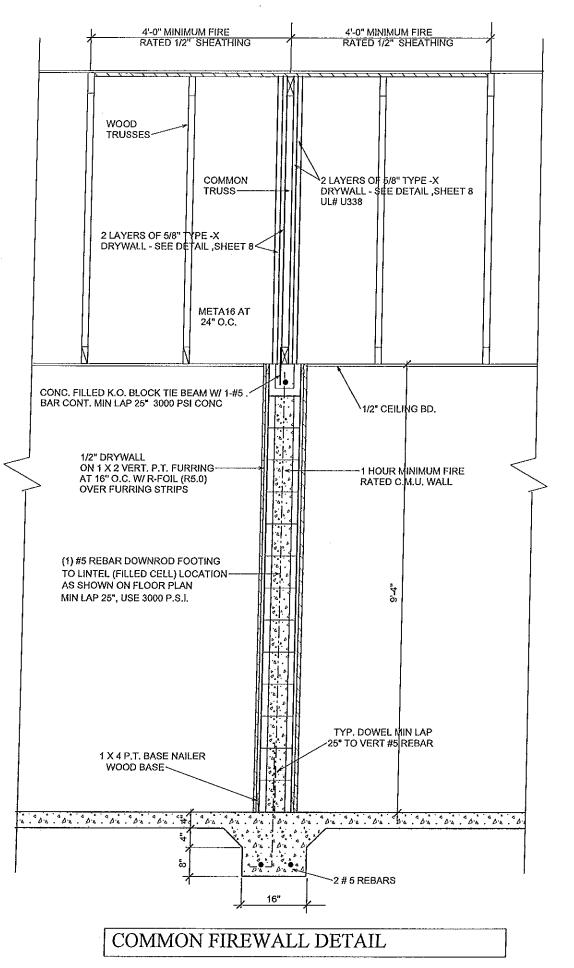


S

CONSTRUCTION DETAIL

QUAIL 16022 C.S.

& ELK MODEL



CONNECTOR TABLE R-38 INSULATION FLORIDA PRODUC NUMBERS PER INDEX 2-25-2011 DIMENSIONAL COMPOSITE SHINGLES INSTALL SIMPSON 10566.12 10456.10 10456.16 10456.6 11470.6 11470.7 PER MFGR. RECOMMENDATIONS, OVER MBHA3 56/11.88 #30 FELT MEMBRANE OVER H2 H10 LGT2 MGT LSTA18 LSTA24 SP1 SP2 7/16" OSB ROOF SHEATHING W/ CLIPS W/8d RINGSHANK @ 4" O.C. BOUNDRY AND 10352.4 EDGES AND 6" O.C. IN THE FIELD WITH A SETBACK 10852.4 10456.41 10456.42 OF 5' 0" FROM ALL EDGES. HTS20 HTS16 10456.23 10456.22 -SEE ROOF PLAN FOR STRAP TYPE 9'-4" TOP OF BLOCK META16 11473.17 L30 10446 11 SEE ELEVATIONS FOR WALL HEIGHTS MSTAM24 11473.19 1/2" CEILING BD. MSTAMS6 -ALUM, COVERED FASCIA MSTCM60 11473.19 C\$16 10852.1 1 X 4 P.T. NAILER 10456.46 SPH4 10456.47 1/2" DRYWALL, 5d NAILS 7" O.C. EDGES 12" O.C. FIELD 11496.2 2 X 6 SUB-FASCIA 11496.2 HTT5 10849.6 ABUSS -CONT. VENTED ALUM. SOFFIT CURTAIN NAILER 1 X 6 P.T. @ WINDOWS / 1 X 8 P.T. @ S.G.D. 8 d NAILS AT 4" O.C. SHEATHING TO BLOCKING CONC. FILLED K.O. BLOCK TIE BEAM W/ 1-#5 BAR CONT. MIN LAP 25" 3000 PSI CONC 1 X 4 P.T. @ SIDES & BOTTOM 2 X 4 BLOCKING AT 24" O.C. PRECAST "U" LINTEL, W/ I # 5 ROD IF OVER 6'0" SPAN U.N.O. 2 X 6 SUB-FASCIA (1) #5 REBAR DOWNROD FOOTING GABLE END TO LINTEL (FILLED CELL) LOCATION AS SHOWN ON FLOOR PLAN MIN LAP 25", USE 3000 P.S.I. ALUM FRAMED WINDOW W/ SCREEN WINDOW STOOL 1 X 2 VERT. P.T FURRING PRECAST FLUSH CONC SILL GABLE END OVERHANG AT 16" O.C.-R-FOIL -R-5.0 8 X 8 X 16 CONC. BLOCK **OVER FURRING STRIPS** TEXTURED FINISH ON CONCRETE BLOCK REFER TO ELEV SHEET 1 X 4 P.T. BASE NAILER -WOOD BASE 0'0" F.F. 5 Do . 40 . 40 . 40 . 40 . 40 GRADE TYP, DOWEL MIN LAP 25" TO VERT #5 REBAR SEE FOUNDATION PLAN 2 FOR FTG. REQ. TYPICAL WALL SECTION TERMITE SPECIFICATIONS: INSTALL "BORA-CARE" TERMITE PROTECTION SYSTEM PER MANUF. SPECIFICATIONS

ALLEN ENGINEERING & CONSTRUCTION SERVICES

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

FAMILY

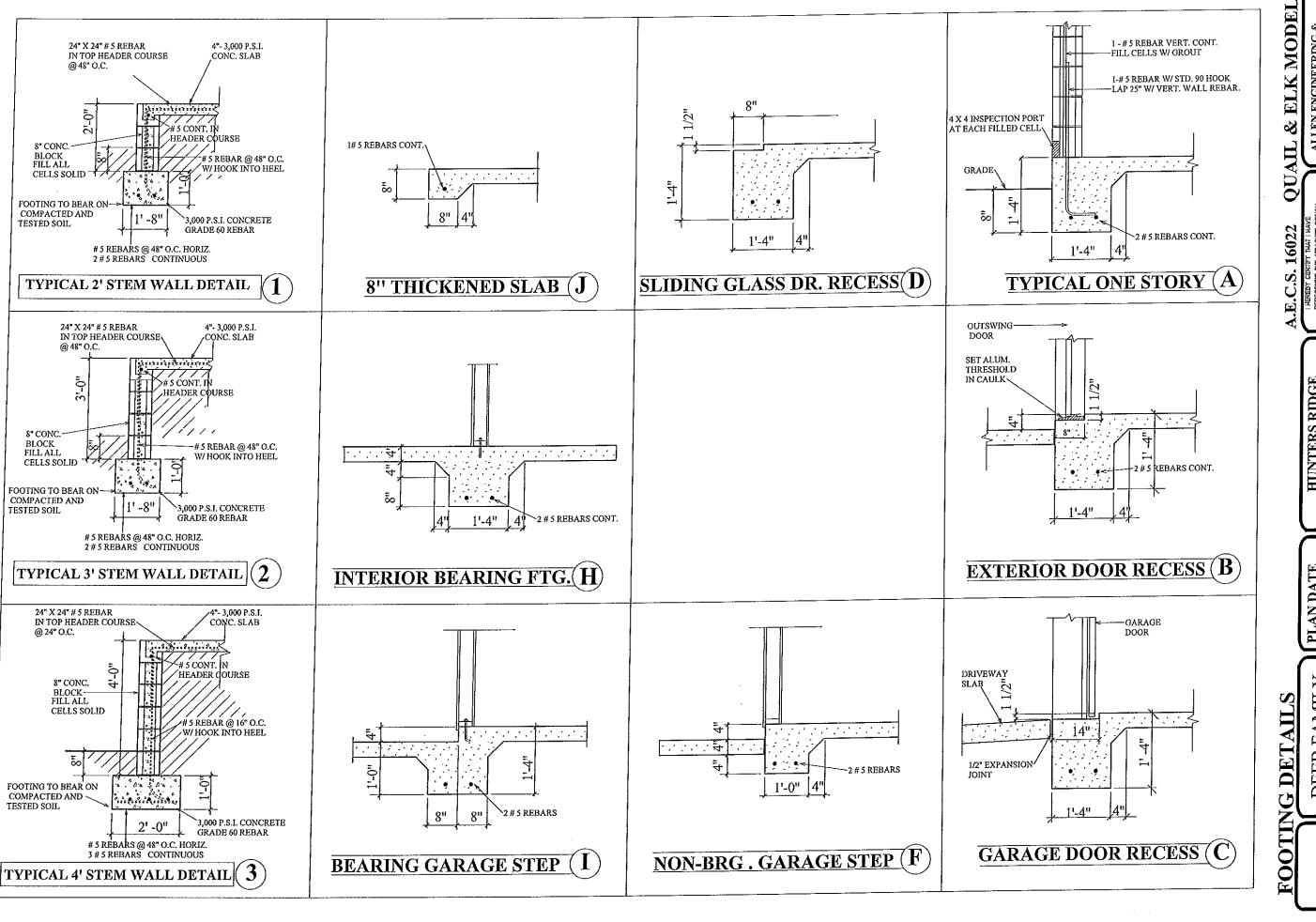
ES, LTD.

R CROSSING BLD.

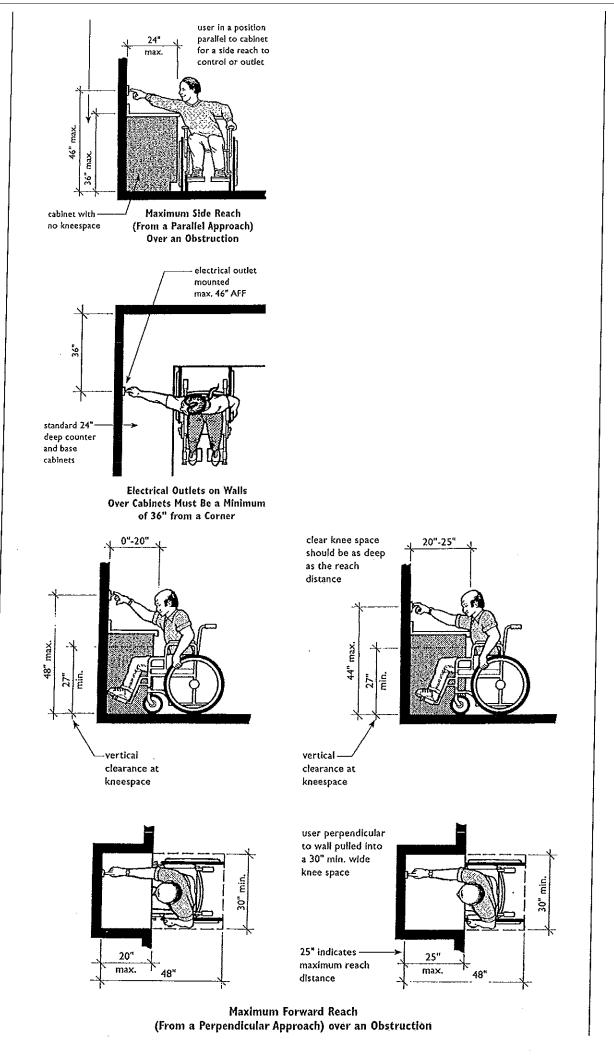
FRICHEY, FL. 34655

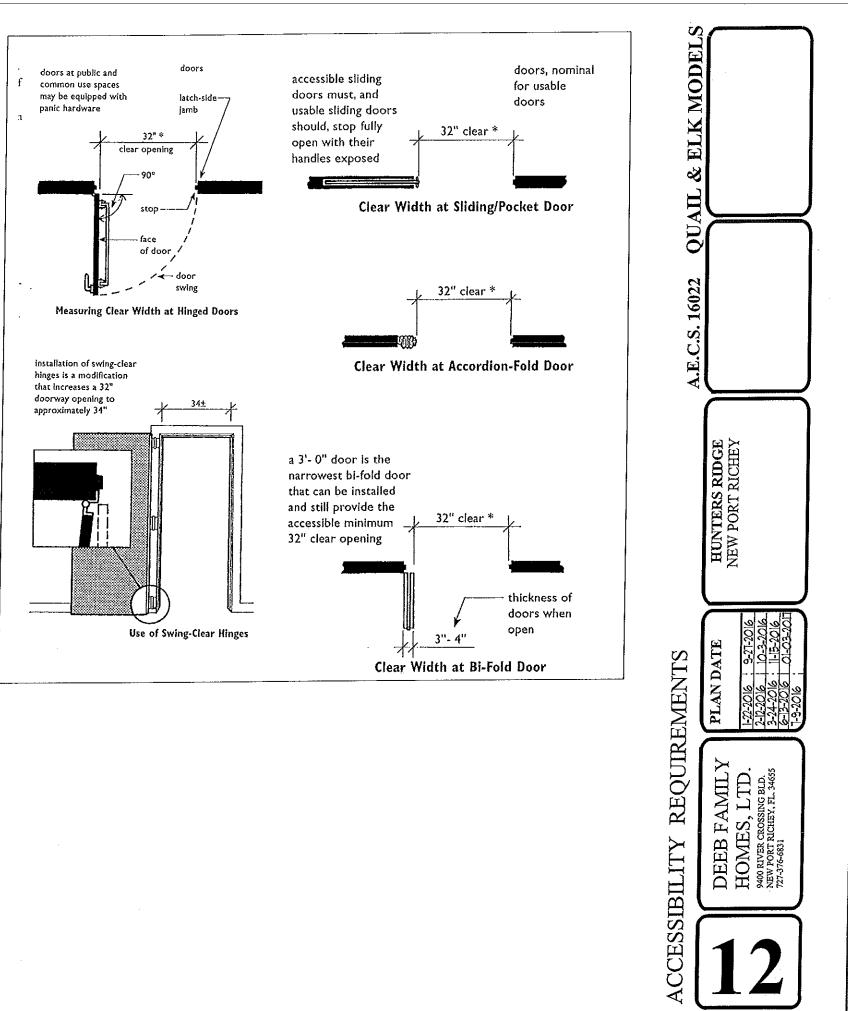
DEEB FA HOMES, I 9400 RIVER CROSSIN NEW PORT RICHEY, 727-376-6831

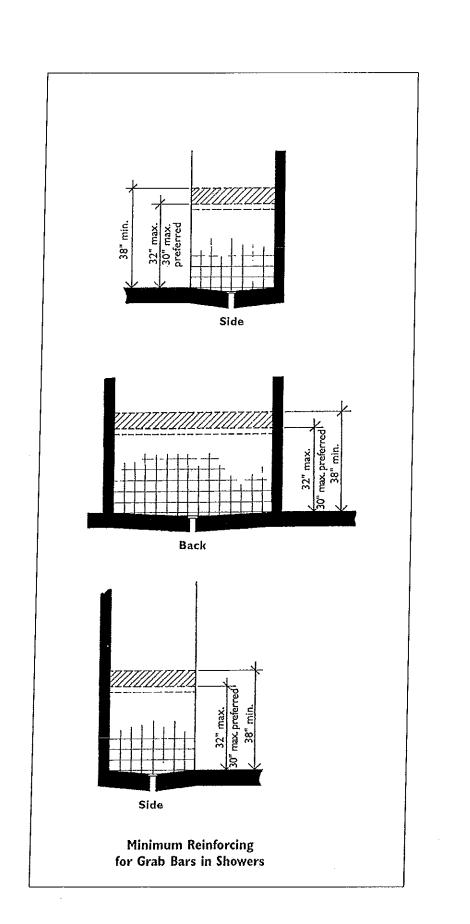
RICH ALLEN PROFESSIONAL E
P.E. # 56920 C.A. # 9542
8809 SKYMASTER DR.
NEW PORT RICHEY, FL. 34654

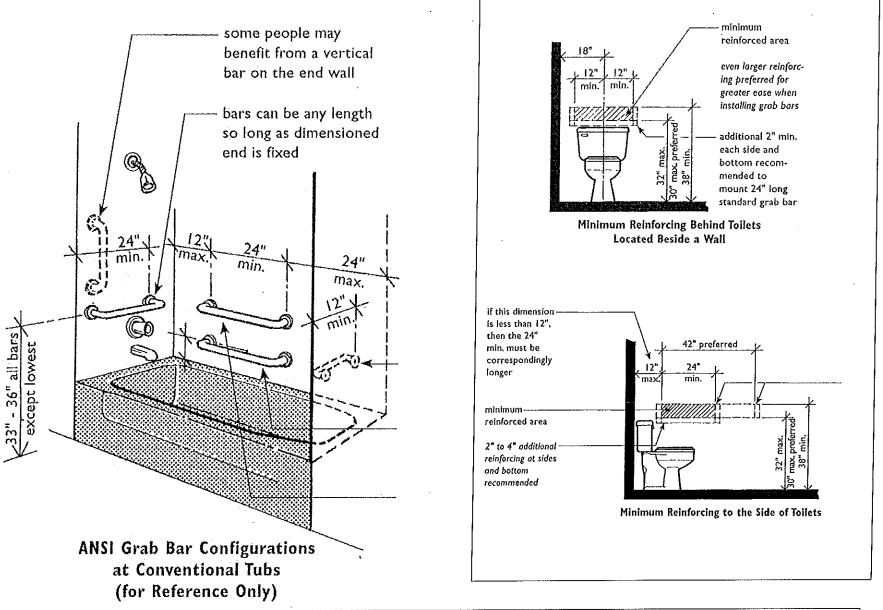


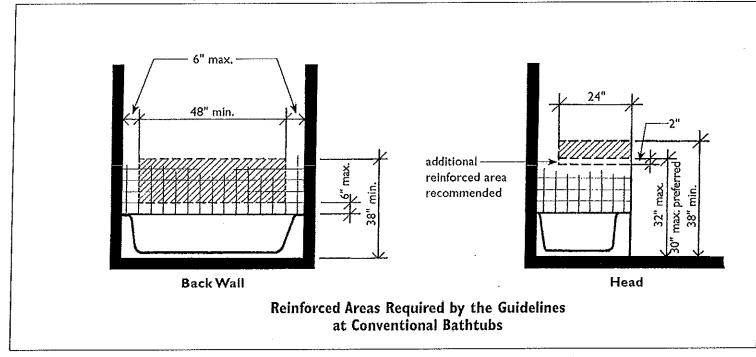
HUNTERS RIDGE NEW PORT RICHEY PLAN DATE DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831







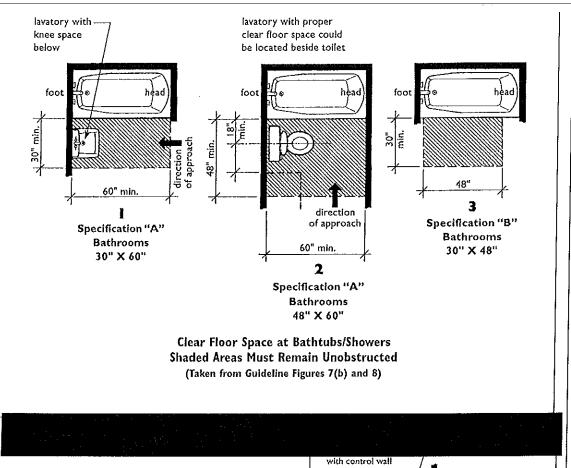




PLAN DATE REQUIREMENTS DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL 34655 727-376-6831 CCESSIBILITY

HUNTERS RIDGE NEW PORT RICHEY

ELK MODEI



30" mln. 36" mln.

**Guideline Requirements for Clear** 

Floor Space at Showers

36" X 48"

shower

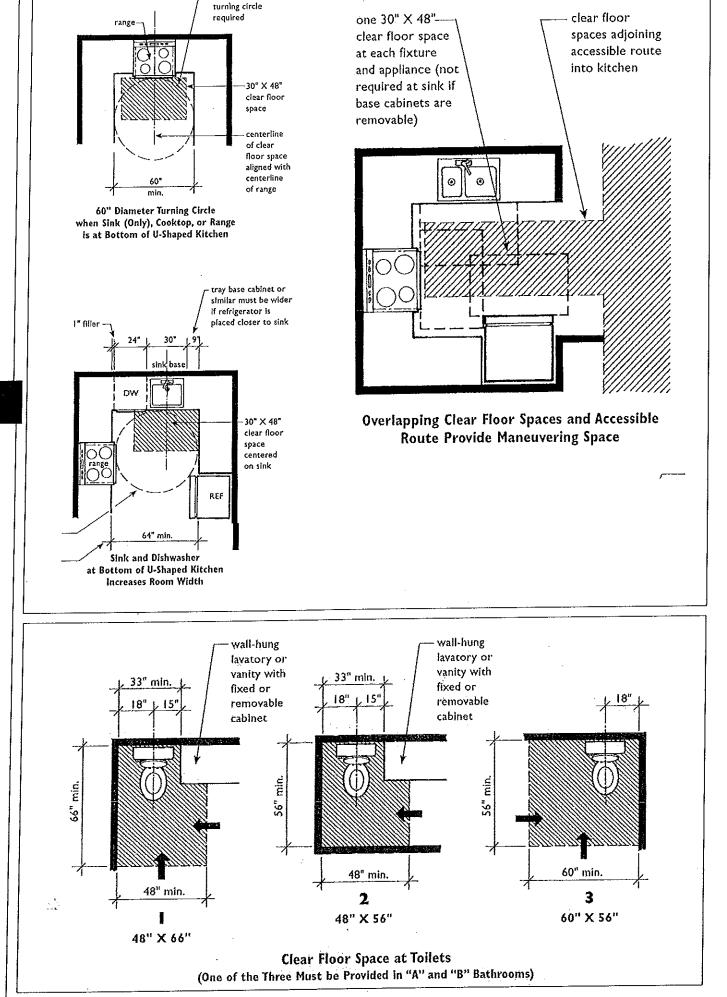
32" X 60"

Other Shower Sizes Meet the Requirements of the Guidelines

30" X 48" clear floor space flush with the control wall

30" X 48" clear floor

O space flush with the control wall



& ELK MODEL

A.E.C.S. 16022

HUNTERS RIDGE NEW PORT RICHEY

PLAN DATE

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

REQUIREMENTS

CCESSIBILITY

-60° diameter