### GENERAL NOTES:

THE FOLLOWING TECHNICAL CODES SHALL APPLY: 2017 FLORIDA BUILDING CODE. PLUMBING, MECHANICAL, FUEL GAS. ENERGY EFFICIENCY, ACCESSIBILITY, AND NATIONAL ELECTRICAL CODES

- 1. TANK TYPE WATER CLOSET VOLUME 1,6 GALLONS
- 2. WALL MOUNT WATER CLOSET VOLUME 3.5 GALLONS
- 3. WATER FLOW RATE. PUBLIC FACILITIES 0.5 G.P.M. PRIVATE FACILITIES 2.2 G.P.M. 2.5 G.P.M. SHOWER HEADS

VTR LOCATIONS ARE APPROXIMATE AND MAY CHANGE DUE TO JOBSITE CONDITIONS

THE FOLLOWING SHALL COMPLY WITH THE 2017 FBC.

- El PORCHES AND BALCONIES
- HANDRAILS
- C GUARDRAILS
- C) STAIRS
- CHIMNEY & FIREPLACE
- ☐ EGRESS WINDOWS
- I. ALL OPENINGS SHALL COMPLY WITH 2017 FBC WIND LOADS AS STATED BELOW, ATTACHMENTS OF WINDOWS, DOORS, SLIDING GLASS DOORS AND O.H. GARAGE DOORS ARE DELEGATED THE MANUFACTURER OF THESE ITEMS. THE MANUFACTURER OF THESE ITEMS SHALL SUBMIT ATTACHMENTS TO ENGINEER
  OF RECORD FOR REVIEW PRIOR TO INSTALLATION. SEE ATTACHED SPECIFICATION SHEETS FOR SEE ATTACHED SPECIFICATION SHEETS FOR MANUFACTURERS DESIGN CRITERIA AND INSTALLATION METHODS FOR WINDOWS, DOORS, SLIDING GLASS DOORS, OVERHEAD GARAGE DOORS, AND ROOFING.
- ALL DOORS INTERIOR & EXTERIOR ARE 8' 0" UNLESS OTHERWISE NOTED ALL SHOWER ENCLOSURES TO BE TEMPERED GLASS
- 6. ALL WINDOWS WITHIN 24" OF DOORS (INTERIOR & EXTERIOR) AND WITHIN 18' OFF FLR TO BE TEMPERED GLASS.



20012

ROBBIAN DESIGN
AL ROBBIAN A.I.B.D.
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KEN PORT BUHEY. EL. 24643
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MAIL-2453 (1000) 244-2459

TITLE SHEET

> COVER SHEET STRUCTURAL ENGINEER NOTES STRUCTURAL ENGINEER NOTES STRUCTURAL ENGINEER NOTES WIND LOAD DESIGN DATA **FOUNDATION PLAN FLOOR PLAN NOTES DIMENSION PLAN EXTERIOR ELEVATIONS ENTRY TOWER DETAILS EXTERIOR ELEVATIONS ROOF PLAN** TRUSS PLAN **ELECTRICAL PLAN**

CONSTRUCTION DETAILS

CONSTRUCTION DETAILS TYPICAL WALL SECTIONS TYPICAL FOOTING DETAILS

TYPICAL BATH DETAILS

SHEET COVER

RRIMROSE MODEL 2848

1901

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STUART RESIDENCE LOT 20

DUNEDIN COVE



DEEB FAMILY HOMES, LTD.
9400 RIVER CROSSING BLD.
NEW PORT RICHEY, PL. 34655
727-376-6831

PLAN DATE

**NOTICE TO SUBCONTRACTORS:** 

DUE TO SPACE LIMITATIONS IN THIS 11"X 17" PLAN

OR IN THE SAME LOCATIONS AS PROVIDED FOR BY

OTHER CONTRACTORS OR ARCHITECTS.

THE JOB BEFORE BEGINNING CONSTRUCTION.

READABILITY ISSUES, SOME DETAILS AND NOTATIONS

IT WOULD BE IN YOUR BEST INTREST TO REVIEW THESE

PLANS AND LOCATE THE APPROPORIATE INFORMATION

REQUIRED TO COMPLETE YOUR SPECIFIC PORTION OF

MAY OR MAY NOT BE LOCATED ON THE SAME SHEETS

FORMAT, AND TO ELIMINATE CLUTTER AND TEXT

**NOTICE TO BUILDER** 

IT IS THE INTENT OF THIS DESIGNER THAT THESE PLANS ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE LICENSED PROFESSIONA

TO CONSTRUCT THIS PROJECT. IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION..STOP. AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROFESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY REVIEW THESE DOCUMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE BEFORE ANY WORK IS DONE.

WINDOWS MUST BE FASTENED INTO STRUCTURAL MEMBERS PER MFG'S. DETAIL REQUIREMENTS PER DESIGN CRITERIA

WINDOW INSTALLATION NOTES:

NOTED ON THESE DRAWINGS. 2. WINDOWS ARE NOT IMPACT RESISTANT TYPE, STORM

WINDOWS ARE NOT IMPACT RESISTANT TITPE, STORM
SHUTTERS OR PANELS ARE REQUIRED,
ROOF, WALLS AND WINDOW FASTENINGS MUST BE
ENGINEERED AND SPECIFIED FOR CUMULATIVE INTERNAL
PRESSURE AND EXTERNAL NEGATIVE (SUCTION) PRESSURES
WHICH VARIES ACCORDING TO AREAS AS NOTED IN THE DESIGN
CRITERIA AS NOTED ON PAGE S4.

INDEX OF DRAWINGS

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**ELECTRICAL RISER SCHEMATIC** 

### STRUCTURAL ENGINEER DESIGN NOTES

- 1. THE ENGINEERING FIRM FOR THIS STRUCTURAL DESIGN IS ALLEN ENGINEERING AND CONSTRUCTION SERVICES, INC. HEREIN REFERRED TO AS " AECS OR " A.E.C.S.". 2. THE ENGINEER FOR THIS STRUCTURAL DESIGN IS
- RICHARD E. ALLEN, PE. HERBIN REFERRED TO AS "STRUCTURAL
- 3. THE STRUCTURAL ENGINEER DESIGN NOTES ARE PART OF THE STRUCTURAL DESIGN AND ARE TO BE TAKEN AS TYPICAL REQUIREMENTS UNLESS NOTED OTHERWISE, "UNO", IN THE STRUCTURAL PLANS AND STRUCTURAL DETAILS.
- 4. THE DESIGN SHOWN IN THESE PLANS CONFORM TO THE STRUCTURAL PROVISIONS OF THE CHAPTER IS OF THE FLORIDA BUILDING CODE, SECTION R301 OF THE FLORIDA RESIDENTIAL BUILDING CODE 2017, THE SECTIONS TITLED "STRUCTURAL" OF THE FLORIDA EXISTING BUILDING CODE 2017.
- THE PLOKION EXISTING BUILDING CONTROL OF THE PURPOSE OF THESE PLANS IS TO OBTAIN A BUILDING PERMIT AND FOR SUBSEQUENT CONSTRUCTION OF THE DESIGN AS SHOWN. THESE PLANS ARE TO BE CONSIDERED VOID IF WORK COMMENCES PRIOR TO A PERMIT BEING ISSUED, A CHANGE IN THE BUILDING CODE OCCURES PRIOR TO THE PLANS BEING SUBMITTED FOR PERMIT OR AFTER SIX MONTHS OF THE DATE THAT THESE PLANS ARE SIGNED AND SEALED WITHOUT BEING SUBMITTED FOR PERMITTING, WHICHEVER OCCURES FIRST. ONCE A BUILDING PERMIT HAS BEEN ISSUED BASED ON THESE PLANS, THE BUILDING DEPARTMENT IS NOT AUTHORIZED TO REISSUE OR TRANSFER BUILDING PERMITS WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.
- CONSTRUCTION BASED ON THE STRUCTURAL DESIGN IS TO BE DONE AS SHOWN IN THE PLANS WITHOUT DEVIATION.CHANGE OR OMISSION WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. IF ADDITIONAL DETAIL INFORMATION, OR EXPLANATION IS NEEDED, IT IS TO BE OBTAINED FROM THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ADDITIONAL PARTS OF THESE PLANS, INCLUDING PROVISIONS AS STATED IN ITEM 4.
- 7. IT IS IMPORTANT TO UNDERSTAND THAT STRUCTURAL PROVISIONS OF THE BUILDING CODE ARE COMPLICATED AND THESE PLANS ARE INTENDED TO BE USED BY AN EXPERIENCED BUILDING CONTRACTOR, PROPERTY OWNERS OBTAINING OWNER-BUILDER PERMITS ARE PROCEEDING AT THEIR OWN RISK. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY BRRORS OR OMISSIONS BY PROPERTY OWNERS OR THEIR AGENTS AS A RESULT OF ANY MISUNDERSTANDING OF THE PLANS THE OTHERWISE WOULD BE UNDERSTOOD BY A LICENSED CONTRACTOR.

  8. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR
- CONSTRUCTION MEANS, METHODS, AND SCHEDULE. 9. THE STRUCTURAL PLANS AND ANY RELEVANT DESIGN DOCUMENTS PRODUCED UNDER THE DIRECT CHARGE OF THE STRUCTURAL ENGINEER ARE THE PROPERTY OF THE STRUCTURAL ENGINEER ARE THE PROPERTY OF THE STRUCTURAL ENGINEER AND MAY NOR BE USED BY ANY PERSON OTHER THAN THE CONTRACTED CLIENT AND FOR ANY PURPOSE OTHER THAN THAN THAT STATED IN ITEM 5 ABOVE WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER. MOREOVER, NO OTHER ENGINEER OR ARCHITECT IS TO BE DESIGNATED A DELEGATED ENGINEER FOR ANY PURPOSE RELATED TO THESE STRUCTURAL PLANS OR CONSTRUCTION BASED ON THESE PLANS PRIOR TO THE ISSUANCE OF A

### DESIGN CRITERIA

10. LOAD COMBINATIONS: THIS DESIGN IS BASED ON AN \*ALLOWABLE -STRESS \* FORMULATION RELYING ON THE LOAD COMBINATIONS DEFINED IN FBC 2017 SECTION 1605.3.1 OR SECTION 1605.3,2 WHERE OMEGA EQUALS 1.3 11. FOUNDATION LOADS: SEE NOTES ON "SITE CONDITIONS.

CERTIFICATE OF COMPLETION OR OCCUPANCY WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

SOILS, AND FOUNDATIONS'.

12. FLOOR LIVE LOADS:

RESIDENTIAL ONE AND TWO STORY FAMILY DWELLINGS: ALL LIVE LOADS PER TABLE R301.5 ENINHABITABLE ATTICS WITHOUT STORAGE: 10 PSF UNINHABITABLE ATTICS WITH STORAGE: 20 PSI

HABITABLE ATTICS AND SLEEPING AREAS: 30 PSF BALCONIES: 60 PSF

DECKS: 40 PSF

ALL OTHER ROOMS 40 PSP

GUARDRAILS /HANDRAILS :200PSF CONCENTRATED LOAD APPLIED IN ANY DIRECTION.

13. INFORMATION CONTAINED ON A PLAN SHEET WHERE HIS SIGNATURE AND SEAL APPEAR, THAT DOES NOT PERTAIN TO THE RELEVANT STRUCTURAL PROVISIONS AS STATED IN ITEM 4, INCLUDING, BUT NOT LIMITED TO THE BUILDING OCCUPANCY, THE ARCHITECTURAL DESIGN, IT'S FEATURES, FINISHES (LE, DECORATIVE STUCCO, SIDING, ROOFING SOFFITS, FLASHING, PAINTING, ETC ) AND THEIR INSTALLATION, DIMENSIONS, AND ANY DESIGN OF FIRE PROTECTION, ELECTRICAL PLUMBING, AND MECHANICAL COMPONENTS OR SYSTEMS.

THE ARCHITECTURAL INFORMATION, INCLUDING DIMENSIONS SHOWN IN THESE PLANS AND PROVIDED TO THE ENGINEER.

### SITE CONDITIONS

18. SITE PLAN AND TOPOGRAPHY
A. THE STRUCTURAL ENGINEER IS NOT A SUVEYOR AND IS NOT RESPONSIBLE FOR THE SITE PLAN, ESTABLISHING REQUIRED BET-BACKS, AND LOCATING THE BUILDING ON THE PROPERTY.

B. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE
GRADING OF THE SITE OR ITS COMPLIANCE WITH ANY DRAINAGE
PLAN WHETHER INDIVIDUAL OR AS A PART OF A MASTER

C. THE FOUNDATION DESIGN IS BASED ON THESE PRESUMED CONDITIONS INCLUDING THAT DIFFERENTIAL SETTLING IXXES NOT EXCEED THE SAFE LIMITS OF THE FOUNDATION DESIGN

O I EAGEBLI THE SAPE LIMITS OF THE FOUNDATHON DESIGN (INCLUDING STEMWALLS AND MASONRY AROVE GRADE WALLS) AS STATED IN ITEM 19 BELOW, D. IT IS IMPORTANT TO KNOW THAT THE FOUNDATION DESIGN BASED ON A PRESUMED ALLOWABLE SOIL BEARING CAPACITY OP 2,000 PSF RELIES ON LESS THAN L/500 (E.G.,0.25 INCHES OVER 16 PEET ) OF DIFFERBNTIAL SETFLEMENT, CRACKS IN MASONRY WALLS SHOULD BE EXPECTED WHERE DIFFERENTIAL SETTLEMENT WALLS SHOULD BE EXPECTED WHERE DIFFERENTIAL SETTLEMENT EXCEEDS L/150. FHIS STATEMENT SHOULD BE TAKEN AS A CAUTIONARY NOTE FOR PROCEEDING WITHOUT A SOILS ANALYSIS AND FOUNDATION RECOMMENDATION BY A GEOTECHNICAL ENGINEER FOR THE SITE.

E. COPIES OF ANY AND ALL REQUIRED COMPACTION TESTS ARE TO BE PROVIDED TO THE BUILDING DEPARTMENT FOR THEIR

STRUCTURAL ELEMENTS
19. FOUNDATION, FOOTING AND GROUND FLOOR SLAB
A. THE FOUNDATION AND FOOTINGS ARE TO BEAR A MINIMUM
ON 12 INCHES BELOW GRADE AND ARE TO BE PLACED ON UNDISTURBED SOIL OR FILL COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR PURSUANT TO ASTM D 1557 WITH FILL LIFTS LESS THAN 12".

ALL LIVETOADS PER FBC 2017 TABLE 1607.1
14. ROOF LIVE LOADS:
ALL ROOF / WOOD CONSTRUCTION TYPES ARE 30 PSF. 15. DHAD LOADS:

FLOOR WOOD FRAME: 35 PSF FOR THE/MARBLE FLOOR COVERING, 15 PSF FOR ALL OTHERS.

ROOF WOOD FRAME: 25 PSF FOR SHINGLES, 35 PSF FOR TILE 16. WIND LOADS:

A. WIND LOADS ARE BASED ON THE SPECIFIC REQUIREMENTS AND DEFINITIONS OF FLORIDA RESIDENTIAL BUILDING CODE 2017 EDITION ASCE-7-10.

B. THE COMPONENT AND CLADDING WIND PRESSURES ARE THE MINIMUM REQUIREMENTS FOR STRENOTH AND IMPACT PROTECTION NEEDED FOR SELECTING SATISFACTORY COMPONENTS AND CLADDING, BY OTHERS, FOR THE STRUCTURE.

ENGINEERING BY OTHERS IS PRESUMED ACCURATE AND IS RELIED UPON BY THE STRUCTURAL ENGINEER SOLEY FOR THE PURPOSE OF ACHIEVING COMPLIANCE WITH THE RELEVANT STRUCTURE

20, MIX DESIGNS FOR ALL CONCRETE USED IN THE CONSTRUCTION OF SLAB - ON - GRADE FLOORS SHALL SPECIFY A MINIMUM DESIGN STRENGTH OF 3,000 PSI (20.7 MP4) AT 28 DAYS AND A DESIGN SLUMP NOT TO EXCEED 4 INCHES (102 mm). ON-SITE SLUMPS SHALL NOT EXCEED 5 INCHES (122 mm), PROVIDE TOTAL WATER ADDED TO THE MIX INCLUDING PLANT, TRANSIT AND SITE ADDED WATER DOES NOT EXCEED THE FOLLOWING PARAMETERS:

I. FOR MIXES USING NATURAL SANDS: 275 POUNDS PER CUBIC YARD

2. FOR MIXES USING MANUFACTURED SANDS: 292 POUNDS PER CUBIC YARD (35 GALLONS -132L.)

A. IN ADDITION, THE STRUCTURAL ENGINEER IS NOT A CIVIL OR GEOTECHNICAL ENGINEER AND IS NOT RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SITE FOR CONSTRUCTION, INCLUDING ITS TOPCORAPHY, DRAINAGE AND SUB-SURFACE CONDITIONS (INCLUDING WATER TABLE DEPTH) AND FOR INTERPRETING GEOTECHNICAL DATA CONCERNING THE SITE. IL IF SOIL CONDITIONS AT THE SITE APPEAR QUESTIONABLE AS DETERMINED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER, A SOILS ANALYSIS SHALL BE PERFORMED BY A LICENSED GEOTECHNICAL ENGINEER THAT WILL GIVE SPECIFIC RECOMMENDATIONS FOR A FOUNDATION TYPE, IF THE BUILDING CONTRACTOR OR OWNER-BUILDER DO NOT MAKE THAT DETERMINATION AND A SOILS ANALYSIS IS NOT PERFORMED, THE STRUCTURAL ENGINEER SHALL PROCEED WITH THE DESIGN BASED ON THE PRESUMPTIONS ALLOWED BY THE FBC 2017, SEC. 1801. C. THE DETERMINATIONS OF THE SUITABILITY OF THE SITE FOR CONSTRUCTION (INCLUDING TOPOGRAPHICAL INFORMATION) AND THE SOIL CONDITIONS SHALL HAVE BEEN COMPLETED AND AND THE SUIL CONDITIONS SHALL HAVE BEEN COMPLETED AND ANY RECOMMENDATIONS RESULTING FROM THAT ANALYSIS SHALL HAVE BEEN PROVIDED TO THE STRUCTURAL ENGINER PRIOR TO THE SIGNING AND SPALING OF THE STRUCTURAL PLANS.

D. IN THE ABSENCE OF GEOTECHNICAL INFORMATION, THE SITE IS PRESUMED TO HAVE AN ALLOWABLE SOIL BEARING CAPACITY OF 2000 PSF AND THE TOPOGRAPHY AS IT RELATES TO THE STRUCTURE IS PRESUMED TO BE THAT SHOWN IN THE PLANS E. THE SIZE AND REQUIRED REINFORCEMENT FOR THE FOOTINGS ARE SHOWN ON THE FOUNDATION PLAN. THE GROUND FLOOR SLAB SHALL BE PLACED OVER A 6 MIL. POLYETHYLENE MOISTURE RETARDER.

I. THE TRUSS SYSTEM DESIGN PROVIDED IN THIS PLAN IS FOR THE USE OF THE TRUSS MANUFACTURER IN DEVFLOPING THE ACTUAL ROOF TRUSS SYSTEM DESIGN. IT IS NOT TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL DESIGN.

II. MANUFACTURED FLOOR TRUSSES SHALL BE DESIGNED BY
A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER
ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.

BUILDING CONTRACTOR.

III. THE MANUFACTURED TRUSS DESIGN SHALL INCLUDE
SPECIFYING THE TRUSS TO TRUSS AND TRUSS TO GIRDER
CONNECTIONS ON EITHER THE INDIVIDUAL TRUSS COMPONENT
SHEETS OR THE GIRDER TRUSS COMPONENTS SHEETS AS
APPLICABLE. A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SBALED COMPONENT SHEETS FOR EACH LOCATION THAT A HANGER IS REQUIRED IN THE

INCOSS STATEM.

IV. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION ON THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED UPON THE FINAL PLOOR TRUSS

F. CONVENTIONAL FRAMED JOISTS WITH A MINIMUM 6 INCII OVERLAP OF JOINTS.

G. TERMITE TREATMENT OF THE SITE SHALL BE SPECIFIED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER. H. SHRINKAGE CONTROL OF THE FLOOR SLAB SHALL BE ACCOMPLISHED BY 6 INCH BY 6 INCH. W L4 BY L4 WELDED WIRE FABRIC AS SPECIFIED BY FBC 2017 SECTION 1910 2

EXCEPTION 2 OR PINERMESH ADMIXTURE AS SPECIFIED BY
FBC 2017, SECTION 1910.2 EXCEPTION 1. THE WELDED WIRE
FABRIC SHALL BE PLACED BETWEEN THE MIDDLE AND UPPER 1/3 DEPTH OF THE SLAB AND HELD IN POSITION BY APPROPIATE SUPPORTS SPACED NOT OREATER THAN 3 FERT APART. I. CONTRACTION JOINTS ARE TO BE PROVIDED FOR THE PURPOSE OF CONTROLLING SHRINKAGE ONE INCH DEEP CUTS FOR A FOUR INCH THICK SLAB OR 25 PERCENT OF THE SLAB THICKNESS OTHERWISE) ARE TO BE PROVIDED ACROSS THE WIDTH AND LENGTH OP ANY FLOOR SLAB AT A DISTANCE OF NOT TO EXCEED 30 TIMES THE SLAB THICKNESS. FOR EXAMPLE A FOUR INCH THICK SLAB, CONTRACTION JOINTS SHALL NOT EXCEED 10 FEET ON CENTER FACH WAY, THE CONTRACTION JOINTS ARE OPTIONAL FOR ONE AND TWO STORY FAMILY RESIDENTIAL WHEN WELDED WIRE FABRIC OR FIBERMESH ARE USED IN THE FLOOR SLAB.

## RRIMROS MODEL

19016 Ś E

ALLEN ENGINEERING & CONSTRUCTION SERVIC RUCH ALLEN PROFESSIONAL ENGINEERS

STUART RESIDENCE LOT 20 LOT 20 DUNEDIN COVE

PLAN DATE

NOTES

STRUCTURAL ENGINEER

DEEB FAMILY HOMES, LTD.

SERVICE CROSSING BLD.
NEW POOR UNDER CROSSING BLD.
NEW POOR UNDER CROSSING BLD.
TOTAL PROPERTY. PL. 24655
TOTAL PROPERTY. PL. 24655

A. MANUFACTURED FLOOR TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF

- ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPANY.

  1. PLOOR JOISTS ARE SIZED BASED ON THE SOUTHERN PINE COUNCIL SPAN TABLES FOR NO. 2 GRADE DIMENSIONAL LUMBER.
- II FLOOR JOISTS FOR EXTERIOR DECKS SHALL BE PRESSURE TREATED.
- B. FOR ALL WOOD FLOORS:
- 1. THE TRUSS TO WALL CONNECTIONS ARE IDENTIFIED
- ON THE FLOOR FRAMING PLAN.

  11. A STRUCTURAL BAND JOIST IS TO BE PROVIDED ON THE EXTERIOR PERIMETER OF ALL BOTTOM BEARING FLOOR TRUSSES AND JOISTS.THE STRUCTURAL BAND JOIST IS TO BE FASTENED TO EACH END OF A FLOOR TRUSS OR JOIST WITH A SIMPSON L50 BRACKET USING SIMPSON SHORT 10d COMMON NAILS.
- III. FLOOR TRUSSES OR JOISTS BEARING ON WOOD WALLS
  ARE TO BE SET WHIT A MENIMUM OF THREE IO! COMMON
  NAILS, (FOR NAILED) TO THE TOP PLATE OF THE WALL.
  IV. A MOISTURE BARRIER SHALL BE INSTALLED BETWEEN ANY
  UNTREATED WOOD TRUSSES OR JOISTS AND CONCRETE
- OR ANY MASONRY.
- V. LEDGERS MAILERS SHALL BE PASTENED TO WOOD STUDS OR BAND JOISTS (NOT SHEATHING) WITH A MINIMUM 2 3:8" X 5 1:2" LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AT 16 INCHES ON CENTER AND SHALL CONSIST OF PRESSURIE TREATED LUMBER 2 PLY 1 1/2" THICK BY A HEIGHT SHOWN IN THE PLANS, FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8" X 5 1/2" SIMPSON TITEN HEAD CONCRETE BOLTS.
- VI. FLOOR BEAMS
- BEAMS SUPPORTING FLOOR TRUSSES AND JOISTS ARE TO BE ATTACHED AS SPECIFIED IN THE FLOOR FRAMING PLAN. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.
- 3. MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (LE. GLULAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS
- 4. MILTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAYF INDIVIDUAL PLIES INTERCONNECTED AS FOLLOWS:
  A. FOR TWO PLY BEAMS- ONE ROW OF 10d GALVANIZED COMMON NAILS AT 6" O.C. ON EACH SIDE OF THE BEAM
- B. FOR THREE PLY BEAMS. TWO ROWS OF 16J GALVANIZED COMMON NAILS SPACED AT 6" O.C. (TOP AND BOTTOM) THRU
- C. FOR FOUR PLY BEAMS OR LARGER-TWO ROWS OF 1/2" DIAMETER CARRIAGE BOLTS OR ALL THREAD ROD WITH NUTS AND WASHERS SPACED AT 12 INCHES ON CENTER, 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM.
- D. FLOOR SHEATHING: ALL FLOOR SHEATHING IS TO BE 3/4" TONGUR AND GROOVE PLYWOOD RATED FOR FLOOR SHEATHING
- IL FLOOR SHEATHING SHALL BE FASTENED TO THE FLOOR TRUSSES JIOISTS WITH 101 RING SHANK NAILS AT 6" ON CENTER WITH CONSTRUCTION GRADE ADHESIVE. III. FLOOR SHEATHING SPECIFIED FOR SEALED EXTERIOR
- DECKS AND ITS INSTALLATION SHALL BE THE SAME AS THAT FOR INTERIOR APPLICATION EXCEPT PRESSURE TREATED AND THE PASTENERS TO BE GALVANIZED. E. FXTERIOR DECK FLOORING:
- DECK FLOORING SHALL BE INDIVIDUALLY SPECIFIED ON THE FLOOR FRAMING PLANS AND SHALL BE FASTENED TO THE UNDERLYING PRESSURE TREATED JOISTS WITH 3-3 INCH DECK SCREWS AE EACH FLOORING JOIST INTERSECTION.

22. WALLS: A. MASONRY

- I. CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI.
- II. WALL CMU SHALL BE 8 INCH X 16 INCH IN SIZE OR 8 INCH X 8 INCH X 8 INCH FOR EDGE FINISHES.
- III. CMU SHALL BE PLACED IN A RUNNING BOND AND THERE SHALL BE NO VERTICAL BUTT JOINTS EXCEPT AS SHOWN ON THE FLOOR PLAN FOR CONSTRUCTION JOINTS.

  IV. REINFORCED FILLED CELLS AS SHOWN ON THE PLANS SHALL BE FILLED WITH "FINE" GRADE GROUT, HAVE A MINIMUM COMPRESSION OF THE PLANS SHALL BE FILLED WITH "FINE" GRADE GROUT, HAVE A MINIMUM COMPRESSION OF THE PLANS SHALL BE FILLED WITH "FINE" GRADE GROUT.
- COMPRESSIVE STRENGTH OF 3,000 PSI AND 8 TO 11 INCH SLUMP TO ENSURE CONSOLIDATION.
- V. BOND BEAMS SHALL BE POURED WITH GROUT MONOLITHICALLY
  WITH THE FILLED WALL CELLS-NO COLD JOINTS.
  VI, VERTICAL STREL REINTORCEMENT SHALL BE CONTINUOUS
  BETWEEN THE MIDDLE AND BOTTOM 1/3 OF THE FOOTING HEIGHT
  AND END IN THE TOP COURSE OF THE BOND BEAM WITH A STANDARD 10 INCH 90 DEGREE BEND.
- VIL HORIZONTAL REINFORCING STEEL SHALL BE CONTINUOUS, INCLUDING
- VIII. REINFORCING STEEL SPLICES SHALL CONSIST OF WIRE LAPS NO LESS
  THAN 40 TIMES THE STEEL BAR DIAMETER (LE. 25 INCHES FOR \$5 REBAR,
  15 INCHES FOR \$3 REBAR, AND \$2 INCHES FOR \$7 REBAR.)
  B. WOOD FAME WALLS:
  1. WALL STUD SIZES ARE SHOWN IN THE TYPICAL WALL SECTION.

- 1. WOOD STUDS IN WALLS SHALL BE SPACED 16 INCHES ON CENTER AND PASTENED TO THE TOP AND BOTTOM PLATES PER THE TOP PLATE SPLICE DETAIL, ALL LOAD BEARING STUDS TO BE SOUTHERN YELLOW PINE #2
- 2. LOAD BUARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED) IN CONTACT WITH MASONRY OR CONCRETE. SEE THE TOP PLATE SPICE DETAIL FOR TOP PLATE NAILING AND SPLICING REQUIREMENTS.
- 3. THE WOOD STUDS SHALL HAVE A SIMPSON SP2 AT THE TOP PLATE AND A PROPERLY SIZED SPH FOR THE BOTTOM PLATE (I.E. 4" STUD WALL = SPH4.
- 6-STOD WALL ASPIG)

  4. J STOD FACK SHALL BE INSTALLED DIRECTLY BENEATH BEARING POINTS
  OF ALL GIRDERS AND BEAMS HAVING A GRAVITY LOAD OF UP TO 3,000 LBS.

  5. STEEL TUBE COLUMNS SHALL BE INSTALLED IN THE WALL DIRECTLY BENEATH
  GIRDERS AND BEAMS HAVING GRAVITY LOADS GREATER THAN 3000 LBS.
- 6. BASE PLATES SHALL BE FASTENED TO MONOLITHIC FOOTINGS WITH 5/8" X 8 INCH ANCHOR BOLTS OR SIMPSON TITEN HD. CONCRETE BOLTS
- OF THE SAME SIZE AT 24 INCHES ON CENTER. ALL CONNECTIONS SHALL BE MADE WITH 3 INCH SQUARE BY 1/8 INCH THICK WASHERS 7. BASE PLATES BEARING ON WOOD SHALL BE FASTENED WITH 166 COMMON NAILS AT 8" O.C., THROUGH ANY FLOOR SHEATHING AND TO UNDERLYING LUMBER (NOT SHEATHING ONLY ) AND USE BLOCKING AS NEEDED TO MAINTAIN NAILING SPACING REQUIREMENTS.
- 8. FOR EXTERIOR LOAD BEARING WALLS, EACH STUD ABOVE THE BASE PLATE SHALL BE FASTENED TO THE UNDERLYING BAND JOIST OR BEAM WITH A SIMPSON LSTAIR STRAP FOR THIS SITUATION THE SIMPSON SPH BRACKET
- TO THE BASE PLAN MAY BE OMITTED. 9. FOR INTERIOR LOAD BEARING WALLS, 1/2 INCH ALL THREAD ROD SHALL BE INSTALLED AT 32" O.C. FROM THE BASE PLATE THROUGH THE SHEATHING AND TOP PLATE OF UNDERLYING SUPPORTING WALL. ALL CONNECTIONS SHALL INCLUDE A STANDARD 3 INCH SQUARE WASHER.
- IO. HEADER BEAMS SHALL BE SIZED ACCORDING TO THE ENCLOSED HEADER SCHEDULE AND FASTENED WITH A MINIMUM OF TWO SIMPSON LSTA36 STRAPS OVER EACH END TO THE JACK STUDS BELOW. IN ADDITION, THE HEADER BEAMS SHALL BE FASTENED WITH A MINIMUM OF 3-10d COMMON MAILS ( TOE NAILED ON EACH FACE SIDE AT FACH END TO THE ABUTTING
- FULL LENGTH STUDS.

  III. NON LOAD BEARING WALLS:

  I. WOOD STUDS IN WALLS SHALL BE SPACED AT 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES WITH A MINIMUM OF THREE 10d COMMON NABLS, NABLS INSTALLED IN PRESSURE TREATED WOOD SHALL
- 2. INCIDENTAL, NON STRUCTURAL FRAMING ITEMS SUCH AS KNEE WALLS, DROP CEILINGS , BUILT IN SHELVING , NICHES, ETC. MAY BE CONSTRUCTED WITH 2 X 4'S AT 24' O.C. AT THE DISCRETION OF THE BUILDER.

2. NON LOAD BRARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED AGAINST MASONRY AND CONCRETE) AND A SINGLE TOP PLATE.

3. BASE PLATES SHALL BE FASTENED TO CONCRETE SLABS WITH 1/4 INCH BY 3 1/2 INCH TAPCON SCREWS AT 12 "ON

- BASE PLATES ON WOOD SHALL BE FASTENED WITH 16d COMMON NAILS AT 8" ON CENTER.

- 1. PLYWOOD SHEATHING.
  1. EXTERIOR WALL SHEATHING COVERED BY AN ARCHITECTURAL PINISH SHALL BE MINIMUM 7/16 INCH THICK (NOMINAL) 4 PLY PLYWOOD MANUFACTURED WITH EXTERIOR GLUE.
- 2. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE WALL STUDS.
- 3. FASTEN TO STUDS AND BLOCKING WITH 8d RING SHANK NAILS AT 4 INCHES ON CENTER ALL LOCATIONS.
- 4. IN ADDITION TO THE REQUIAR PASTENING, A SECOND ROW SHALL BE INSTALLED AT THE DOUBLE TOP PLATE AND TO THE LOWEST HORIZONTAL WOOD MEMBER ON AN EXTERIOR WALL. (LE. SILL PLATE, BAND JOIST)
- 5. FOR PLYWOOD SHEATHING COVERED WITH A CEMENTITIOUS FINISH ALL BUTT JOINTS NOT ON WALL STUDS SHALL BE BLOCKED WITH 2 X BLOCKING, TOE NAILED AT EACH END TO THE WALL STUDS WITH 3-84 COMMON NAILS. IL PARTICLE BOARD
- PARTICLE BOARD IS NOT TO BE USED WITHOUT THE EXPRESS, WRITTEN CONSENT OF THE STRUCTURAL ENGINEER AND THE PROPERTY OWNER.

### III. ARCHITECTURAL FINISHES

1. ARCHITECTURAL WALL FINISHES, SUCH AS STUCCO, CEMENTITIOUS COATING, SIDING OR PAINT ARE MENTIONED HERE ONLY FOR THE PURPOSE OF UNDERSTANDING THAT THEIR INSTALLATION AND ASSOCIATED DETAILS ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

### 23. COLUMNS A. CONCRETE / MASONRY COLUMNS

- 1. MASONRY COLDANS SHALL BE CONSTRUCTED OF PILASTER CONCRETE BLOCK OR FORMED AND POURED, WALL BLOCK SHALL NOT BE USED FOR MASONRY COLUMNS.
- II. REINFORCING STEEL SHALL BE GRADE 60 AND HELD IN PLACE BY STIRUPS SPACED AT 12 INCHES ON CENTER VERTICALLY. III. PILASTER BLOCK COLUMNS SHALL BE FILLED WITH A FINE GROUT
- HAVING A MINIMUM OF COMPRESSIVE STRENGTH OF 3,000 PSI
- LY, FORMED AND POURED COLUMNS SHALL CONSIST OF A MINIMUM OF 3,000 PSI CONCRETE, OR IN AREAS OF HIGH CHLORIDES, SUCH AS NEAR THE COAST OR BODIES OF SALT WATER, THE MINIMUM SHALL BE 5,000 PSI
- V. ALL MASONRY COLUMNS SHALL BEGIN AT THE FOUNDATION OR AT A MONOLITHIC FOOTING, IN NO CASE SHALL THERE BE A BREAK OR A COLD JOINT IN THE GROUT OF A COLUMN EXCEPT AT 1 FOOT FROM THE TOP IN PREPARATION FOR INSTALLATION OF A CONCRETE LINTEL.
- VI. METAL CONNECTORS AT THE TOP OF THE COLUMN FOR HOLDING WOOD BEAMS OR GIRDERS SHALL BE INSTALLED WITH THE MINIMUM EMBEDMENT OF THE ASSOCIATED FASTENERS FOR THE CONNECTOR AS SHOWN ON THE PLANS.
- WOOD COLUMNS:
- I. ALL LOAD BEARING WOOD COLUMNS SHALL BE A MINIMUM OF #2 GRADE PRESSURE TREATED WOOD. II. DIMENSIONAL WOOD COLUMNS OF 4 INCHES BY 4 INCHES IN CROSS
- SECTION SHALL ONLY BE USED FOR SUPPORTING OPEN WOOD DECKS WHERE THE FLOOR HEIGHT ABOVE THE FLOOR BELOW IS 8 FEET OR LESS ALL OTHER DIMENSIONAL WOOD COLUMNS SHALL HAVE A MINIMUM OF
- III. METAL CONNECTORS AT THE BASE AND THE TOP OF WOOD COLUMNS SHALL BE OF THE TYPE THAT RESISTS LATERAL LOADS AS WELL AS UPLIFT AND GRAVITY LOADS. IN NO CASE SHALL FLAT STRAPS BE USED UNLESS SPECIFICALLY SHOWN IN THE PLANS OR CROSS SECTION DETAILS.

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C. COMPOSITE COLUMNS

- 1. A COMPOSITE COLUMN HERE IS DEFINED AS A HOLLOW COLUMN CONSISTING OF ANY MATERIAL SPECIFICALLY DESIGNED BY ITS MANUFACTURER TO BE LOAD BEARING. ANY OTHER TYPE OF HOLLOW COLUMN IS CONSIDERED AN ARCHITECTURAL FINISH INTENDED TO FIT OVER A STRUCTURAL COLUMN AND ITS USE AND DETAILS OF INSTALLATION ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- LOAD BEARING COMPOSITE COLUMNS ARE A MANUFACTURED PRODUCT SUBJECT TO THE DESIGN AND LOAD BEARING CAPACITY AS DETERMINED BY THE MANUFACTURER. A SHOP DRAWING OR A LETTER FOR THE INSTALL ATION OF THE COLUMN SHALL BE PROVIDED BY THE STRUCTURAL ENGINEER TO SUPPLEMENT THE CONSTRUCTION PLANS AFTER THE SPECIFIC COLUMN AND MANUFACTURER HAVE BEEN IDENTIFIED.

  HEIN ALL CASES, THE COLUMN MANUFACTURES INFORMATION SHALL BE
- PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT FOR REVIEW PRIOR TO US ACCEPTANCE FOR THE STRUCTURAL DESIGN. THE INFORMATION SHALL INCLUDE THE LATERAL AS WELL AS UPLIET AND GRAVITY LOAD BEARING CAPACITIES.
- D., STEEL TUBE COLUMNS: LOAD BEARING STEEL TUBE COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS OF 14 INCH AND BE MADE OF STEEL WITH A DESIGN YIELD STRENGTH OF 46 PSI UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN
- II. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE STEEL TUBE COLUMN IS TO BE INSTALLED.

- I. LOAD BEARING ALUMINUM COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS
- II. ALL FASTENERS AND CONNECTORS FOR ALUMINUM COLUMNS SHALL BE STAINLESS STEEL OR MONEL TO AVOID CORROSION DUE TO DISSIMILAR METALS BEING IN CONTACT.
  III. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL
- DESIGN WHERE THE ALUMINUM COLUMN IS TO BE INSTALLED.

- A. MANUFACTURED WOOD TRUSSES

  1. THE MANUFACTURED ROOF TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL SYSTEM DESIGN. IT IS NOT INTENDED TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL
- II. MANUFACTURED ROOF TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE, THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.
- III. THE TRUSS PLAN'S SIGNED AND SEALED'BY THE DELEGATED ENGINEER SHALL HIL THE TRUSS PLANT SHORED AND SEALED BY THE DELCAR THE MANIFES SHALL.

  BE PROVIDED TO AND PRIOR TO CONSTRUCTION OF THE UNDERLYING STRUCTURE
  AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL
  CHANGES BASED ON THE FINAL FLOOR TRUSS SYSTEM.

  VI. THE TRUSS MANUFACTURER SHALL PROVIDE ALL LATERAL BRACING
  REQUIREMENTS TO THE BUILDING CONTRACTOR, IF NOT, THE BUILDING
- CONTRACTOR IS TO NOTIFY THE STRUCTURAL ENGINEER FOR GUIDANCE.
- Y. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH TRUSS IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 100 COMMON NAILS (TOE-NAILED)
- VI. A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE / MASONRY
- 23,2 CONVENTIONAL FRAME
- 1. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH RAFTER IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 3-104 COMMON NAILS (TOE-NAILED)
- II. ANY WOOD COMING IN CONTACT WITH MASONRY OR CONCRETE IS TO BE PRESSURE TREATED OR A MOISTURE BARRIER IS TO HE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE OR MASONRY.

- III. COLLAR TIES ARE TO BE INSTALLED BETWEEN RAFTERS AT 2/3 OF THE RIDGE HEIGHT FROM WHERE THE RAFTERS BEAR ON WALLS. THE COLLAR TIES ARE TO BE PASTENED WITH A MINIMUM OF 4-106 16 COMMON NAILS (CLINCHED) AT EACH LAP JOINT. EACH RAPTER IS TO BE ATTACHED TO THE RIDGE BEAM WITH A LIGHT ANOLE HANGER AS SHOWN IN THE FRAMING PLAN. IN ADDITION, A FLAT METAL STRAP SHALL BE INSTALLED ACROSS THE RIDGE BEAM TO TWO OPPOSING RAFTER. TO BE REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN
- AND FOR ANY CHANGES TO THE "TRUSS TO THE UNDERLYING STRUCTURE" CONNECTIONS.

  IV. AS PART OF THE REVIEW, THE STRUCTURAL ENGINEER WILL DETERMINE WHETHER THE TRUSS TO WALL/BEAM METAL. CONNECTORS SHOWN IN THE ORIGINAL PLANS ARE ACCEPTABLE OR WHETHER THEY NEED TO BE CHANGED OR SUPPLEMENTED TO ACCOMMODATE THE LOADS SHOWN IN THE TRUSS COMPONENT
- V. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR VERIFYING THE DIMENSIONAL, ARCHITECHURAL, OR FORM ASPECTS OF THE OF THE TRUSS MANUFACTURERS PLAN WITH THE ORGINAL PLANS. VI. THE MINIMUM LIVE LOADS FOR THE ROOF TRUSS DESIGN IS TO BE
- ON FBC 2017 SECTION 1607 FOR ROOF TYPE AND ROOFING MATERIAL VII. THE DEAD LOADS ARE LASTED IN ITEM 16 ABOVE.
- VII. ALL TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTORS ARE TO BE SPECIFIED BY THE TRUSS MANUFACTURER, INCLUDING CONNECTORS FOR TRUSS TO MANUFACTURED BEAM (LE. GLUELAM, OR MICROLAM) SPECIFIED BY THE TRUSS MANUFACTURER. A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION, A HANGER IS REQUIRED IN THE TRUSS SYSTEM.
- IX. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED IX. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BIL PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORGINALPLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER.

  X. A RINGE BEAM TERMINATION AT A GABLE END SHALL BE SUPPORTED ON A MOUNTAIN DESIGN.
- BY A MINIMUM 3 STUD PACK COLUMN BEARING ON THE UNDERLYING
- NALL OR BEADS.

  XI. TREATED LUMBER-DOUBLE 1 1/2 INCH BY A HEIGHT SHOWN ON THE PLANS, FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8 INCH BY 5 1/2 INCH SIMPSON TITEN HD CONCRETE BOLTS.

  XII. SLEEPERS SHALL BE FASTENED TO UNDERLYING ROOF TRUSSES.
- OR RAPTERS (NOT SHEATHING ) WITH A MINIMUM OF 2-3/8 INCH BY 3 1/2 INCH LAG BOLTS AND WASHERS AT EACH TRUSS OR RAFTER INTERSECTION AND NO GREATER THAN 24 INCHES ON CENTER AND SHALL CONSIST OF DIMENSIONAL LUMBER 1 1/2 INCH THICK BY A WIDTH SHOWN IN THE PLANS.
- XIII. USE 2 INCH BY 4 INCH BLOCKING ATTACHED BETWEEN UNDERLYING STUDS, TRUSSES OR RAFTERS WITH A MINIMUM OF 3-104 NAILS AT EACH IN ORDER TO SATISFY THE ON CENTER SPACING FOR THE LEDGERS OR SLEEPERS.
- XIV BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS ARE TO BE ATTACHED AS SPECIFIED IN THE ROOF FRAMING PLANS.
- 24. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.
- A. LEDGERS SEEPERS

  1 LEDGERS MALERS SHALL BE FASTENED TO WOOD STUDS (NOT SHEATHING)
  WITH A MINIMUM OF 2- 3% INCH BY 5-1/2 INCH LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AND NO GREATER THAN 16 INCHES ON CENTER AND SHALL CONSIST ON PRESSURE TREATED WOOD.
- II. MILTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (I.E. GLUELAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS SPECIFICATIONS.

- III. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED
- 1. FOR TWO PLY BEAMS ONE ROW OF 108 GALVANIZED COMMON NAILS AT 6 INCHES ON CENTER ON EACH SIDE OF BEAM.
- NAILS AT BIRCHES ON CENTER OF EACH SIDE OF BEAST.

  D. FOR THREE PLY BEAMS: TWO ROWS OF 164 GALVANIZED COMMON NAILS AT 6"ON CENTER (TOP AND BOTTOM )

  THRU EACH SIDE OF THE BEAM.

  BILFOR FOUR PLY BEAMS AND LARGER: TWO ROWS OF 1/2 INCH DIAMETER CARRIAGE BOLTS OR ALL THREAD RODS WITH NUTS AND WASHERS SPACED AT 12" ON CENTER 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM.
- D. SHEATHING COVERED BY COMPOSITE ROOFING SHALL BE A MINIMUM OF 15/32 INCH THICK (KOMINAL.) O.S.B., MANUFACTURED WITH EXTERIOR GLUE.

  11. ROOF SHEATHING COVERED BY THE SHALL BE A MINIMUM OF 5/8 INCH THICK (NOMINAL.) MANUFACTURED WITH EXTERIOR CLUE.

- GIUE.

  BI. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE ROOF TRUSS SYSTEM.

  JV. FASTENING SHALL BE 80 RING SHANK NAILS AT 4 INCHES ON CENTER AT BOUNDARY AND EDGES AND 6 INCHES ON CENTER IN THE FIELD WITH A SETBACK OF 5 °0" FROM ALL EDGES.

  V. METAL "H" CLIPS OR SOLID WOOD BLOCKING SHALL BE USED AT ALL INSUPPORTED BUTT JOINTS BETWEEN TRUSSES OR RAFTERS.
- 25. PRECAST CONCRETE LINTELS
- A. PRECAST AND PRISTRESSED CONCRETE LINTELS SHALL BE MANUFACTURED BY CASTCRETE AND INSTALLED PER MANUFACTURES
- SPECIFICATIONS AND INSTRUCTIONS.

  B. THE SIZE OF THE LINTELS SHALL BE BASED ON THE SPAN AND LOAD.
  REFER TO THE ATTACHED SCHEDULE UNLESS OTHERWISE SHOWN IN
  THE STRUCTURAL DESIGN FOR THE SPECIFIED LINTEL.
  C. LINTEL SCHEDULE UNIO, ON PLANS:
- L SPAN UP TO 3'- 8F8-0H
- II. SPAN UP TO 3' TO < 6' 8F8-OB III. SPAN 6' TO > 14' - 8F16- 1B/1T
- D. THE MINIMUM SPECIFIED GROUT COMPRESSIVE STRENGTH TO BE USED
- FOR LINTELS IS 3,000 PSI.

  B. THE REINFORCING STEEL SHALL BE ASTM GRADE 60
- 26. FASTENERS / METAL CONNECTORS.
- A. ALL FASTENERS AND METAL CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE AND INSTALLED PER THE MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS,
- B. THESE FASTENERS DO NOT INCLUDE TYPICAL NABS AND SCREWS WHICH MAY BE MANUFACTURED BY OTHERS.
- C. FOLLOW ALL MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS FOR ALL FASTINERS, METAL CONNECTIONS, SCREWS, NAILS, KTC. THAT ARE IN CONFACT WITH PRESSURE TREATED LUMBER.
- 27. DIMENSIONAL LUMBER:
- A. ALL LOAD BEARING WALLS SHALL BE SOUTHERN YELLOW PINE #2 OR BETTER GRADED AND STAMPED BY THE CERTIFYING AGENCY. IN ADDITION, ALL WOOD SHALL BE PRESSURE TREATED FOR EXTERIOR USE WHERE EXPOSED TO MOISTURE, PLACED WITHIN 12 INCHES OF SOIL OR IN CONTACT WITH CONCRETE OR MASONRY.

  28. STRUCTURAL SHEATHING:
- 28. STRUCTURAL SHEATHING:
  A. ALL SHEATHING USED FOR EXTERIOR APPLICATIONS SHALL BE EXTERIOR GRADE AND ADA STAMPED AND VERIFYING ITS RATING.
- A. CONCRETE MASONRY UNITS SHALL CONFORM WITH AMERICAN MASONRY INSTITUTE STANDARD 530
- B. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI
- C. MORTAR SHALL BE OF TYPE M OR S GRAY MORTAR.
- A. ALL GROUT SHALL BE A FINE TYPE HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI UNLESS SPECIFICALLY SHOWN OTHERWISE BY A MANUFACTURER PURSUANT TO GROUT USE WITH ITS PRODUCTS.
- ALL REINFORCING STEEL: ALL BE ASTM GRADE 40 EXCEPT GRADE 60 SHALL BE USED FOR GRADE BEAMS, ALL LINTEL TYPES (I.E. PRECAST AND FIELD PRIFORMED) COLUMNS UNLESS OTHERWISE SHOWN IN THE STRUCTURAL PLANS.

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ALLEN ENGINEERING & CONSTRUCTION SERVING ALCH ALLEN PROFESSIONAL ENGPER 56920 C.A. # 9502

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32. STRUCTURAL STEEL AND CONNECTION ACCESSORY MATERIAL:
A. LEBIAMS, FORMED STRUCTURAL STEEL, FLAT BAR OR PLATE
SHALL BE ASTM GRADE A36 UNLESS STATED OTHERWISE.

SHALL BE ASTM GRADE A36 UNLESS STATED OTHERWISE.

B. ALL STRUCTURAL STEEL SHALL HAVE A MINIMUM OF TWO COATS OF PRIMER AND TWO COATS OF EPOXY AS A CORROSION PREVENTIVE. THE BUILDING CONTRACTOR MAY VARY FROM THIS SPECIFICATION WITH THE APPROVAL OF THE STRUCTURAL ENGINEER IF IT CAN BE DEMONSTRATED ANOTHER MEANS OF CORROSION CONTROL IS EQUALLY EFFECTIVE.

C. ALL WILLDING OF STRUCTURAL STEEL SHALL BE MADE WITH E60/70 TYPE ELECTRODES, THE DEPTH AND LENGTH FOR THE WELD SHALL BE SPECIFIC DIN THE STRUCTURAL DESIGN FOR THE

SPECIFIC CONNECTION.

VENTILATION:
 A. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR DETERMINING VENTILATION REQUIREMENTS OF CRAWL SPACES, FLOORS AND ATTICS NOR THE MEANS AND METHODS FOR IMPLEMENTING THESE REQUIREMENTS.

- ATTICS NOR THE MEANS AND METHODS FOR IMPLEMENTING THESE REQUIREMENTS.

  34. WATERPROOFING:
  A. ANY RENDERING OF NOTES OF WATERPROOFING MEASURES FOR BASEMENTS OR HALF BASEMENTS SHOWN IN THESE PLANS WHERE A SPECIFIC CONSTRUCTION DETAIL IS NOT SHOWN IN THE STRUCTURAL DESIGN IS AN ARCHITECTURAL ILLUSTRATION ONLY AND IS NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

  B. CRICKETS ARE ASSOCIATED WITH THE ADOLUTION IN TRACE IN TRACE.
- B. CRICKETS ARE ASSOCIATED WITH THE ARCHITECTURAL FINISHES AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

35. FIRE RESISTANT DESIGN:

35. FIRE RESISTANT DESIGN:
A. FIRE RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON UNDERWRITERS LABORATORY OR GYPSUM ASSOCIATION DESIGN

- ON UNDERWRITERS LABORATORY OR GYPSUM ASSOCIATION DESIGN FOR FIRE RATED FLOOR, WALL AND ROOF ASSEMBLIES.

  36. FLOOD RESISTANT DESIGN:
  A. FLOOD RESISTANT DESIGN OF FLOOD RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCHIENTAL TO THEIR STRUCTURAL DEIGN AND SHALL BE BASED ON THE REQUREMENTS STATED IN TITLE 44 CFR SECTIONS 59 AND 60, AND ON THOSE OF THE INDIVIDUAL COMMUNITY RATING AGENCIES FOR THE GOVERNMENTAL JURISDICTION WHERE THE CONSTRUCTION IS TO BE DONE.

  B. HOWEVER, THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR IDENTIFYING AND SHOWING ON THE PLANS THE FLOOD ZONE CATEGORY, BASE FLOOD ELEVATION, AND THE FLOOR AND STORY HEIGHTS OF THE BUILDING IN RELATION TO THE BASE FLOOD ELEVATION. THIS INFORMATION IS CONSIDERED ARCHITECTURAL AND SITE RELATED AND SHALL BE PROVIDED TO THE STRUCTURAL FNGINEER BY THE CONTRACTING CLIENT OR HIS AGENT. OR HIS AGENT.
  37. SPECIAL CONSTRUCTION:
  1. ALUMINUM STRUCTURAL COLUMNS.

- L. ALIMINUM STRUCTURAL COLUMNS.
  A. ANY ALUMINUM STRUCTURES SHOWN IN THESE PLANS SUCH AS PORCH AND POOL FUCLOSURES OR GUARDRAILS AND HANDRAILS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL.
- ENGINEER.

  B. WHERE THE ALUMINUM STRUCTURE ATTACHES TO THE MAIN STRUCTURE OR IS INCORPORATED IN THE MAIN STRUCTURE, SHOP DRAWINGS FOR THESE STRUCTURES SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER TO DETERMINE THEIR EFFECT ON THE MAIN STRUCTURE.

II. SWIMMING POOLS:

II. SWIMMING POOLS:

A. ANY SWIMMING POOL OR HOT TUBS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL BLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL DESIGN.

III. FENCES AND RETAINING WALLS: HIL PENCES AND RETAINING WALLS:
A. ANY RENDERING OF PENCES, RETAINING WALLS OR EXTERIOR PLANTERS WHERE A SPECIFIC STRUCTURAL DETAIL IS NOT SHOWN FOR THEIR CONSTRUCTION ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

IN INDIDDUCAGE ASSISTED PHASES.

IV. DRIVEWAYS AND WALKWAYS:

A. ANY DRIVEWAYS OR WALKWAYS SHOWN IN THESE PLANS ARE FOR

ARCHITECTURAL ILLUSTRATION PURPOSES ONLY AND ARE NOT PART OF THE

STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

	oof Live Loads					
Attics: 20 psf w/ storage, 10 psf w/o storage						
1)Hottablo 1 titles, is em v =	psf					
WII Ottion troumer:	psf					
Ontrige.	40 psf					
Roofs: 20	psf					
Wind D	esign Data					
Ultimate Wind Speed:		145 mph				
Nominal Wind Speed:		112 mph				
Risk Category:		11				
Wind Exposure:	•	В				
Enclosure Classification:		Enclosed				
internal Pressure Coefficient:		0.18 +/-				
Components and Cladding Design Pressur	res:					
Roofing Zone 1:	+16.0 psf max.,					
Roofing Zone 2:	+16.0 psf max.,					
Roofing Zone 3:		-53.2 psf min.				
Roofing at Zone 2 Overhangs:						
Roofing at 2	Zone 3 Overhangs:	-70.9 psf min.				
Stucco, Cladding, Doors & Windows:						
	+22.6 psf max.,					
Zone 5:	+22.6 psf max.,					
End Zone Width:		4.00 ft.				
The Nominal Wind Speed was used to	determine the above	Component and				
Cladding Design Pressures.						
All exterior glazed openings shall be pr Section 1609,1,2 of the 2017 FBC.						
The site of this building is not subject to Section 1609.1.1.1 of the 2017 FBC.	special topographic	wind offects as per				
	al Information					
Design Soll Load-Bearing Capacity:		2,000 psf				
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\* 0.6 ALLOWABLE STRESS DESIGN USED \*

# **MODEL 2848** RRIMROSE

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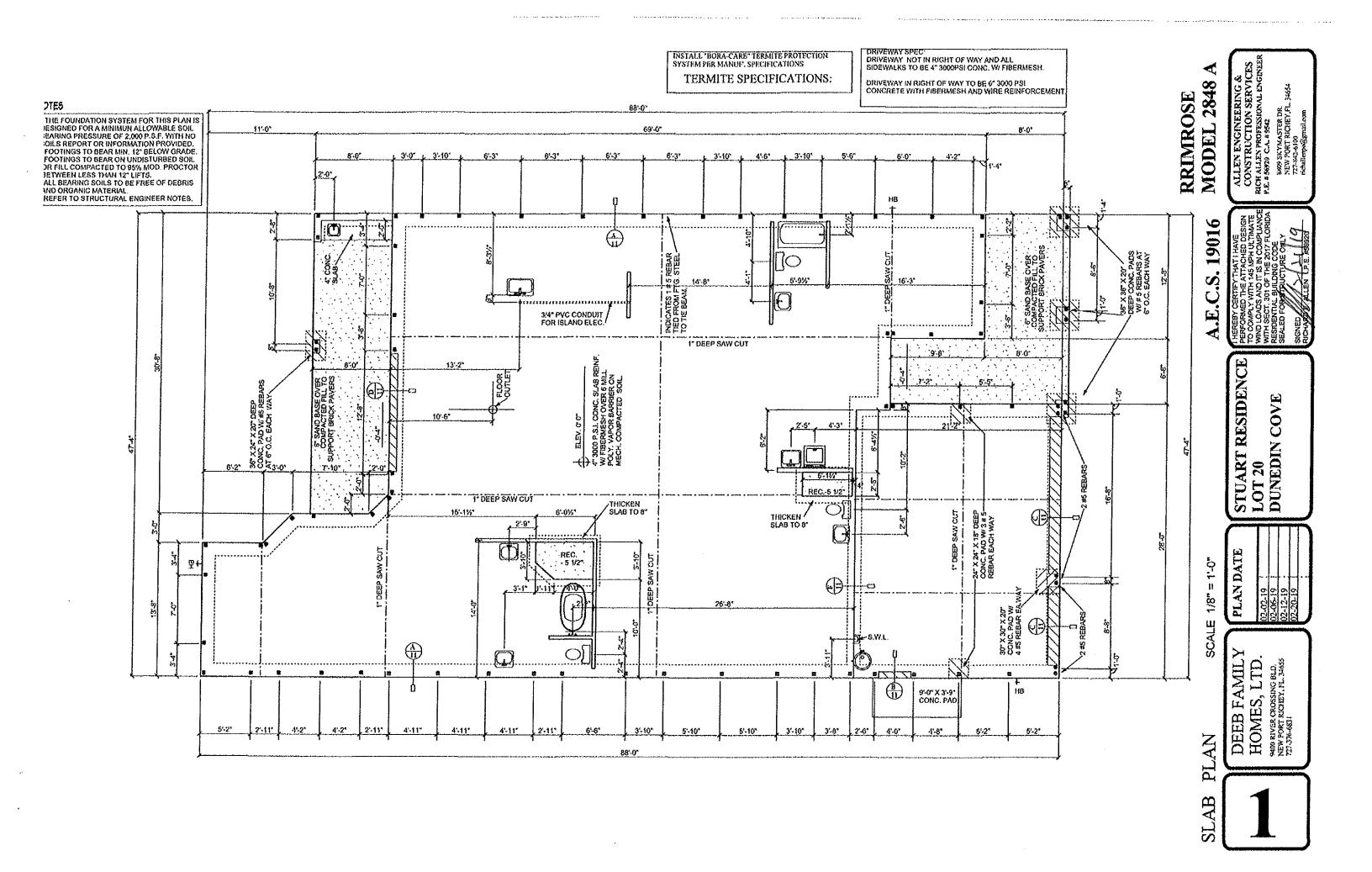
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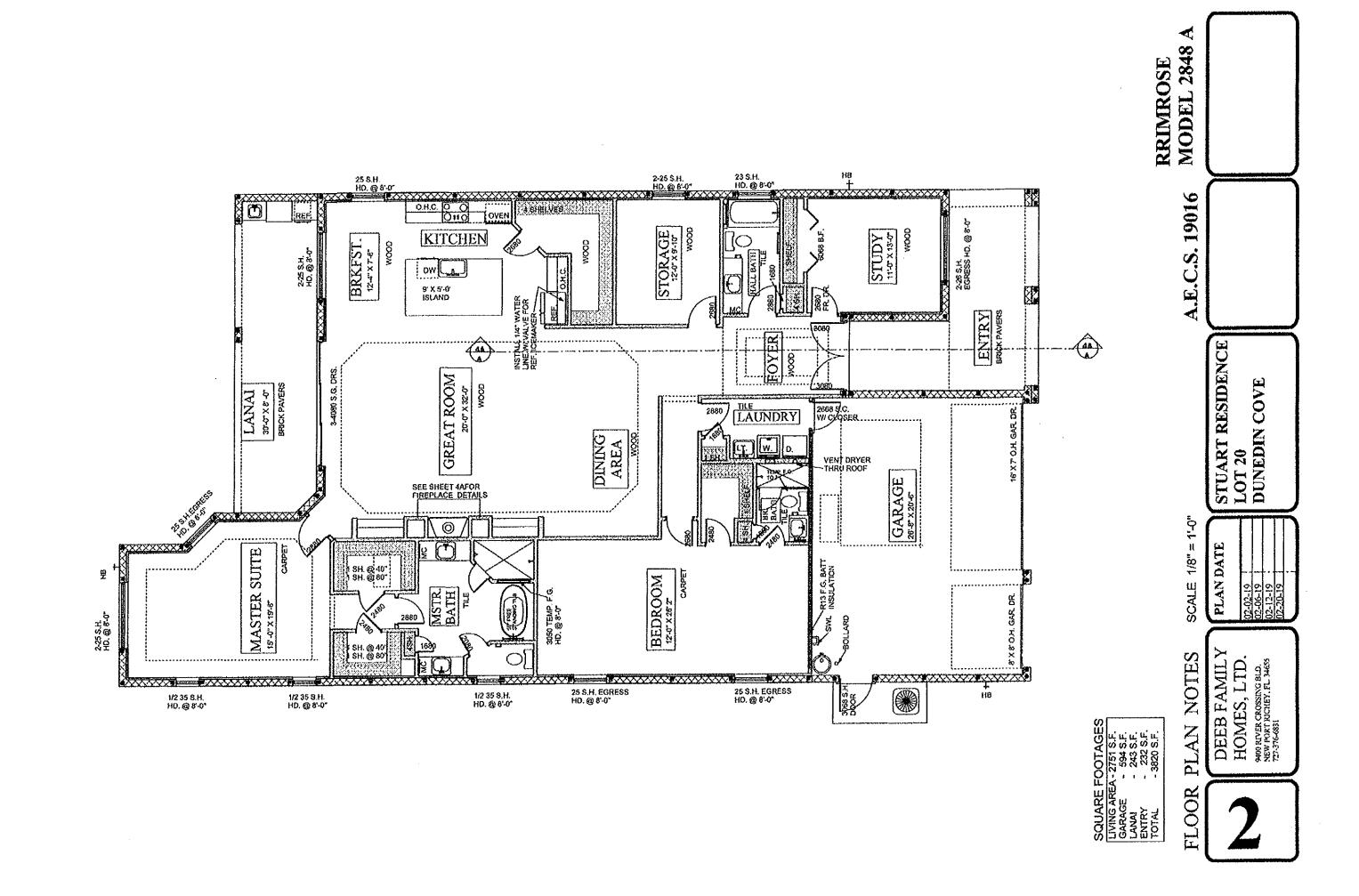
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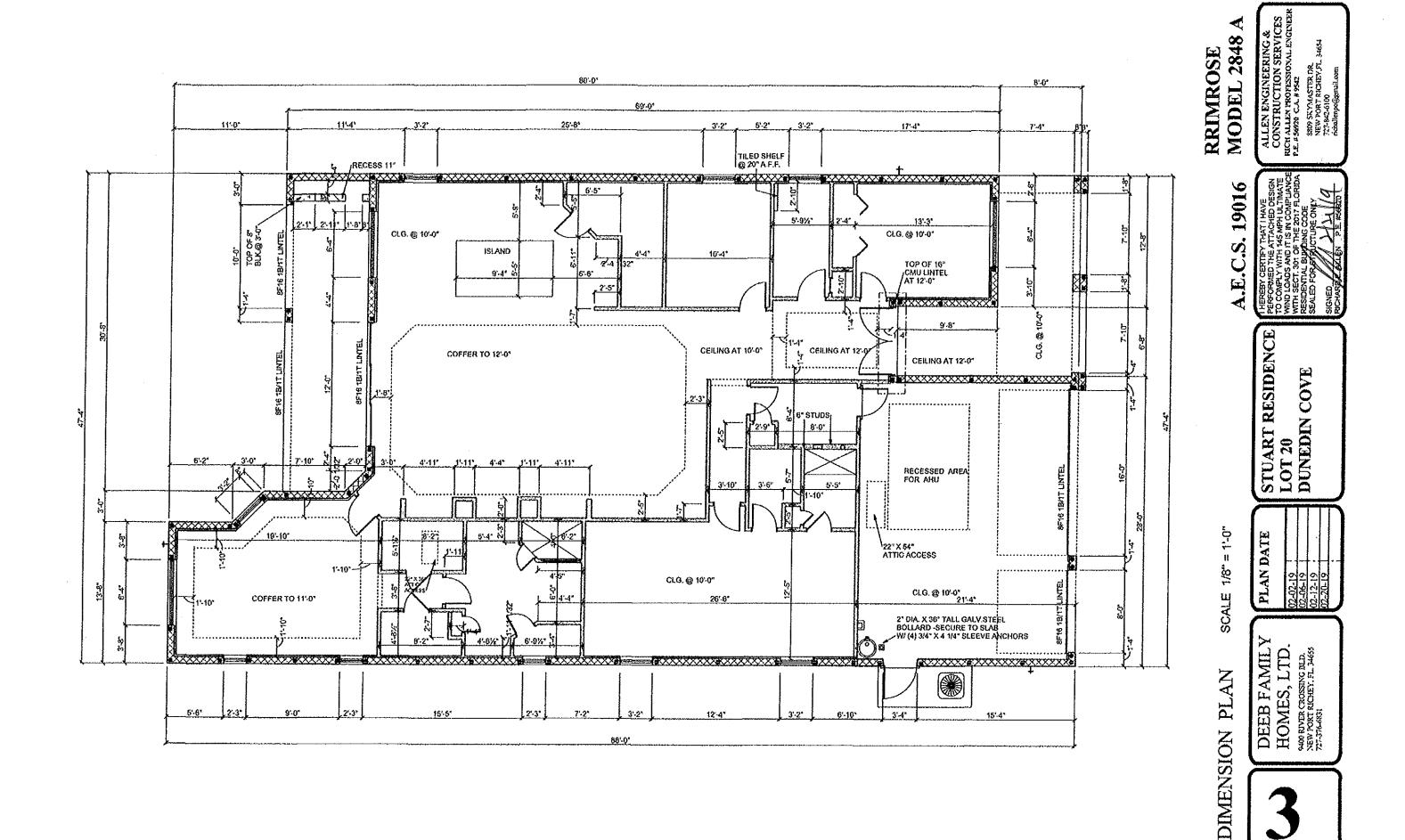
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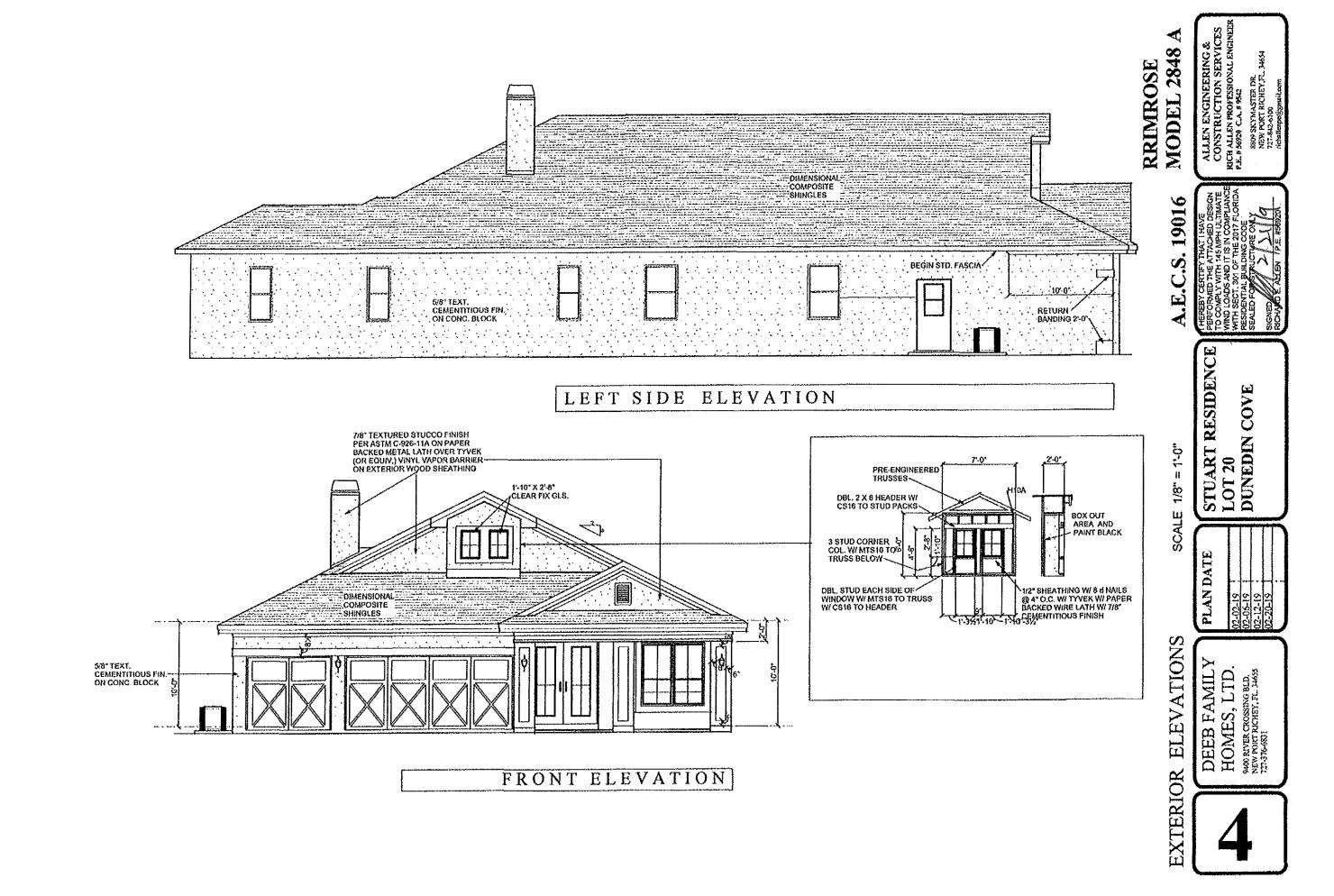
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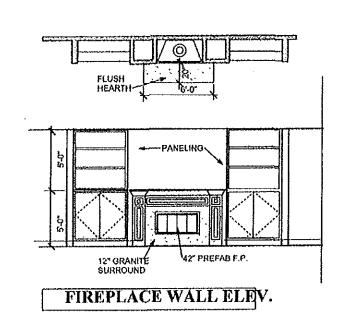
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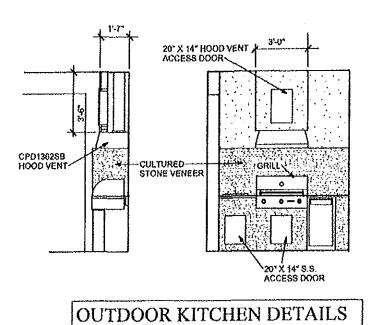


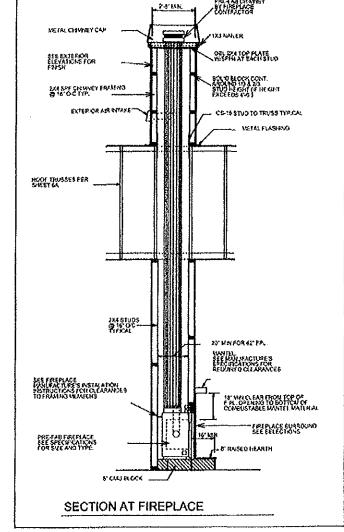


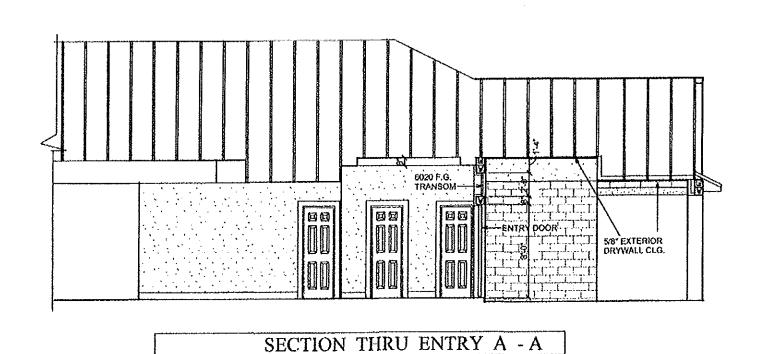


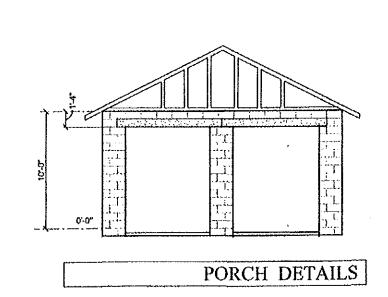


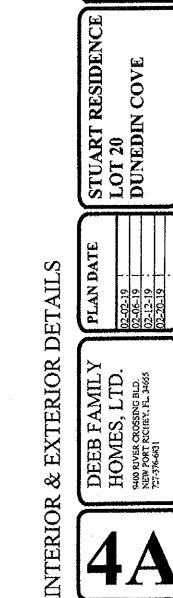






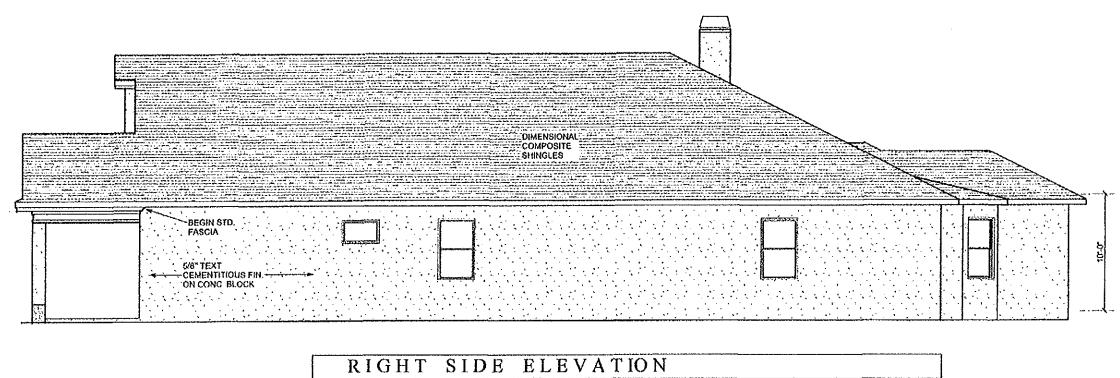






RRIMROSE MODEL 2848

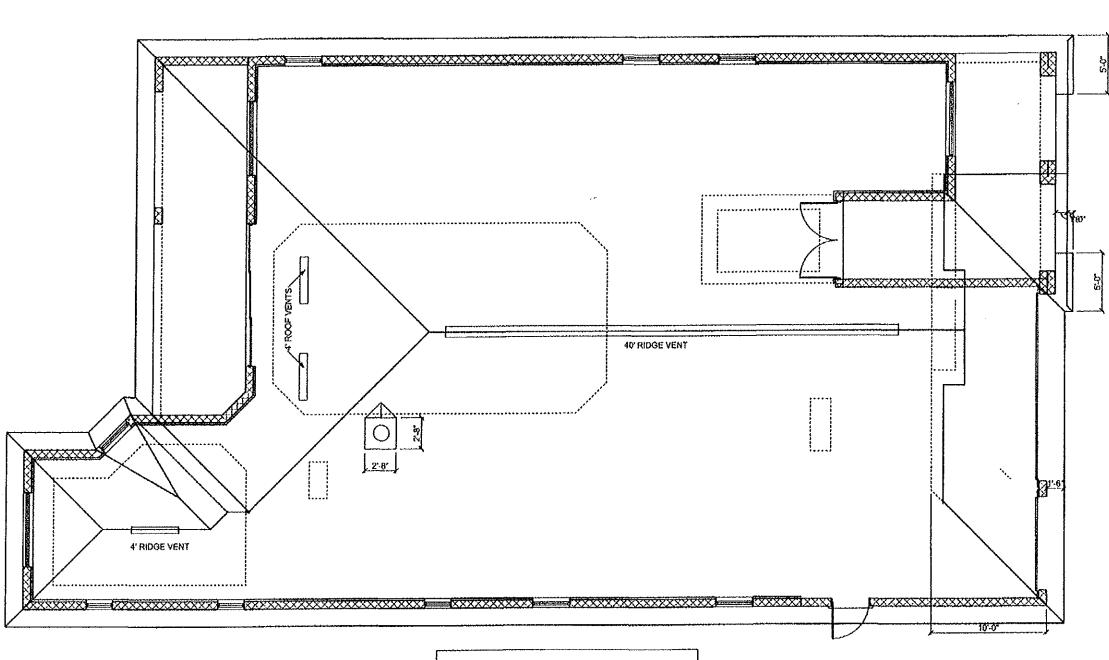
19016





**MODEL 2848** RRIMROSE A.E.C.S. 19016 STUART RESIDENCE LOT 20 DUNEDIN COVE SCALE 1/8" = 1'-0" PLAN DATE EXTERIOR ELEVATIONS DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW FORT RICHEY, FL. 34655 27:376-6831

REAR ELEVATION



TOTAL NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1 TO 300 PROVIDED THAT AT LEAST 50 % AND NOT MORE THAN 80 % IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED PER SECT. R808.2

TOTAL AREA TO BE VENTILATED = 3820 S.F. 3820/300 = 12.73 S.F. OR 1834 SQUARE INCHES.

ROOF VENTS ARE RATED AT 36 SQUARE INCHES OF OPENING PER LINEAU FT. 1834 S.1.38 S.1. =51 LINEAU FEET REQUIRED.

INSTALLATION FOR THIS ROOF IS 51 OF ROOF VENTING

1/8" = 1-0" SCALE **PLAN** ROOF

PLAN DATE

STUART RESIDENCE LOT 20 DUNEDIN COVE

RRIMROSE MODEL 2848

A.E.C.S. 19016

DEEB FAMILY HOMES, LTD.

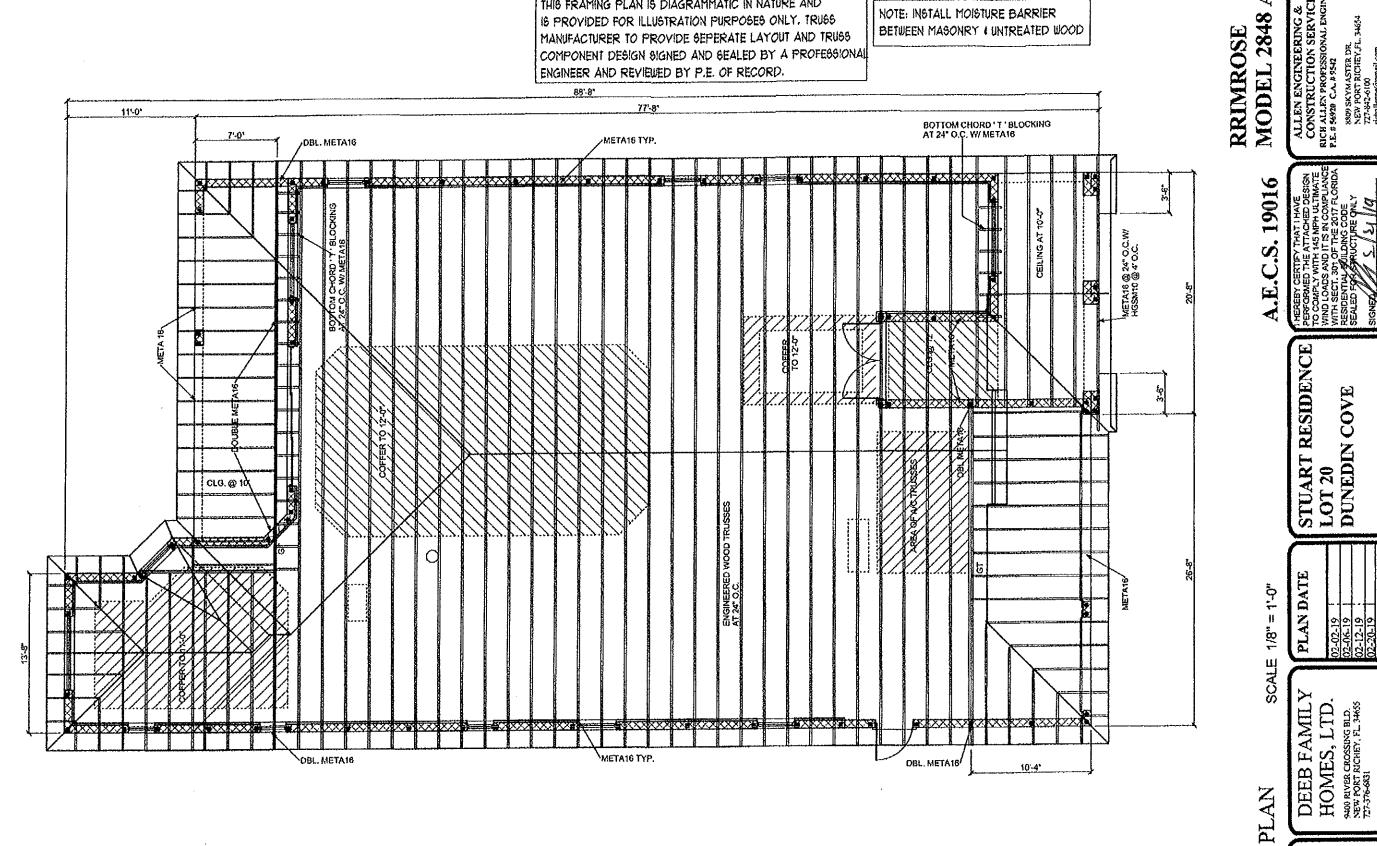
9400 RIVER CROSSING SED.
NEW FORT RICHEY, FL. 34655
T27-376-6851

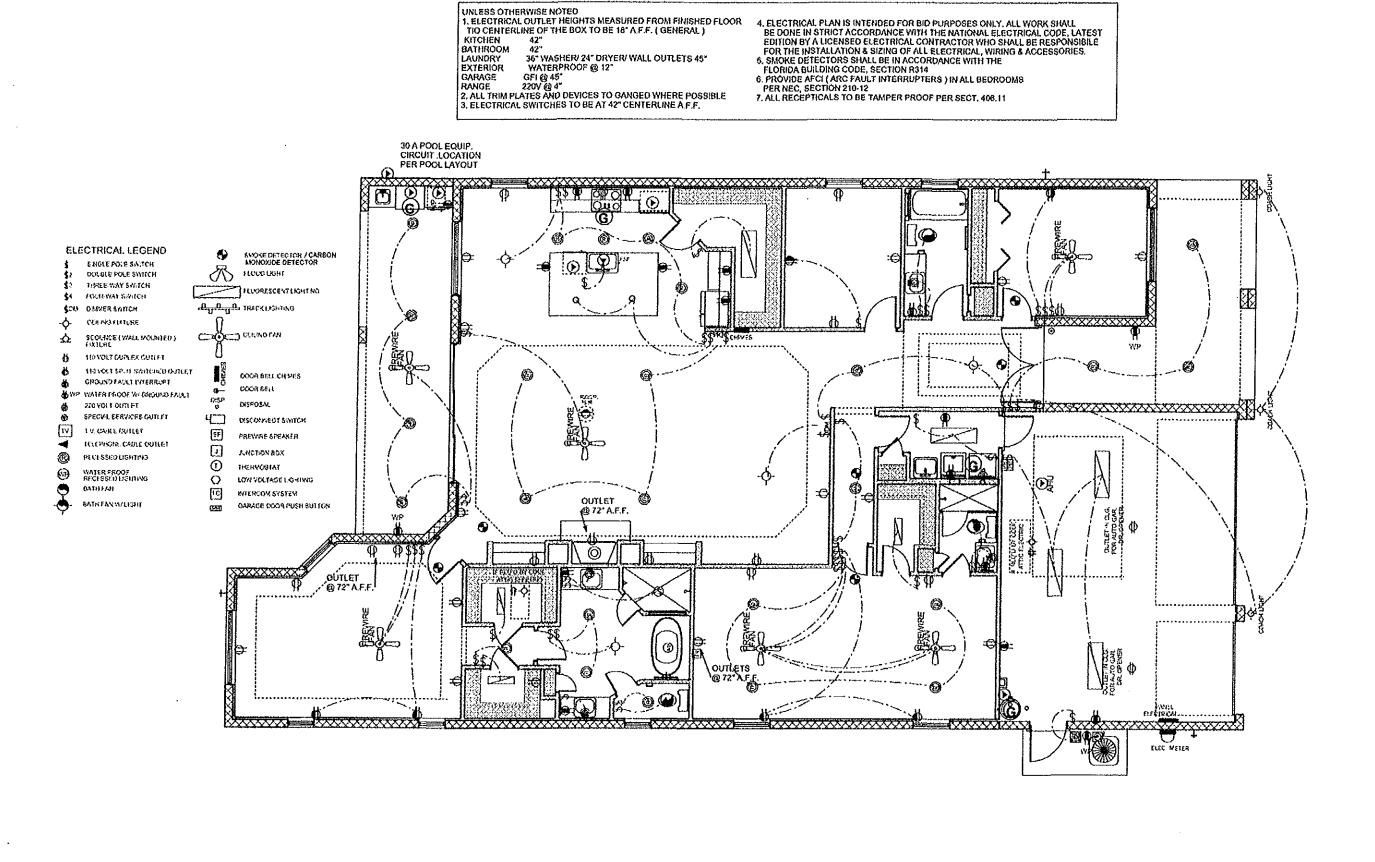
IMPORTANT NOTE:

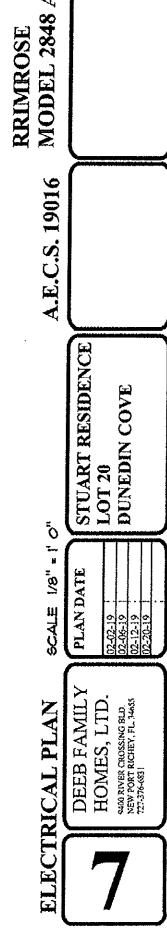
THIS FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY, TRUSS MANUFACTURER TO PROVIDE SEPERATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER AND REVIEWED BY P.E. OF RECORD.

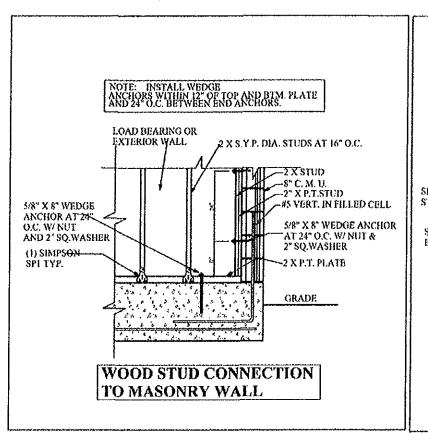
NOTE: INSTALL MOISTURE BARRIER BETWEEN MASONRY I UNTREATED WOOD

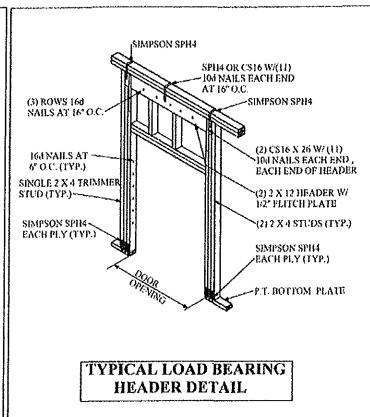
TRUSS

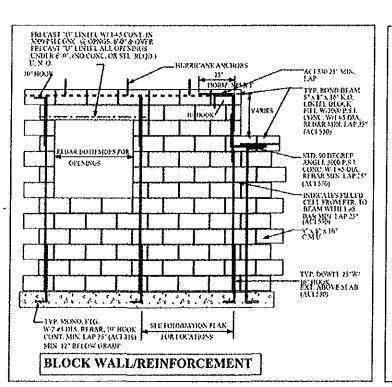


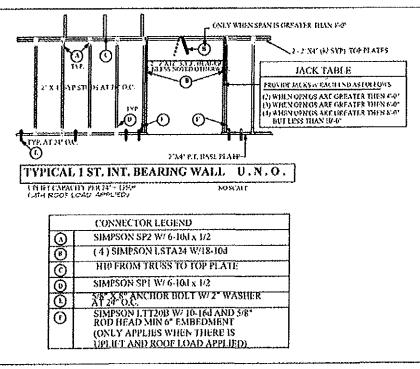


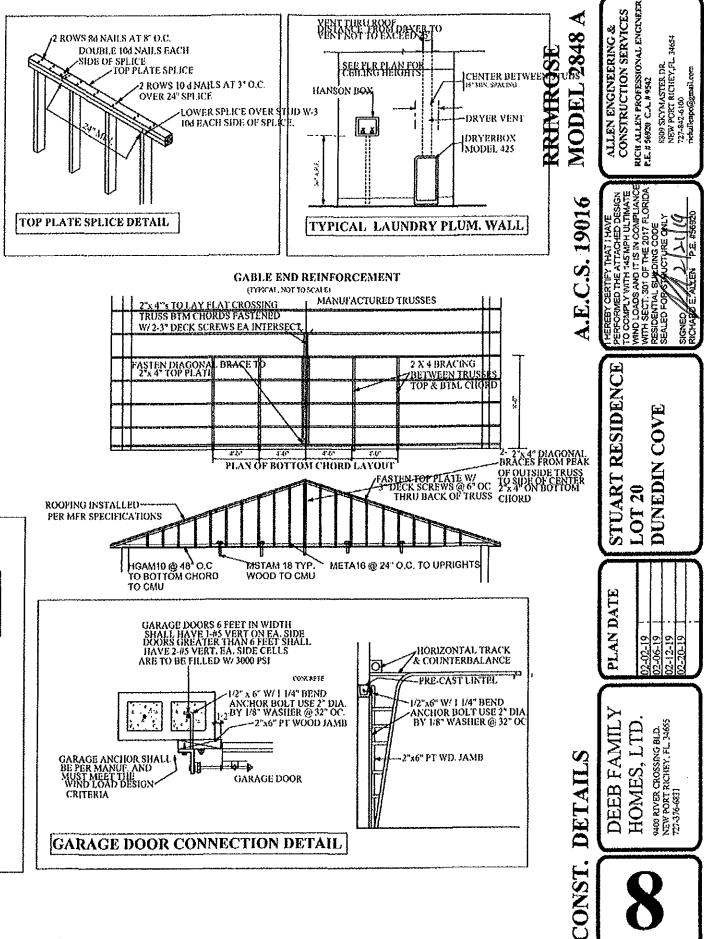


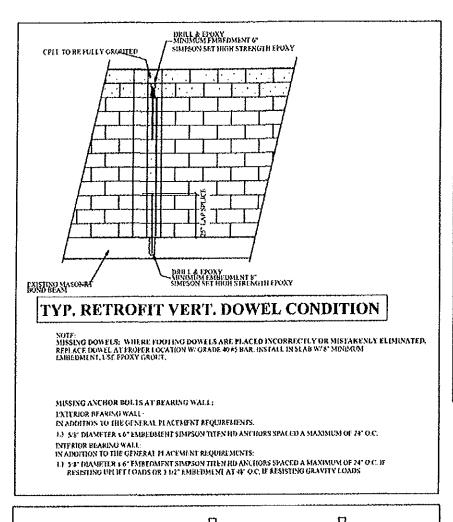


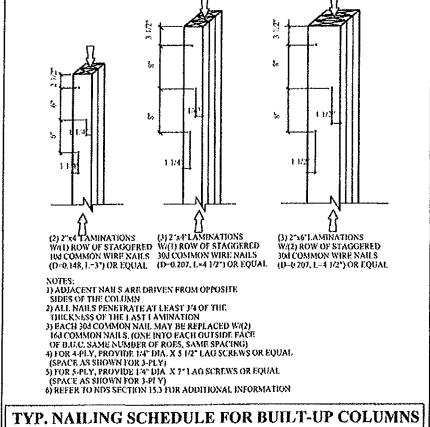


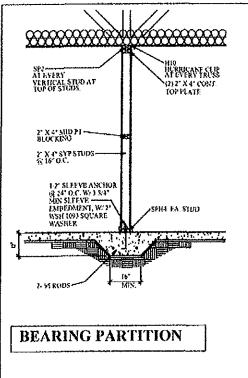


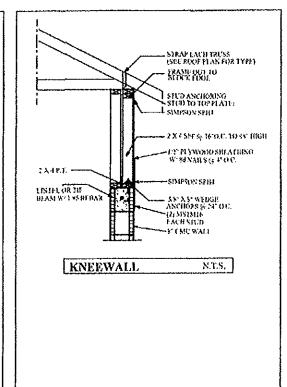














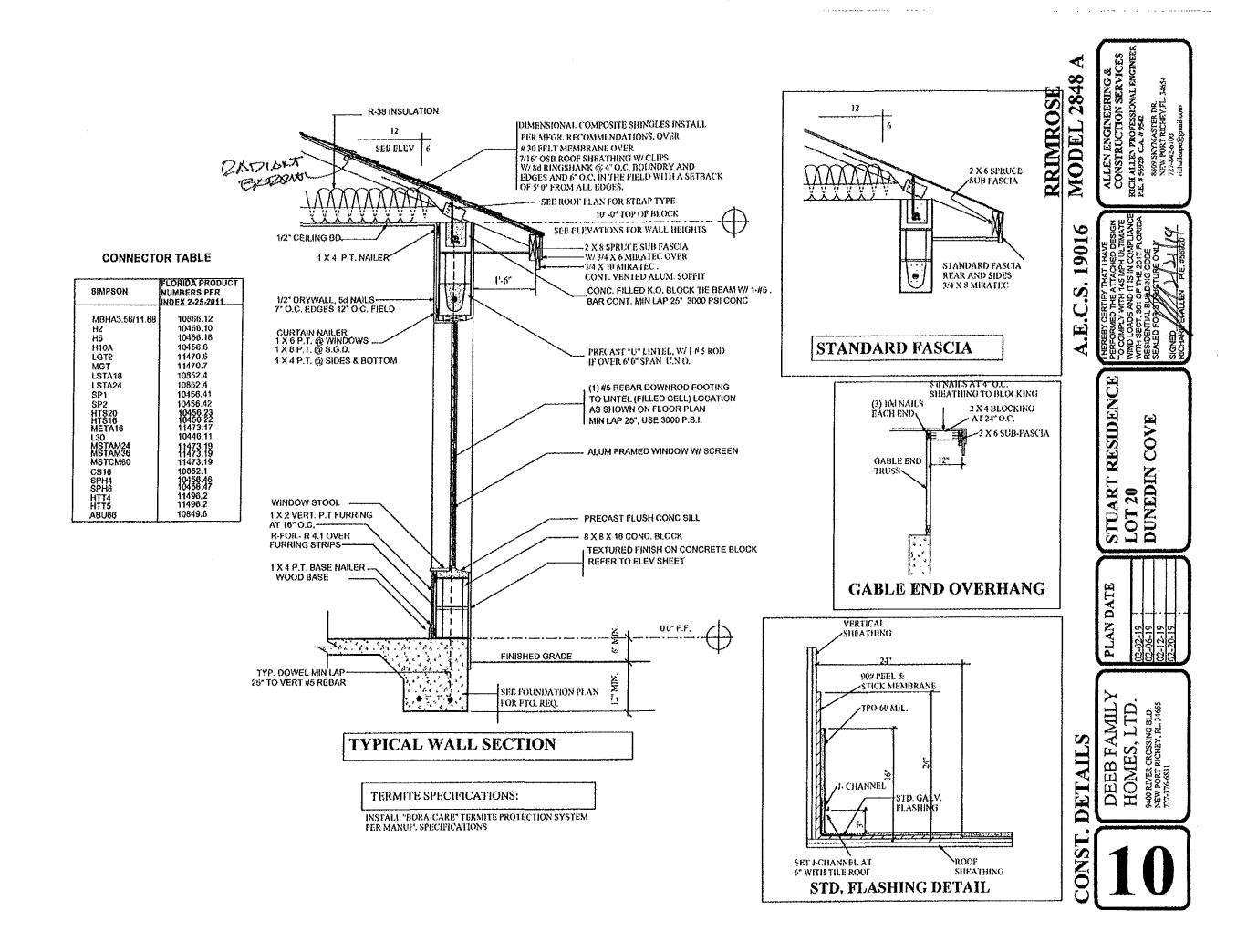
RRIMIROSE

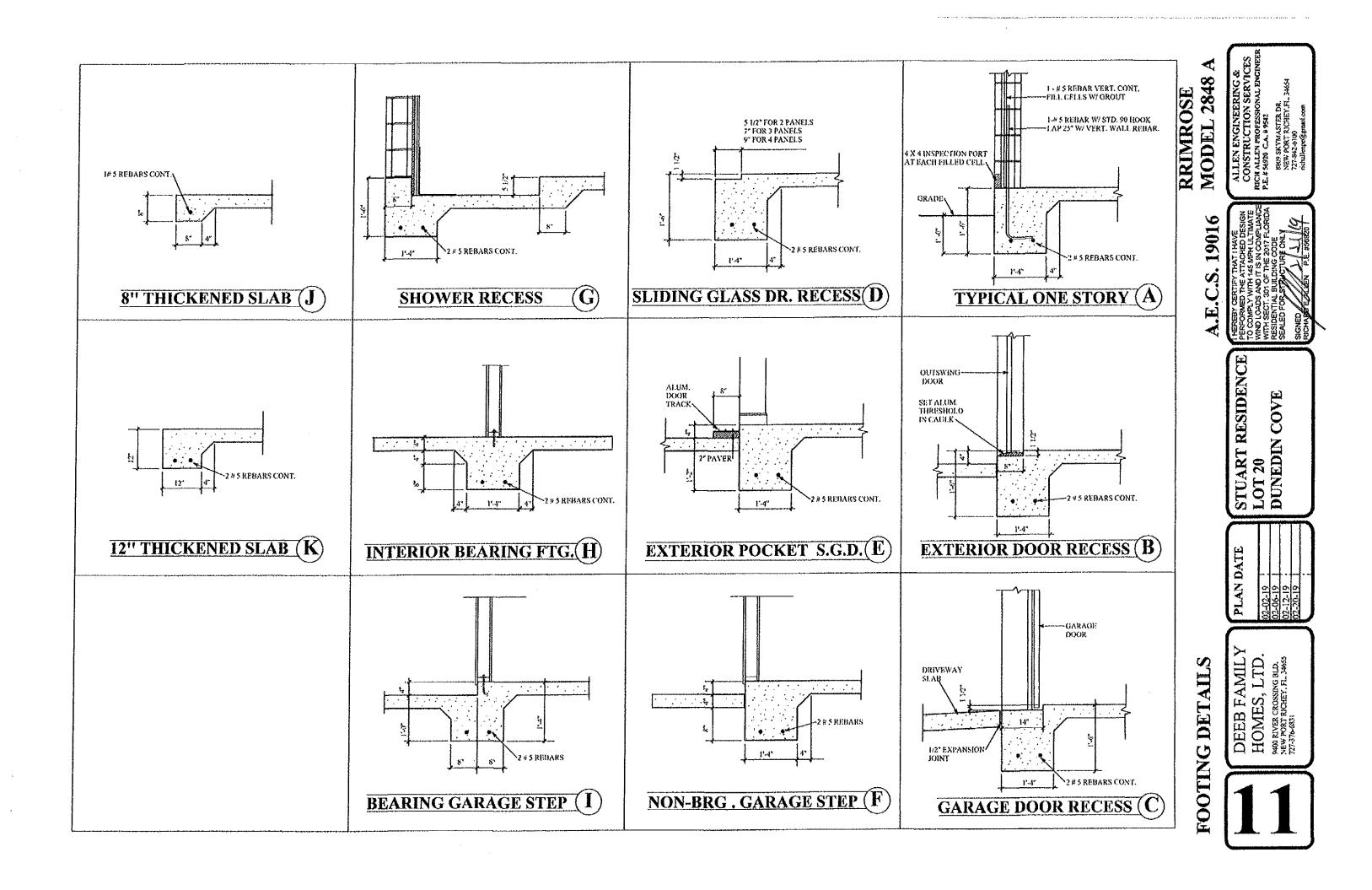
STUART RESIDENCE LOT 20 DUNEDIN COVE

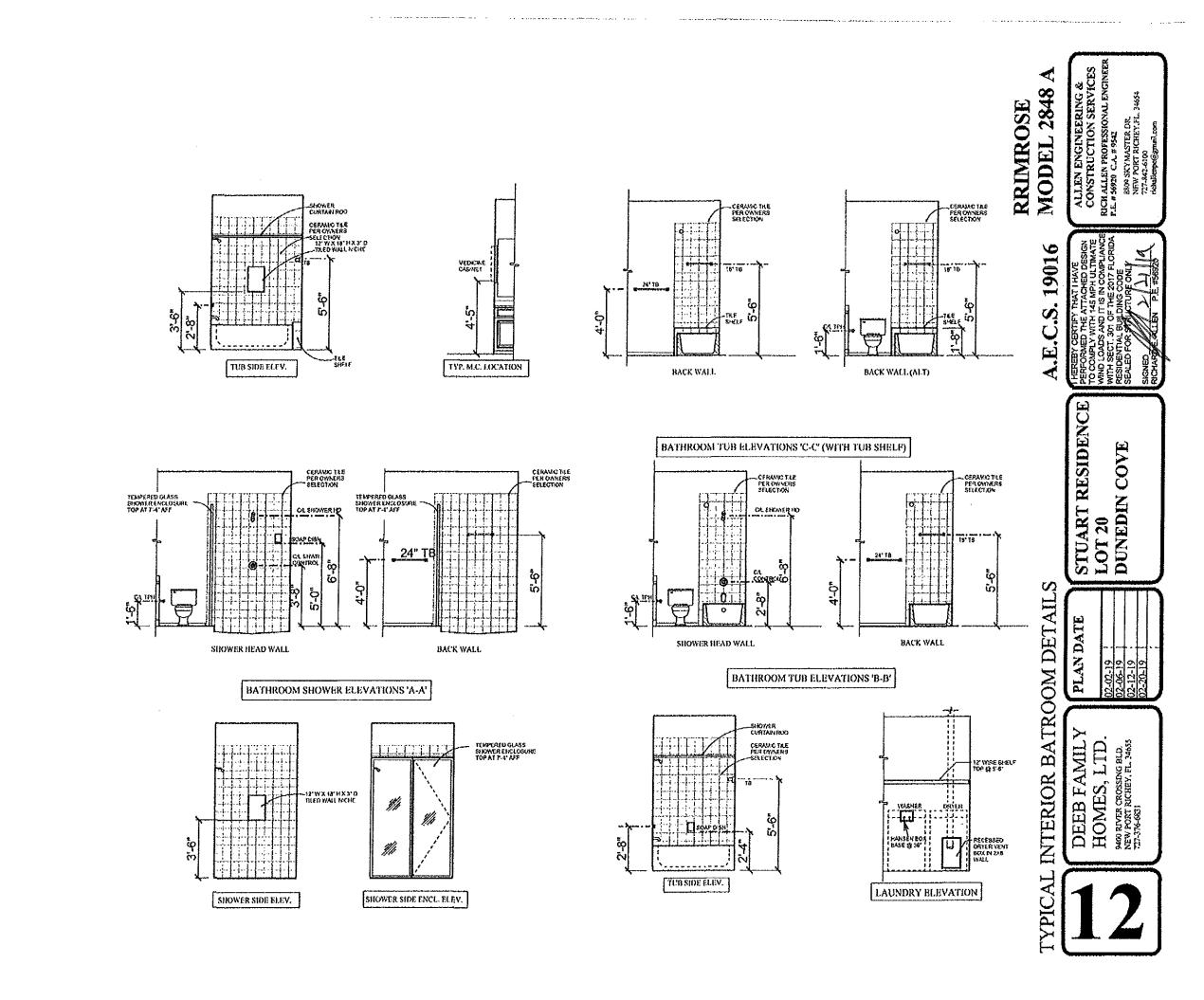
PLAN DATE

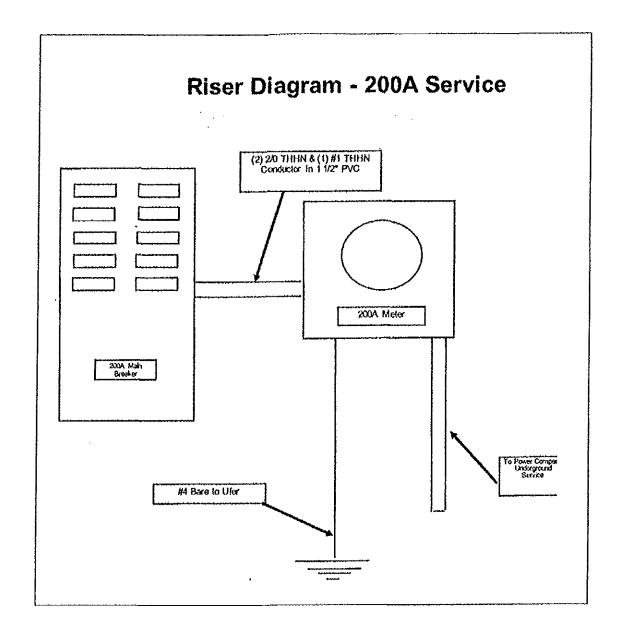
DEEB FAMILY HOMES, LTD. S400 XXVIEW CROSSING BLD. NEW PORT RICHEY, FL. 34655 ZZZZJG-6821

DETAILS CONST.









Load	d Calculatio	n		
Project Informatio	n: Lot 20		Concord I	Orive
	Dunedin	, F	34698	
Description	Qty.		Qty.	Watts
Sq. Ft. x 3 Watts	2725	Х	3	817
Small Appliance Branch	4	Х	1500	6000
Laundry	1	Х	1500	1500
Disposal	1	Х	1080	1080
Dishwasher	1	X	1300	1300
Range	1	Х	8000	8000
Oven		Х	9600	(
Cook Top		Х	9000	(
Jen Air		X	7680	(
Water Heater		Х	4500	
Dryer	1	Х	5000	5000
Microwave	1	Х	1200	1200
Jacuzzi		Х	2400	(
Pool		Х	1200	(
		х	7200	. (
Pool Heater		х	14400	(
Bath Fans	4	х	60	240
		Х	60	(
	32,495.00			
	(10,000.00			
	22,495.00			
	x .40%			
x .40% Sub Total =				8,998
A. A	10,000.00			
AC Name plate or 4 x Sq Ft			·····	
AC # 1	2725	x	4	10900
AC # 2		Х		C
AC # 3		X		C
AH (KW + 1000 + Fan)				
AH # 1	8kW			8900
AH # 2		1		C
AH # 3		1	<del>-,</del>	C
EV Charging station		1	12000	C
		ota	l Watts =	38,798.00
	240			
	<del></del>		vided by I Amps ≃	162

Main Breaker Size

RRIMROSE MODEL 2848 A.E.C.S. 19016 STUART RESIDENCE LOT 20 DUNEDIN COVE PLAN DATE ELECTRICAL RISER & LOAD CALCULATIONS DEEB FAMILY HOMES, LTD.

MAD RIVER CROSSING BLD.

NEW PORT RICHEY, R., 34655

277-576-6851 200