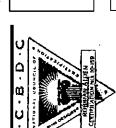
GENERAL NOTES: THE FOLLOWING TECHINCAL CODES SHALL APPLY: 2007 FLORIDA BUILDING CODE,W/ 2009 SUPPLEMENTS. PLUMBING, MECHANICAL, FUEL GAS, ENERGY EFFICIENCY, ACCESSIBILITY, AND NATIONAL ELECTRICAL CODES 1. TANK TYPE WATER CLOSET VOLUME 1.6 GALLONS 2. WALL MOUNT WATER CLOSET VOLUME 3.5 GALLONS 3. WATER - FLOW RATE. PUBLIC FACILITIES 0.5 G.P.M. PRIVATE FACILITIES 2.2 G.P.M. 2.5 G.P.M. SHOWER HEADS VTR LOCATIONS ARE APPROXIMATE AND MAY CHANGE DUE TO JOBSITE CONDITIONS THE FOLLOWING SHALL COMPLY WITH THE 2007 FBC. □ PORCHES AND BALCONIES ☐ HANDRAILS ☐ GUARDRAILS ☐ STAIRS ☐ CHIMNEY & FIREPLACE ☐ EGRESS WINDOWS 4. ALL OPENINGS SHALL COMPLY WITH 2007 FBC WIND LOADS AS STATED BELOW. ATTACHMENTS OF WINDOWS, DOORS, SLIDING GLASS DOORS AND O.H. GARAGE DOORS ARE DELEGATED THE MANUFACTURER OF THESE ITEMS. THE MANUFACTURER OF THESE ITEMS SHALL SUBMIT ATTACHMENTS TO ENGINEER OF RECORD FOR REVIEW PRIOR TO INSTALLATION. SEE ATTACHED SPECIFICATION SHEETS FOR MANUFACTURERS DESIGN CRITERIA AND INSTALLATION METHODS FOR WINDOWS, DOORS, SLIDING GLASS DOORS, OVERHEAD GARAGE DOORS, AND ROOFING. 5. ALL DOORS INTERIOR & EXTERIOR ARE 8' 0" UNLESS OTHERWISE NOTED ALL SHOWER ENCLOSURES TO BE TEMPERED GLASS 6. ALL WINDOWS WITHIN 24" OF DOORS (INTERIOR & EXTERIOR) AND WITHIN 18" OFF FLR TO BE TEMPERED GLASS.



ROBBIAN DESIGN
AL ROBBIAN A.I.B.D.
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1727 WEW PORT RICHEY, FL. 34653
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INDEX OF DRAWINGS

SHEET TITLE

> **COVER SHEET** STRUCTURAL ENGINEER NOTES S1 STRUCTURAL ENGINEER NOTES S2 **S**3 STRUCTURAL ENGINEER NOTES STRUCTURAL ENGINEER NOTES WIND LOAD DESIGN DATA \$4 \$5 **FOUNDATION PLAN FLOOR PLAN NOTES** DIMENSION PLAN **EXTERIOR ELEVATIONS** 5 **EXTERIOR ELEVATIONS** 6 **ROOF PLAN** 6A TRUSS PLAN **ELECTRICAL PLAN** CONSTRUCTION DETAILS CONSTRUCTION DETAILS 10 **TYPICAL WALL SECTIONS** 11 **TYPICAL FOOTING DETAILS**

WINDOW INSTALLATION NOTES:

DUE TO SPACE LIMITATIONS IN THIS 11"X 17" PLAN FORMAT, AND TO ELIMINATE CLUTTER AND TEXT READABILITY ISSUES, SOME DETAILS AND NOTATIONS

NOTICE TO SUBCONTRACTORS:

MAY OR MAY NOT BE LOCATED ON THE SAME SHEETS OR IN THE SAME LOCATIONS AS PROVIDED FOR BY OTHER CONTRACTORS OR ARCHITECTS. IT WOULD BE IN YOUR BEST INTREST TO REVIEW THESE PLANS AND LOCATE THE APPROPORIATE INFORMATION REQUIRED TO COMPLETE YOUR SPECIFIC PORTION OF THE JOB BEFORE BEGINNING CONSTRUCTION.

IT IS THE INTENT OF THIS DESIGNER THAT THESE PLANS ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE LICENSED PROFESSIONAL TO CONSTRUCT THIS PROJECT. IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION..STOP..AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROPESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY REVIEW THESE DOCUMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE BEFORE ANY WORK IS DONE.

NOTICE TO BUILDER

- . WINDOWS MUST BE FASTENED INTO STRUCTURAL MEMBERS PER MFG'S. DETAIL REQUIREMENTS PER DESIGN CRITERIA NOTED ON THESE DRAWINGS.
- , WINDOWS ARE NOT IMPACT RESISTANT TYPE. STORM SHUTTERS OR PANELS ARE REQUIRED.
- . ROOF ,WALLS AND WINDOW FASTENINGS MUST BE ENGINEERED AND SPECIFIED FOR CUMULATIVE INTERNAL PRESSURE AND EXTERNAL NEGATIVE (SUCTION) PRESSURES WHICH VARIES ACCORDING TO AREAS AS NOTED IN THE DESIGN CRITERIA AS NOTED ON THIS PAGE.

SHEET COVER DEEB FAMILY HOMES, LTD

PLAN DATE

WOODRUFF RESIDENCE LOT 4 CREEKSIDE PINELLAS COUNTY

ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEE P.E.# 56920 C.A.# 9542

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ATION IS ACCEP! HE PURPOSE OF RESISTANCE

ALLEN ENGINEERING AND CONSTRUCTION SERVICES, INC. RESPONSIBLE FOR THE ARCHITECTURAL DESIGN, ITS FEAT ASSOCIATED DIMENSIONS. THE ARCHITECTURAL INFORMA AS BEING ACCURATE AND IS USED BY AECS SOLELY FOR THE DETERMINING STRENCTH, FIRE PROTECTION, AND FLOOD CONSTRUCTION REQUIREMENTS.

- 1. THE ENGINEERING PIRM FOR THIS STRUCTURAL DESIGN IS ALLEN ENGINEERING AND CONSTRUCTION SERVICES, INC. HEREIN REFERRED TO AS "AECS" OR
- 2. THE ENGINEER FOR THIS STRUCTURAL DESIGN IS RICHARD E. ALLEN, PE. HEREIN REFERRED TO AS "STRUCTURAL ENGINEER".
- 3. THE STRUCTURAL ENGINEER DESIGN NOTES ARE PART OF THE STRUCTURAL DESIGN AND ARE TO BE TAKEN AS TYPICAL REQUIREMENTS UNLESS NOTED OTHERWISE, "UNO", IN THE STRUCTURAL PLANS AND STRUCTURAL DETAILS.
- 4. THE DESIGN SHOWN IN THESE PLANS CONFORMS TO THE STRUCTURAL PROVISIONS OF THE CHAPTER 16 OF THE FLORIDA BUILDING CODE 207, SECTION R301 OF THE FLORIDA RESIDENTIAL BUILDING CODE 2007, THE SECTIONS TITLED "STRUCTURAL" OF THE PLORIDA EXISTING BUILDING CODE 2007 AND ALL CODES INCLUDE THE RELATED 2009 SUPPLEMENT.
- 5. THE PURPOSE OF THESE PLANS IS TO OBTAIN A BUILDING PERMIT AND FOR SUBSEQUENT CONSTRUCTION OF THE DESIGN AS SHOWN. THESE PLANS ARE TO BE CONSIDERED VOID IF WORK COMMENCES PRIOR TO A PERMIT BEING ISSUED, A CHANGE IN THE BUILDING CODE OCCURS PRIOR TO THE PLANS BEING SUBMITTED FOR PERMIT OR AFTER SIX MONTHS OF THE DATE THAT THESE PLANS ARE SIGNED AND SEALED WITHOUT BEING SUBMITTED FOR PERMITTING, WHICHEVER OCCURS FIRST, ONCE A BUILDING PERMIT HAS BEEN ISSUED BASED ON THESE PLANS, THE BUILDING DEPARTMENT IS NOT AUTHORIZED TO REISSUE

DESIGN OF FIRE PROTECTION, ELECTRICAL, PLUMBING, AND MECHANICAL COMPONENTS OR SYSTEMS.

- 10. THE ARCHITECTURAL INFORMATION, INCLUDING DIMENSIONS, SHOWN IN THESE PLANS AND PROVIDED TO THE STRUCTURAL ENGINEER BY OTHERS IS PRESUMED ACCURATE AND IS RELIED UPON BY THE STRUCTURAL ENGINEER SOLELY FOR THE PURPOSE OF ACHIEVING COMPLIANCE WITH THE RELEVANT STRUCTURAL PROVISIONS AS STATED IN ITEM 4.
- 11. THE STRUCTURAL PLANS AND ANY RELEVANT DESIGN DOCUMENTS PRODUCED UNDER THE DIRECT CHARGE OF THE STRUCTURAL ENGINEER ARE THE PROPERTY OF THE STRUCTURAL ENGINEER AND MAY NOT BE USED BY ANY PERSON OTHER THAN THE CONTRACTED CLIENT AND FOR ANY PURPOSE OTHER THAT THAT STATED IN ITEM 5 ABOVE WITH OUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER. MOREOVER, NO OTHER ENGINEER OR ARCHITECT IS TO BE DESIGNATED A DELEGATED ENGINEER FOR ANY PURPOSE RELATED TO THESE STRUCTURAL PLANS OR CONSTRUCTION BASED ON THESE PLANS PRIOR TO THE ISSUANCE OF A CERTIFICATE OF COMPLETION OR OCCUPANCY WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL

DESIGN CRITERIA

- 12. LOAD COMBINATIONS: THIS DESIGN IS BASED ON AN "ALLOWABLE-STRESS" FORMULATION RELYING ON THE LOAD COMBINATIONS DEFINED IN FBC 2007 SECTION 1605,3.1 OR SECTION 1605.3.2 WHERB OMEGA EQUALS 1.3
- 13. FOUNDATION LOADS: SEE NOTES ON "SITE CONDITIONS, SOILS, AND FOUNDATIONS."
- FLOOR LIVE LOADS:

SITE CONDITIONS

- 18. SITE PLAN AND TOPOGRAPHY
- A. THE STRUCTURAL ENGINEER IS NOT A SURVEYOR AND IS NOT RESPONSIBLE FOR THE SITE PLAN, ESTABLISHING REQUIRED SET-BACKS, AND LOCATING THE BUILDING ON THE PROPERTY.
- B. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE GRADING OF THE SITE OR ITS COMPLIANCE WITH ANY DRAINAGE PLAN WHETHER INDIVIDUAL OR AS PART OF A MASTER DRAINAGE PLAN.

- A. IN ADDITION, THE STRUCTURAL ENGINEER IS NOT A CIVIL OR GEOTECHNICAL ENGINEER AND IS NOT RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SITE FOR CONSTRUCTION, INCLUDING ITS TOPOGRAPHY, DRAINAGE, AND SUB-SURFACE CONDITIONS (INCLUDING WATER TABLE DEPTH), AND FOR INTERPRETING GEOTECHNICAL DATA CONCERNING
- B. IF THE SOIL CONDITIONS AT THE SITE APPEAR **OUESTIONABLE AS DETERMINED BY THE BUILDING** CONTRACTOR OR OWNER-BUILDER, A SOILS ANALYSIS SHALL BE PERFORMED BY A LICENSED GEOTECHNICAL ENGINEER THAT WILL GIVE SPECIFIC RECOMMENDATIONS FOR A FOUNDATION TYPE. IF THE BUILDING CONTRACTOR OR OWNER-BUILDER DO NOT MAKE THAT DETERMINATION AND A SOILS ANALYSIS IS NOT PERFORMED, THE STRUCTURAL ENGINEER SHALL PROCEED WITH THE DESIGN BASED ON THE PRESUMPTIONS ALLOWED BY THE FBC 2007, SECTION
- C. THE DETERMINATIONS OF THE SUITABILITY OF THE SITE FOR CONSTRUCTION (INCLUDING TOPOGRAPHICAL INFORMATION) AND THE SOIL

OR TRANSFER BUILDING PERMITS WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

- 6. CONSTRUCTION BASED ON THE STRUCTURAL DESIGN IS TO BE DONE AS SHOWN IN THE PLANS WITHOUT DEVIATION, CHANGE, OR OMISSION WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. IF ADDITIONAL DETAIL INFORMATION, OR EXPLANATION IS NEEDED, IT IS TO BE OBTAINED FROM THE STRUCTURAL ENGINEER PRIOR TO THE WORK BEING DONE AND NOT AFTER THE PACT.
- 7. IT IS IMPORTANT TO UNDERSTAND THAT THE STRUCTURAL PROVISIONS OF THE BUILDING CODE ARE COMPLICATED AND THESE PLANS ARE INTENDED TO BE USED BY AND EXPERIENCED BUILDING CONTRACTOR. PROPERTY OWNERS OBTAINING OWNER-BUILDER PERMITS ARE PROCEEDING AT THEIR OWN RISK. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ERRORS OR OMISSIONS BY PROPERTY OWNERS OR THEIR AGENTS AS A RESULT OF ANY MISUNDERSTANDING OF THE PLANS THAT OTHERWISE WOULD BE UNDERSTOOD BY A LICENSED CONTRACTOR.
- 8. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, AND SCHEDULE.
- THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY PART OF THESE PLANS, INCLUDING INFORMATION CONTAINED ON A PLANS SHEET WHERE HIS SIGNATURE AND SEAL APPEAR, THAT DOES NOT PERTAIN TO THE RELEVANT STRUCTURAL PROVISIONS AS STATED IN ITEM 4, INCLUDING BUT NOT LIMITED TO THE BUILDING OCCUPANCY, THE ARCHITECTURAL DESIGN, ITS FEATURES, FINISHES (E.G. DECORATIVE STUCCO, SIDING, ROOFING, SOFFITS, FLASHING, PAINTING ETC) AND THEIR INSTALLATION, DIMENSIONS, AND ANY

A, RESIDENTIAL ONE AND TWO FAMILY DWELLINGS: ALL LIVE LOADS PER TABLE R301.5: UNINITABLE ATTICS WITHOUT STORAGE: 10 PSF UNINHABITABLE ATTICS WITH STORAGE: 20 PSF HABITABLE ATTICS AND SLEEPING AREAS: 30PSF BALCONIES: 60 PSF DECKS: 40 PSF STAIRS: 40 PSF ALL OTHER ROOMS: 40 PSF QUARDRAILS/HANDRAILS: 200 LB CONCENTRATED

LOAD APPLIED IN ANY DIRECTION B. COMMERCIAL
ALL LIVE LOADS PER FBC 2007 TABLE 1607.1

15. ROOF LIVE LOADS

ALL ROOF WOOD CONSTRUCTION TYPES ARE 30 PSF

16. DBAD LOADS

FLOOR WOOD FRAME: 35 PSF FOR TILE/MARBLE FLOOR COVERING, 15 PSF FOR ALL OTHER ROOF WOOD FRAME: 25 PSF FOR SHINGLES, 35 PSF FOR

- 17. WIND LOADS

 A. WIND LOADS ARE BASED ON THE SPECIFIC REQUIREMENTS AND DEFINITIONS OF PBC 2007, SECTION 1609, AND ON THE METHODOLOGY DESCRIBED IN ASCE 7, SECTION 6, AND THE SITE SPECIFIC CONDITIONS
- B. THE COMPONENT AND CLADDING WIND PRESSURES ARE THE MINIMUM REQUIREMENTS FOR STRENGTH AND IMPACT PROTECTION NEEDED FOR SELECTING SATISFACTORY COMPONENTS AND CLADDING, BY OTHERS, FOR THE STRUCTURE.
- C. SEB WIND LOAD TABLE FOR PROJECT SPECIFIC WIND LOADING DESIGN AND COMPLIANCE REQUIREMENTS

CONDITIONS SHALL HAVE BEEN COMPLETED AND ANY RECOMMENDATIONS RESULTING FROM THAT ANALYSIS SHALL HAVE BEEN PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO THE SIGNING AND SEALING OF THE STRUCTURAL PLANS.

- D. IN THE ABSENCE OF GEOTECHNICAL INFORMATION, THE SITE IS PRESUMED TO HAVE AN ALLOWABLE SOIL BEARING CAPACITY OF 2000 PSF, AND THE TOPOGRAPHY AS IT RELATES TO THE STRUCTURE IS PRESUMED TO BE THAT SHOWN IN THE ARCHITECTURAL ELEVATIONS. THE FOUNDATION DESIGN IS BASED ON THESE PRESUMED CONDITIONS INCLUDING THAT DIFFERENTIAL SETTLING DOES NOT EXCEED THE SAFE LIMITS OF THE FOUNDATION DESIGN (INCLUDING STEMWALLS AND MASONRY ABOVE GRADE WALLS) AS STATED IN ITEM 19.3 BELOW.
- E. IT IS IMPORTANT TO KNOW THAT THE FOUNDATION DESIGN BASED ON A PRESUMED ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF RELIES ON LESS THAN L/500 (E.O., 0.25 INCHES OVER 10 PEET) OF DIFFERENTIAL SETTLEMENT. CRACKS IN MASONRY WALLS SHOULD BE EXPECTED WHERE DIFFERENTIAL SETTLEMENT EXCEEDS L/300 (E.G. 0.4) INCHES OVER 10 FEET) AND STRUCTURAL DAMAGE SHOULD BE EXPECTED WHERE DIFFERENTIAL SETTLEMENT EXCREDS 1/150. THIS STATEMENT SHOULD BE TAKEN AS A CAUTIONARY NOTE FOR PROCEEDING WITHOUT A SOILS ANALYSIS AND FOUNDATION RECOMMENDATION BY A GEOTECHNICAL ENGINEER FOR
- P. COPIES OF ANY AND ALL REQUIRED COMPACTION TESTS ARE TO BE PROVIDED TO THE BUILDING DEPARTMENT FOR THEIR RECORDS.

STRUCTURAL ELEMENTS

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PLAN DATE

GINEER

TRUCTURAL

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A. THE FOUNDATION AND FOOTINGS ARE TO BEAR A MINIMUM OF 12 INCHES BELOW GRADE AND ARE TO BE PLACED ON UNDISTURBED SOIL OR FILL COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR PURSUANT TO ASTM D 1557 WITH FILL LIFTS LESS THAN 12".

B. FOOTINGS (AND ANY ASSOCIATED MONOLITHIC FLOOR SLAB) SHALL BE CONSTRUCTED OF CONCRETE WITH A SPECIFIED COMPRESSIVE STRENGTH OF 3,000 PSI, 3 TO 5 INCH SLUMP, AND 3/8" AGGREGATE.

C. THE SIZE AND REQUIRED REINFORCEMENT FOR THE FOOTINGS ARE SHOWN IN THE FOUNDATION PLAN.

D. THE GROUND FLOOR SLAB SHALL BE PLACED OVER A 6 MIL POLYETHYLENE MOISTURE RETARDER WITH MINIMUM 6 INCH OVERLAPS OF JOINTS.

E. TERMITE TREATMENT OF THE SITE SHALL BE SPECIFIED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER.

P. SHRINKAGE CONTROL OF THE FLOOR SLAD SHALL BE ACCOMPLISHED BY 6 INCH BY 6 INCH, WL4 BY WL4 WELDED WIRE FABRIC AS SPECIFIED BY FBC 2007 SECTION 1910.2, EXCEPTION 2 OR FIBERMESH ADMIXTURE AS SPECIFIED BY FBC 2007, SECTION 1910.2 EXCEPTION 1. THE WELDED WIRE FABRIC SHALL BE PLACED BETWEEN THE MIDDLE AND UPPER 1/3 DEPTH OF THE SLAB AND HELD IN POSITION BY APPROPRIATE SUPPORTS SPACED NOT ORBATER THAN 3 FEET APART.

G. CONTRACTION JOINTS ARE TO BE PROVIDED FOR THE PURPOSE OF CONTROLLING SHRINKAGE. ONE INCH DEEP CUTS (FOR A FOUR INCH THICK SLAB OR 25 PERCENT OF THE SLAB THICKNESS OTHERWISE) ARE TO BE PROVIDED ACROSS THE WIDTH AND LENGTH OF ANY FLOOR SLAB AT A DISTANCE NOT TO EXCEED 30 TIMES THE SLAB THICKNESS, FOR EXAMPLE FOR A FOUR INCH

COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION OF THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED UPON THE FINAL FLOOR TRUSS SYSTEM

B. CONVENTIONAL PRAMED JOISTS FLOOR JOISTS ARE SIZED BASED ON THE SOUTHERN PINB COUNCIL SPAN TABLES FOR NO. 2 GRADE DIMENSIONAL LUMBER.

FLOOR JOISTS FOR EXTERIOR DECKS SHALL BE PRESSURE TREATED.

FOR ALL WOOD FLOORS

THE TRUSS TO WALL CONNECTIONS ARE IDENTIFIED ON THE FLOOR FRAMING PLAN.

A STRUCTURAL WOOD BAND JOIST IS TO BE PROVIDED ON THE EXTERIOR PERIMETER OF ALL BOTTOM BEARING FLOOR TRUSSES AND JOISTS, THE STRUCTURAL BAND JOIST IS TO BE FASTENED TO EACH END OF A FLOOR TRUSS OR JOIST WITH A SIMPSON L50 BRACKET USING SIMPSON SHORT 10D COMMON NAILS.

FLOOR TRUSSES OR JOISTS BEARING ON WOOD WALLS ARE TO BE SET WITH A MINIMUM OF THREE 10D COMMON NAILS (TOE NAILED) TO THE TOP PLATE OF THE

IV. A MOISTURE DARRIER SHALL BE INSTALLED BETWEEN ANY UNTREATED WOOD TRUSSES OR JOISTS AND CONCRETE OR MASONRY.

LEDOBRS/NAILERS SHALL BE FASTENED TO WOOD STUDS OR BAND JOISTS (NOT SHEATHING) WITH A MINIMUM OF 2- 3/8" X 5 H" LAO BOLTS WITH WASHERS AT EACH STUD INTERSECTION OR 16 INCHES ON CENTER AND SHALL CONSIST OF PRESSURE TREATED LUMBER 2

FLOOR SHEATHING SHALL BE FASTENED TO THE FLOOR TRUSSES/JOISTS WITH JOD RING SHANK NAILS AT 6" ON CENTER WITH CONSTRUCTION GRADE ADHESIVE. FLOOR SHEATHING SPECIFIED FOR SEALED EXTERIOR DECKS AND ITS INSTALLATION SHALL BE THE SAME AS THAT FOR INTERIOR APPLICATION EXCEPT PRESSURE TREATED AND THE FASTENERS SHALL BE GALVANIZED.

E. EXTERIOR DECK FLOORING

I. DECK FLOORING SHALL BE INDIVIDUALLY SPECIFIED ON THE PLOOR PRAMING PLANS AND SHALL BE PASTENED TO THE UNDERLYING PRESSURE TREATED JOIST WITH 3 - 3 INCH DECK SCREWS AT EACH FLOORING/JOIST INTERSECTION.

22, WALLS

A. MASONRY

CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM COMPRESSIVE STRENOTH OF 1900 PSI.

WALL CMU SHALL BE 8 INCH BY 8 INCH X 16 INCH IN SIZE OR 8 INCH X 8 INCH X 8 INCH FOR EDGE FINISHES. CMU SHALL DE PLACED IN A RUNNING BOND AND

THERE SHALL BE NO VERTICAL BUTT JOINTS EXCEPT AS SHOWN ON THE FLOOR PLAN FOR CONSTRUCTION

IV. REINFORCED FILLED CELLS AS SHOWN IN THE I'LANS SHALL BE FILLED WITH A "FINE" GRADE GROUT, HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI. AND 8 TO 11 INCH SLUMP TO ENSURE CONSOLIDATION. V. BOND BEAMS SHALL BE POURED WITH OROUT MONOLITHICALLY WITH THE PILLED WALL CELLS - NO COLD JOINTS.

VERTICAL STEEL REINPORCEMENT SHALL BE CONTINUOUS BETWEEN THE MIDDLE AND BOTTOM 1/3 OF THE FOOTING HEIGHT AND END IN THE TOP COURSE OF

THICK SLAB, CONTRACTION JOINTS SHALL NOT EXCEED 10 FEET ON CENTER EACH WAY, THE CONTRACTION JOINTS ARE OPTIONAL FOR ONE AND TWO FAMILY RESIDENTIAL WHEN WELDED WIRE FABRIC OR FIBERMESH ARE USED IN THE PLOOR SLAB.

21. FLOORS

A. MANUFACTURED WOOD TRUSSES

THE MANUFACTURED FLOOR TRUSS FRAMING PLAN CONTAINED HERBIN IF THE FOR THE OLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPONENT AND TRUSS SYSTEM ENGINEERS OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL FLOOR TRUSS SYSTEM DESIGN. IT IS NOT TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DUTERBUT FROM THE FINAL DESIGN.

MANUFACTURED FLOOR TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEUR AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.

III. THE MANUPACTURED TRUSS DESIGN SHALL INCLUDE SPECIFYING THE TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTIONS ON EITHER THE INDIVIDUAL TRUSS COMPONENT SHEETS OR THE GIRDER TRUSS COMPONENTS SHEETS AS APPLICABLE, A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR BACH LOCATION A HANGER IS REQUIRED IN THE TRUSS

IV. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR

PLY 1 33" THICK BY A HEIGHT AS SHOWN IN THE PLANS. FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8 INCH BY 5 1/2 INCH SIMPSON TITEN HD CONCRETE BOLTS.

VI. FLOOR BEAMS

BBAMS SUPPORTING FLOOR TRUSSES AND JOISTS ARE TO BE ATTACHED AS SPECIFIED IN THE FLOOR

2. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING PINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE COMMNUOUS BETWEEN BEARING POINTS.

3. MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (E.G. GLULAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURER'S SPECIFICATIONS.

4. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE THE INDIVIDUAL PIJES INTERCONNECTED AS POLLOWS:

A. FOR TWO PLY BEAMS - ONE ROW OF 10D GALVANIZED COMMON NAILS AT 6" O.C, ON EACH SIDE OF THE BEAM,

B. FOR THREE PLY BEAMS - TWO ROWS OF 16D GALVANIZED COMMON NAILS SPACED AT 6" O.C. (TOP AND BOTTOM) THRU EACH SIDE OF THE BRAM

C. FOR FOUR PLY BEAMS AND LARGER - TWO ROWS OF IS INCH DIAMETER CARRIAGE BOLTS OR ALL THREAD ROD WITH NUTS AND WSHERS SPACED AT 12 INCHES ON CENTER 2 INCHES FROM THE TOP AND BOTTOM BODES OF

D. PLOOR SHBATHING:

ALL PLOOR SHEATHING IS TO DE X INCH TONGUE AND GROOVE PLYWOOD RATED FOR PLOOR SHEATHING THE BOND BEAM WITH A STANDARD 10 INCH 90 DEGREE

VIL HORIZONTAL REINFORCING STEEL SHALL BE CONTINUOUS, INCLUDING AROUND CORNERS.

III. REINFORCING STEEL SPLICES SHALL CONSIST OF WIRE LAPS NO LESS THAN 40 TIMES THE STEEL BAR DIAMETER (E.G. 25 INCHES FOR #5 REBAR, 15 INCHES FOR # 3 REBAR, AND 52 INCHES FOR # 7 REBAR).

B. WOOD PRAME WALLS

WALL STUD SIZES ARE SHOWN IN THE TYPICAL WALL SECTION

H. LOAD BEARING

WOOD STUDS IN WALLS SHALL BE SPACED AT 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES PER THE TOP PLATE SPLICE DETAIL. ALL LOAD BEARING STUDS TO BE SOUTHERN YELLOW PINE #2 GRADE OR BETTER.

2. LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED IN CONTACT WITH MASONRY OR CONCRETE) AND A DOUBLE TOP PLATE. SEE THE TOP PLATE SPLICE DETAIL FOR TOP PLATE NAILING AND SPLICING REQUIREMENTS.

3. THE WOOD STUDS SHALL HAVE A SIMPSON SP2 AT THE TOP PLATE AND A PROPERLY SIZED SPH FOR THE BOTTOM PLATE (E.G. 4" STUD WALL = SPH4, 6" STUD WALL = SPH6)

4. A 3 STUD PACK SHALL BE INSTALLED DIRECTLY DENEATH BEARING POINTS OF ALL GIRDERS AND BEAMS HAVING GRAVITY LOADS OF UP TO 3000 LBS.

5. STEEL TUBE COLUMNS SHALL BE INSTALLED IN THE WALL DIRECTLY BENEATH GIRDERS AND DEAMS HAVING GRAVITY LOADS GREATER THAN 3000 LDS.

6. BASE PLATES SHALL BE FASTENED TO MONOLITHIC FOOTINGS WITH 5/8 INCH BY 8 INCH ANCHOR BOLTS OR SIMPSON TITEN HD CONCRETE BOLTS

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DEEB FAMILY HOMES,

- 7. BASE PLATES BEARING ON WOOD SHALL BE FASTENED WITH 16D COMMON NAILS AT 8 INCHES ON CENTER THROUGH ANY FLOOR SHEATHING AND TO UNDERLYING LUMBER (NOT SHEATHING ONLY AND USE BLOCKING AS NEEDED TO MAINTAIN NAILING SPACING REQUIREMENT.
- 8. FOR EXTERIOR LOAD BEARING WALLS, EACH STUD ABOVE THE BASE PLATE SHALL BE FASTENED TO THE UNDERLYING BAND JOIST OR BRAM WITH A SIMPSON LSTA 18 STRAP. FOR THIS SITUATION THE SIMPSON SPH BRACKET TO THE BASE PLATE MAY BE OMITTED.
- 9. FOR INTERIOR LOAD BEARING WALLS, IS INCH ALL THREAD ROD SHALL BE INSTALLED AT 32" O.C. FROM '171E BASE PLATE THROUGH THU SHEATHING AND TOP PLATE OF UNDERLYING SUPPORTING WALL ALL CONNECTIONS SHALL INCLUDE A STANDARD 3 INCH SQUARE WASHER.
- 10. HEADER BEAMS SHALL BE SIZED ACCORDING TO THE ENCLOSED HEADER SCHEDULE AND PASTENED WITH A MINIMUM OF TWO SIMPSON LSTA 36 STRAPS OVER THE BACH END TO THE JACK STUDS BELOW. IN ADDITION, THE HEADER BEAMS SHALL BE FASTENED WITH A MINIMUM OF 3-10D COMMON NAILS (TOE NAILED ON EACH SIDE AT RACH END TO THE ABUTTING FULL LENGTH STUDS.
- III. NON-LOAD BEARING WALLS
- WOOD STUDS IN WALLS SHALL BE SPACED AT 16 INCRES ON CENTER AND PASTENED TO THE TOP AND BOTTOM PLATES WITH A MINIMUM OF THREE JOD COMMON NAILS. NAILS INSTALLED IN PRESSURE TREATED WOOD SHALL BE GALVANIZED.

- 2. NON LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED AGAINST MASONRY AND CUNCRETE) AND A SINGLE TOP PLATE
- 3. BASE PLATES SHALL BE FASTENED TO CONCRETE SLABS WITH M INCH BY 3 M INCH TAPCON SCREWS AT 12" ON CENTER
- 4. BASE PLATES ON WOOD SHALL BE PASTENED WITH 16D COMMON NAILS AT 8 INCHES ON CENTER.
- C. SHEATHING
- L. PLYWOOD SHEATHING

 1. PLYWOOD SHEATHING

 1. EXTERIOR WALL SHEATHING COVERED BY AN ARCHITECTURAL FINISH SHALL BE MINIMUM 7/16 INCH
 THICK (NOMINAL) 4 PLY PLYWOOD MANUFACTURED WITH EXTERIOR OLUB.
- 2. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE WALL STUDS.

 3. FASTEN TO STUDS AND BLOCKING WITH 8D RING
- SHANK NAILS AT 4 INCHES ON CENTER ALL LOCATIONS,
 4. IN ADDITION TO THE REGULAR FASTENING, A 2¹⁰⁰
- ROW SHALL BE INSTALLED AT THE DOUBLE TOP PLATE
 AND TO THE LOWEST HORIZONTAL WOOD MEMBER ON
 AN EXTERIOR WALL (E.G. SILL PLATE, BAND JOIST)
- AN EXTERIOR WALL (E.G. SILL PLATE, BARD 30132)
 5. FOR PLYWOOD SHEATHING COVERED WITH A
 CEMENTITIOUS EXTERIOR FINISH, ALL BUTT JOINTS NOT
 ON WALL STUDS SHALL BE BLOCKED WITH 2X BLOCKING
 TOB NAILED AT EACH END TO THE WALL STUDS WITH 3-8D COMMON NAILS.
- PARTICLE BOARD
- 1. PARTICLE BOARD IS NOT TO BE USED WITH THE EXPRESS WRITTEN CONSENT OF THE STRUCTURAL ENGINEER AND THE PROPERTY OWNER.
- THE USE OF PARTICLE BOARD SHEATHING WILL RESULT IN LESS SIJEAR STRENGTH AND MAY REQUIRE A REDESIGN OF THE WALL SYSTEM IF A REQUEST OR SUBSTITUTION IS MADE.

D. ARCHITECTURAL FINISHES

I. ARCHITECTURAL WALL FINISHES, SUCH AS STUCCO, CHMENTITIOUS COATINGS, SIDING OR PAINT ARE MENTIONED HERE ONLY FOR THE PURPOSE OF UNDERSTANDING THAT THEIR INSTALLATION AND ASSOCIATED DETAILS ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

23. COLUMNS

CONCRETE / MASONRY COLUMNS

- MASONRY COLUMNS SHALL BE CONSTRUCTED OF PILASTER CONCRETE BLOCK OR FORMED AND POURLD. WALL BLOCK SHALL NOT BE USED FOR MASONRY
- REINFORCING STEEL SHALL BE GRADE 60 AND HELD IN PLACE BY STIRRUPS SPACED AT 12 INCHES ON CENTER YBRTICALLY.
- PILASTER BLOCK COLUMNS SHALL BE FILLED WITH A FINE GROUT HAVING A MINIMUM OF COMPRESSIVE STRENGTH OF 3,000 PSI.
- IV. FORMED AND POURED COLUMNS SHALL CONSIST OF A MINIMUM OF 3,000 PSI CONCRETE, OR IN AREAS OF HIGH CHLORIDES SHCH AS NEAR THE COAST OR BODIES OF SALT WATER, THE MINIMUM SHALL BE 5,000 PSI.
- ALL MASONRY COLUMNS SHALL DEGIN AT THE FOUNDATION OR AT A MONOLITHIC FOOTING. IN NO CASE SHALL THERE DIS A BREAK OR A COLD JOINT IN THE GROUT OF A COLUMN EXCEPT AT 1 FOOT FROM THE TOP IN PREPARATION FOR INSTALLATION OF A CONCRETE
- METAL CONNECTORS AT THE TOP OP THE COLUMN FOR HOLDING WOOD BEAMS OR GIRDERS SHALL BE INSTALLED WITH THE MINIMUM EMBEDMENT OF THE ASSOCIATED FASTENER FOR THE CONNECTOR AS SHOWN IN THE PLANS. WOOD COLUMNS

I ALL LOAD BEARING WOOD COLUMNS SHALL BE A

IN CROSS SECTION SHALL BE ONLY BE USED FOR SUPPORTING OPEN WOOD DECKS WHERE THE FLOOR

MINIMUM OF #2 ORADE PRESSURE TREATED WOOD.

II DIMENSIONAL WOOD COLUMNS OF 4 INCHES BY 4 INCHES

HEIGHT ABOVE THE PLOOR BELOW IS 8 FEET OR LESS. ALL

MINIMUM CROSS SECTION OF A MINIMUM OF 6 INCHES BY 6

OTHER DIMENSIONAL WOOD COLUMNS SHALL HAVE A

UI METAL CONNECTORS AT THE BASE AND THE TOP OF WOOD COLUMNS SHALL BE OF THE TYPE THAN RESISTS LATERAL LOADS AS WELL AS UPLIFT AND GRAVITY LOADS. IN NO CASE SHALL FLAT STRAPS BE USED UNLESS

SPECIFICALLY SHOWN IN THE FRAMING PLANS OR CROSS

C. COMPOSITE COLUMNS

I. A COMPOSITE COLUMN HERE IS DEFINED AS A HOLLOW

COLUMN CONSISTING OF ANY MATERIAL SPECIFICALLY

DESIGNED BY ITS MANUFACTURER TO BE LOAD DEARING.

ARCHITECTURAL PINISH INTENDED TO FIT OVER A STRUCTURAL COLUMN AND ITS USE AND DEFAILS OF

INSTALLATION ARE NOT THE RESPONSIBILITY OF THE

MANUFACTURER, A SHOP DRAWING OR LETTER FOR THE

INSTALLATION OF THE COLUMN SHALL BE PROVIDED BY

THE STRUCTURAL ENGINEER TO SUPPLEMENT THE CONSTRUCTION PLANS AFTER THE SPECIFIC COLUMN AND MANUFACTURER HAVE BEEN IDENTIFIED.

INFORMATION SHALL DE PROVIDED TO THE STRUCTURAL

ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT

III. IN ALL CASES, THE COLUMN MANUPACTURER'S

II. LOAD DBARING COMPOSITE COLUMNS ARB A MANUFACTURED PRODUCT SUBJECT TO THE DESIGN AND LOAD BEARING CAPACITY DETERMINED BY THE

ANY OTHER TYPE OF HOLLOW COLUMN IS CONSIDERED AN

SECTION DETAILS.

STRUCTURAL ENGINEER.

FOR REVIEW PRIOR TO ITS ACCEPTANCE FOR THE STRUCTURAL DESIGN. THE INFORMATION SHALL INCLUDE THE LATERAL AS WELL AS UPLIFF AND GRAVITY, LOAD BUARING CAPACITIES.

D. STEEL TUBE COLUMNS

I. LOAD BBARING STEEL TUBE COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS OF WINCH AND BE MADE OF STEBL WITH A DESIGN YIELD STRENGTH OF 46 KSI UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN. II. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE STEEL TUBE COLUMN IS TO BE INSTALLED.

P. ALUMINUM COLUMNS

I. LOAD BEARING ALUMINUM COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS OF WINCH. IL ALL FASTENERS AND CONNECTORS FOR ALUMINUM COLUMNS SHALL BE STAINLESS STEEL OR MONEL TO AYOID CORROSION DUE TO DISSIMILAR METALS BEING IN CONTACT.

III. THE SPECIFIC CONNECTION SCHEME SHALL DE SHOWN IN THE STRUCTURAL DESIGN WHERE THE ALUMINUM COLUMN IS TO BE INSTALLED.

A. MANUFACTURED WOOD TRUSSES

I, THE MANUFACTURED ROOF TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL ROOF TRUSS SYSTEM DESIGN. IT IS NOT TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY HE DIFFERENT FROM THE FINAL DESIGN

IL MANUFACTURED ROOF TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.

III. THE TRUSS PLAN "SIGNED AND SEALED" BY THE DELEGATED ENDINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENDINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE TRUSS TO THE UNDERLYING STRUCTURE" CONNECTIONS. IV. AS PART OF THE REVIEW, THE STRUCTURAL ENGINEER WILL DETERMINE WHETHER THE TRUSS TO WALLABRAM METAL CONNECTORS SHOWN IN THE ORIGINAL PLANS ARE ACCEPTABLE OR WHETHER THEY NEED TO BE CHANGED OR SUPPLEMENTED TO ACCOMMODATE THE LOADS SHOWN IN THE TRUSS

COMPONENT SHEETS,

Y. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR VERIFYING THE DIMENSIONAL, ARCHITECTURAL, OR FORM ASPECTS OF THE TRUSS MANUPACTURER'S PLAN

WITH THE ORIGINAL PLANS.
VI. THE MINIMUM LIVE LOADS FOR THE ROOF TRUSS DESIGN IS TO DE BASED ON FDC 2007, SECTION 1607 FOR ROOF TYPE AND ROOPING MATERIAL.

VII. THE DRAD LOADS ARE LISTED IN FIEM 16 ABOVE. VIII, ALL TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTORS ARE TO BE SPECIFIED BY THE TRUSS MANUFACTURER, INCLUDING CONNECTORS FOR TRUSS TO MANUFACTURED BEAM (E.G., GLULAM OR MICROLAM) SPECIFIED BY THE TRUSS MANUFACTURER. A SPECIFIC HANGER MUST DI SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHIBBIS FOR

GINEER UCTURAL EN

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- THE TRUSS PLAN SIGNED AND STALED BY THE DBLEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS, THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION OF THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED UPON THE FINAL FLOOR TRUSS SYSTEM. IX. THE TRUSS MANUFACTURER SHALL PROVIDE ALL LATERAL BRACING REQUIREMENTS TO THE BUILDING CONTRACTOR. IF NOT, THE BUILDING CONTRACTOR IS TO NOTIFY THE STRUCTURAL ENGINEER FOR GUIDANCE. X. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH TRUSS IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 3 - 10D COMMON NAILS (TOE-
- THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH RAFTER IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 3- 10D COMMON NAILS (TOE-NAILED) II. ANY WOOD COMING IN CONTACT WITH MASONRY OR

I. IN ADDITION TO THE METAL CONNECTORS SHOWN IN

XI. A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE/MASONRY.

CONCRETE IS TO BE PRUSSURE TREATED OR A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE OR MASONRY.

III. COLLAR TIES ARE TO INSTALLED BETWEEN RAFTERS AT 2/3 OF THE RIDGE HEIGHT FROM WHERE THE RAFTERS DEAR ON WALLS. THE COLLAR TIES ARE TO BE FASTENED WITH A MINIMUM OF 4- 16D COMMON NAILS (CLINCHED) AT EA LAP JOINT.

IV. EACH RAFTER IS TO BE ATTACHED TO THE RIDGE BEAM WITH A LIGHT ANOLE HANGER AS SHOWN IN THE PRAMING PLAN. IN ADDITION, A FLAT METAL STRAP SHALL DB INSTALLED ACROSS THE RIDGE BEAM TO TWO OPPOSING RAFTERS.

V. A RIDGE BEAM TERMINATING AT A GABLE END SHALL BE SUPPORTED AS A MINIMUM BY A 3 STUD PACK COLUMN BEARING ON THE UNDERLYING WALL OR BEAM.

A. LEDGERS/SLEEPERS

23.2. CONVENTIONAL FRAME

LEDGERSMAILERS SHALL BE FASTENED TO WOOD STUDS (NOT SHEATHING) WITH A MINIMUM OF 2-3/8INCH BY 5 13 INCH LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AND NO GREATER THAN 16 INCHES ON CENTER AND SHALL CONSIST OF PRESSURE TREATED LUMBER - DBL 1 1/2 INCH BY A HEIGHT AS SHOWN IN THE PLANS. FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5 % INCH BY 5 % INCH SIMPSON TITEN HE CONCRETE BOLTS

IL SLEEPERS SHALL BE FASTENED TO UNDERLYING ROOF TRUSSES OR RAFTERS (NOT SHEATHING) WITH A MINIMUM OF 2 – 3/8 INCH BY 3 1/2 INCH LAG EQUITS WITH WASHERS AT EACH TRUSS OR RAFTER INTERSECTION AND NO GREATER THAN 24 INCHES ON CENTER AND SHALL CONSIST OF DIMENSIONAL LUMBER 1 1/3 INCH THICK BY A WIDTH AS SHOWN IN THE PLANS.

USB 2 INCH BY 4 INCH BLOCKING ATTACHED BETWEEN UNDERLYING STUDS, TRUSSES OR RAFTERS WITH A MINIMUM OF 3 10D COMMON NAILS AT EACH END IN ORDER TO SATISFY THE ON CENTER SPACING FRO THE LEDGERS OR SLEEPERS.

BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS ARE TO BE ATTACHED AS SPECIFIED IN THE ROOF

UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE CONTINUOUS DETWEEN BEARING POINTS.

MULTIPLE BEAMS CONSISTING OF MANUFACTURED BEAMS CONSISTING OF MANUFACTURED WOOD (E.O. GLULAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURER'S SPECIFICATIONS.

1. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE THE INDIVIDUAL PLIES

- INTERCONNECTED AS FOLLOWS:
- 2 FOR TWO PLY BRAMS ONE ROW OF 10D GALYANIZED COMMON NAILS AT 6" O.C., ON EACH SIDE
- 3. FOR THREE PLY BEAMS TWO ROWS OF 16D UALVANIZED COMMON NAILS SPACED AT 6" O.C. (TOP AND BOTTOM) THRU EACH SIDE OF THE BEAM
- 4. FOR FOUR PLY HEAMS AND LARGER TWO ROWS OF IS INCH DIAMETER CARRIAGE BOLTS OR ALL THREAD ROD WITH NUTS AND WASHERS SPACED AT 12 INCHES ON CENTER 2 INCHES FROM THE TOP AND BOTTOM EDGES OF
- C. SHEATHING
- I. O.S.B. SHEATHING

1. ROOF SHEATHING COVERED BY COMPOSITE ROOPING SHALL BE A MINIMUM OF 15/32 INCH THICK (NOMINAL) O.S.B. MANUFACTURED WITH EXTERIOR

ROOF SHEATHING COVERED BY TILE SHALL BE A MINIMUM OF 5 / B INCH THICK (NOMINAL) MANUFACTURED WITH EXTERIOR OLUB

3. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE ROOF TRUSS

- 4. FASTENING SHALL BE 8D RING SHANK NAILS AT 4"
 O.C BOUNDRY & EDGES & 6" O.C. IN THE FIELD WITH A
 SETBACK OF 5"-0" FROM ALL HDGES,
 5. METAL "H" CLIPS OR SOLID WOOD BLOCKING
 SHALL DE USED AT ALL UNSUPPORTED BUTT JOINTS
 DECEMBER 1 TO BLOCK OR BLOCKING OF BLOCK
- BETWEEN TRUSSES OR RAFTERS.

25. PRECAST CONCRETE LINTELS

- PRECAST AND PRESTRESSED CONCRETE LINTELS SHALL BE MANUFACTURED BY CASTCRETE AND INSTALLED PER MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS
- B. THE SIZE OF THE LINTELS SHALL BE BASED ON THE SPAN AND LOAD. REFER TO THE ATTACHED LINTEL SCHEDULE UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN FOR THE SPECIFIC LINTEL
- C. LINTEL SCHEDULE U.N.O. ON PLANS:
 - J. SPAN UP TO 3' 8F8 0B
 - II. SPAN +3' TO <6' -8F8-0B III. SPAN +6' TO >14' 8F16-1B/IT
- THE MINIMUM SPECIFIED GROUT COMPRESSIVE STRENGTH TO BE USED FOR LINTELS IS 3,000 PSF
- B. THE RBINFORCING STBEL SHALL DB ASTM GRADE
- 26. FASTENBRS / METAL CONNECTORS
 A. ALL PASTENBRS AND METAL CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE AND

ALL FASTENERS AND METAL CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE AND INSTALLED PER THE MANUFACTURERS SPECIFICATIONS

II. THESE FASTENERS DO NOT INCLUDE TYPICAL NAILS AND SCREWS WHICH MAY BE MANUFACTURED BY

C. FOLLOW ALL MANUFACTURERS SPECIFICATIONS AND INSTRUCTIONS FOR ALL FASTENERS, METAL CONNECTORS, SCREWS, NAILS ETC THAT ARE IN CONTACT WITH PRESSURE TRRATED LUMBER.

27. DIMENSIONAL LUMBUR

- A. ALL WOOD FOR LOAD BEARING WALLS SHALL BE SOUTHERN YELLOW PINE #2 OR DETTER GRADE AND STAMPED BY THE CERTIFYING AGENCY. IN ADDITION, ALL WOOD SHALL BE PRESSURE TREATED FOR INTERIOR OR EXTERIOR USE WHERE EXPOSED TO MOISTURE, PLACED WITHIN 12 INCHES OF SOIL OR IN CONTACT WITH MASONRY OR CONCRETE.
- 28. STRUCTURAL SHEATHING
- A. ALL SHEATHING USED FOR EXTERIOR APPLICATIONS SHALL BE EXTERIOR GRADE AND ADA STAMPED VERIFYING ITS RATING.

- 29. MASONRY
 A. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENOTTI OF 1990 FSI
 B. CONCRETE MASONRY UNITS SHALL CONFORM
 WITH AMERICAN CONCRETE INSTITUTE STANDARD 590.
- C. MORTAR SHALL DE OFTYPE M OR S GRAY

30. GROUT

A. ALL GROUT SHALL BE A FINE TYPE HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI UNLESS SPECIFICALLY SHOWN OTHERWISE BY A

MANUFACTURER PURSUANT TO GROUT USE WITH ITS

31.REINFORCING STEEL (GENERAL)

31.1 ALL REINFORCING STEEL SHALL BE ASTM GRADE 40 EXCEPT GRADE 60 SHALL DE USED FOR GRADE BEAMS, ALL LINTELS TYPES (E.G., PRECAST AND FIELD FORMED). AND COLUMNS UNLESS OTHERWISE SHOWN IN THE STRUCTURAL PLANS.

32. STRUCTURAL STEEL AND CONNECTION ACCESSORY MATERIAL (GENBRAL):

32.1 I-DEAMS, FORMED STRUCTURAL STEEL, FLAT BAR OR PLATE SHALL BE ASTM ORADE A36 UNLESS STATED OTHER WISE.

32.2 ALL STRUCTURAL STEEL SHALL HAVE A MINIMUM OF TWO COATS OF FRIMER AND TWO COASTS OF BPOXY AS A CORROSION PROVENTIVE, THE BUILDING CONTRACTOR MAY VARY FROM THIS SPECIFICATION WITH THE APPROVAL OF THE STRUCTURAL ENGINEER IF IT CAN BE DEMONSTRATED ANOTHER MEANS OF CORROSION CONTROL IS EQUALLY EFFECTIVE.

32.3 ALL WELDING OF STRUCTURAL STEEL SHALL BE MADE WITH E60/70 TYPE BLECTRODES. THE DEPTH AND LENGTH FOR THE WELD SHALL BE SPECIFIED IN THE STRUCTURAL DESIGN FOR THE SPECIFIC CONNECTION

33. VENTILATION [GENERAL]

33.1 THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR DETERMINING VENTILATION REQUIREMENTS OF CRAWL SPACES, FLOORS, AND ATTICS NOR THE MEANS AND MITHODS FOR IMPLEMENTING THESE REQUIREMENTS.

OTE **UCTURAL ENGINEER**

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HOMES,

34.1 ANY RENDERING OR NOTES OF WATERPROOFING MEASURES FOR BASEMENTS OR HALF-BASEMENTS SHOWN IS THESE PLANS WHERE A SPECIFIC CONSTRUCTION DETAIL IS NOT SHOWN IN THE STRUCTURAL DESIGN IS AN ARCHITECTURAL ILLUSTRATION ONLY AND IS NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

34.2 WATERPROOFING MEASURES ABOVE GRADE [E.G., FLASHING, CAULKING, SHAPE, AND LOCATION OF CRICKETS] ARE ASSOCIATED WITH ARCHITECTURAL FINISHES AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

35. FIRE RESISTANT DESIGN [GENERAL]

35.1 FIRE RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON UNDERWRITER'S LABORATORY OR GYPSUM ASSOCIATION DESIGN FOR FIRE RATED FLOOR, WALL, AND ROOF ASSEMBLIES.

36. FLOOD RESISTANT DESIGN [GENERAL]:

36.1 FLOOD RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON THE REQUIREMENTS STATED IN TITLE 44 CFR SECTIONS 59 AND 60, AND ON THOSE OF THE INDIVIDUAL COMMUNITY RATING AGENCIES FOR THE GOVERNMENTAL

JURISDICTION WHERE THE CONSTRUCTION IS TO BE

36.2 HOWEVER, THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR IDENTIFYING AND SHOWING ON THE PLANS THE FLOOD ZONE CATEGORY, BASE FLOOD ELEVATION, AND THE FLOOR AND STORY HEIGHTS OF THE BUILDING IN RELATION TO THE BASE FLOOD ELEVATION. THIS INFORMATION IS CONSIDERED ARCHITECTURAL AND SITE RELATED AND SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT.

37. SPECIAL CONSTRUCTION (GENERAL):

37.1 ALUMINUM STRUCTURES OTHER THAN

STRUCTURAL ALUMINUM COLUMNS.

I. ANY ALUMINUM STRUCTURES SHOWN IN THESE PLANS SUCH AS PORCH AND POOL ENCLOSURES OR GUARDRAILS AND HANDRAILS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

II. WHERE THE ALUMINUM, STRUCTURE ATTACHES TO THE MAIN STRUCTURE OR IS INCORPORATED IN THE MAIN STRUCTURE, SHOP DRAWINGS FOR THESE STRUCTURES SHALL BE PROVIDE TO THE STRUCTURAL ENGINEER TO DETERMINE THEIR EFFECT ON THE MAIN STRUCTURE.

37.2 SWIMMING POOLS

L ANY SWIMMING POOLS OR HOT TUBS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER. 37.3 FENCES AND RETAINING WALLS

1. ANY RENDERINGS OF FENCES, RETAINING WALLS, OR EXTERIOR PLANTERS SHOWN IN THESE PLANS WHERE A SPECIFIC STRUCTURAL DETAIL IS NOT SHOWN FOR THEIR CONSTRUCTION ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

37.4 DRIVEWAYS AND SIDEWALKS

I. ANY DRIVEWAYS OR SIDEWALKS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

WIND LOADING AND DESIGN PRESSURES:

- 1. FLORIDA RESIDENTIAL BUILDING CODE 2007 EDITION WITH 2009 SUPPLEMENT, ASCE 7-05
- 2. BASIC DESIGN WIND SPEED: 123 MPH
- 3. WIND IMPORTANCE FACTOR: 1.0
- 4. BUILDING CATEGORY: II
- 5. WIND EXPOSURE: B= 1.0
- 6. INTERNAL PRESSURE COEFFICIENT: +/- .18 ENCLOSED
- 7. WIND BORNE DEBRIS ZONE SHUTTERS REQUIRED
- 8. COMPONENTS AND CLADDING PRESSURES (PSF):

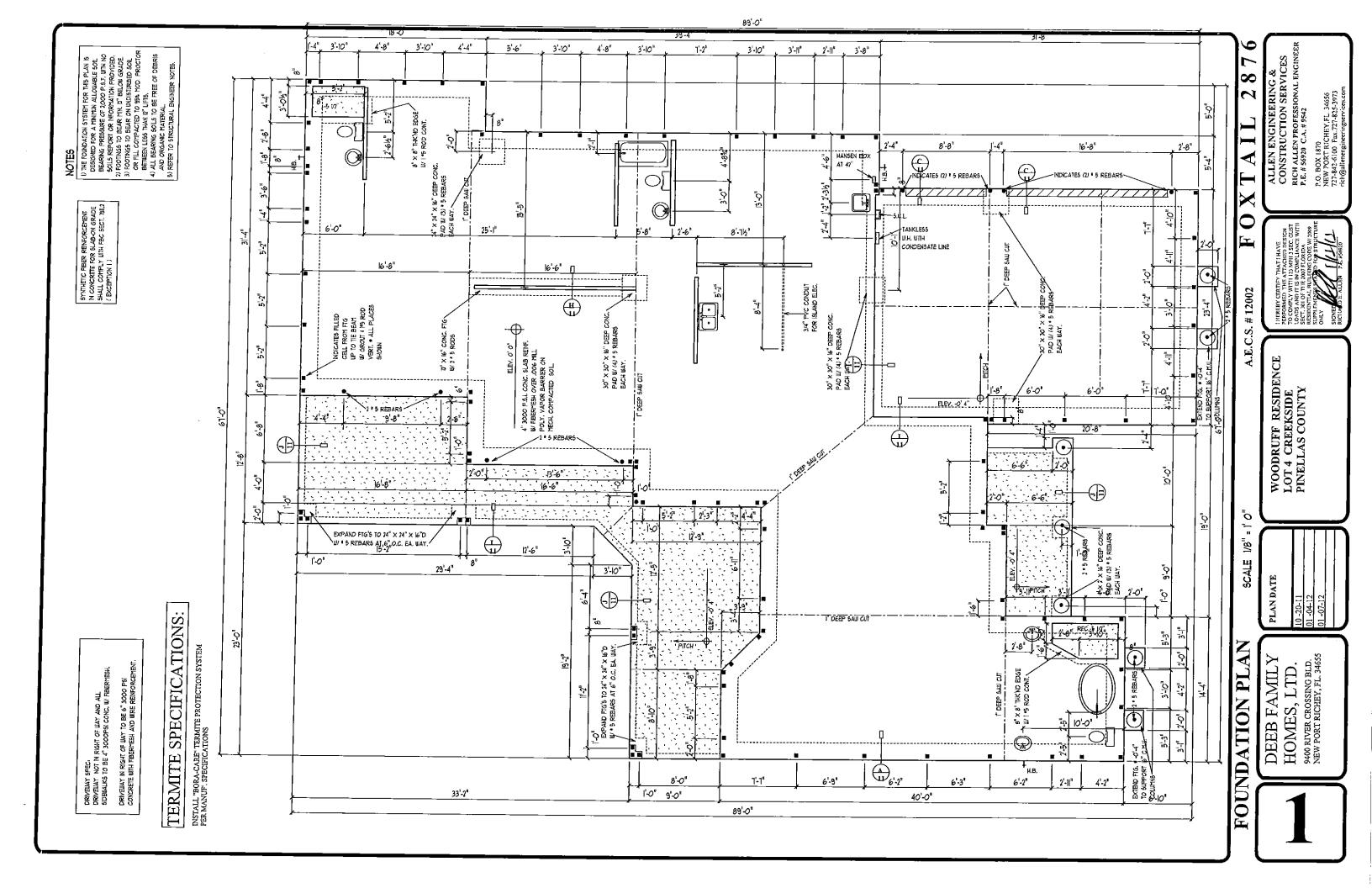
A. ROOF – ALL ZONE 3, 10 SQFT: +17.5, -58.7 PSF B. WALLS – ALL ZONE 5, 10 SQFT: +30.4, -40.7 PSF

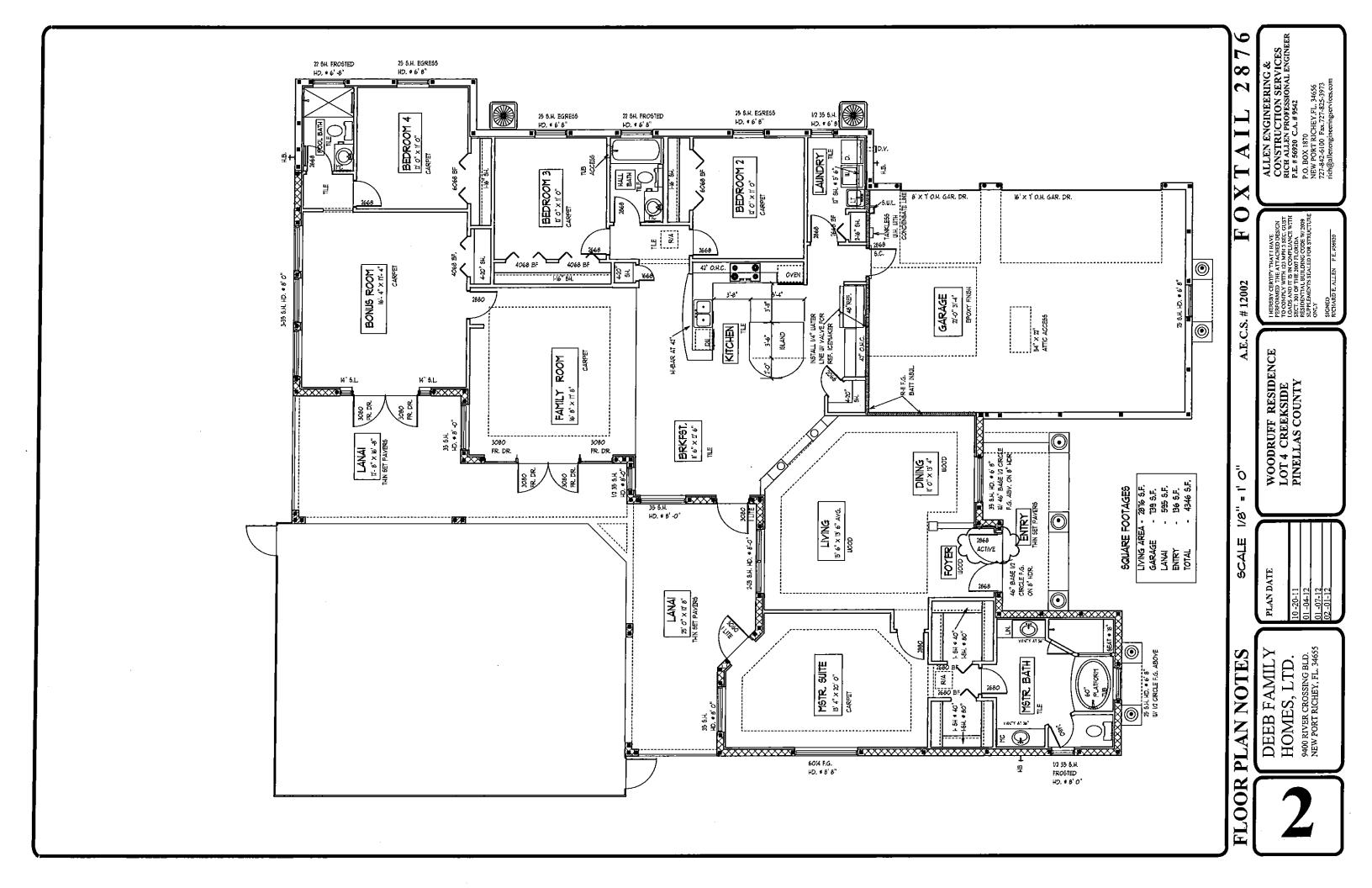
C. OVERHANGS – ALL ZONE 3, 10 SQFT: -95.3 PSF

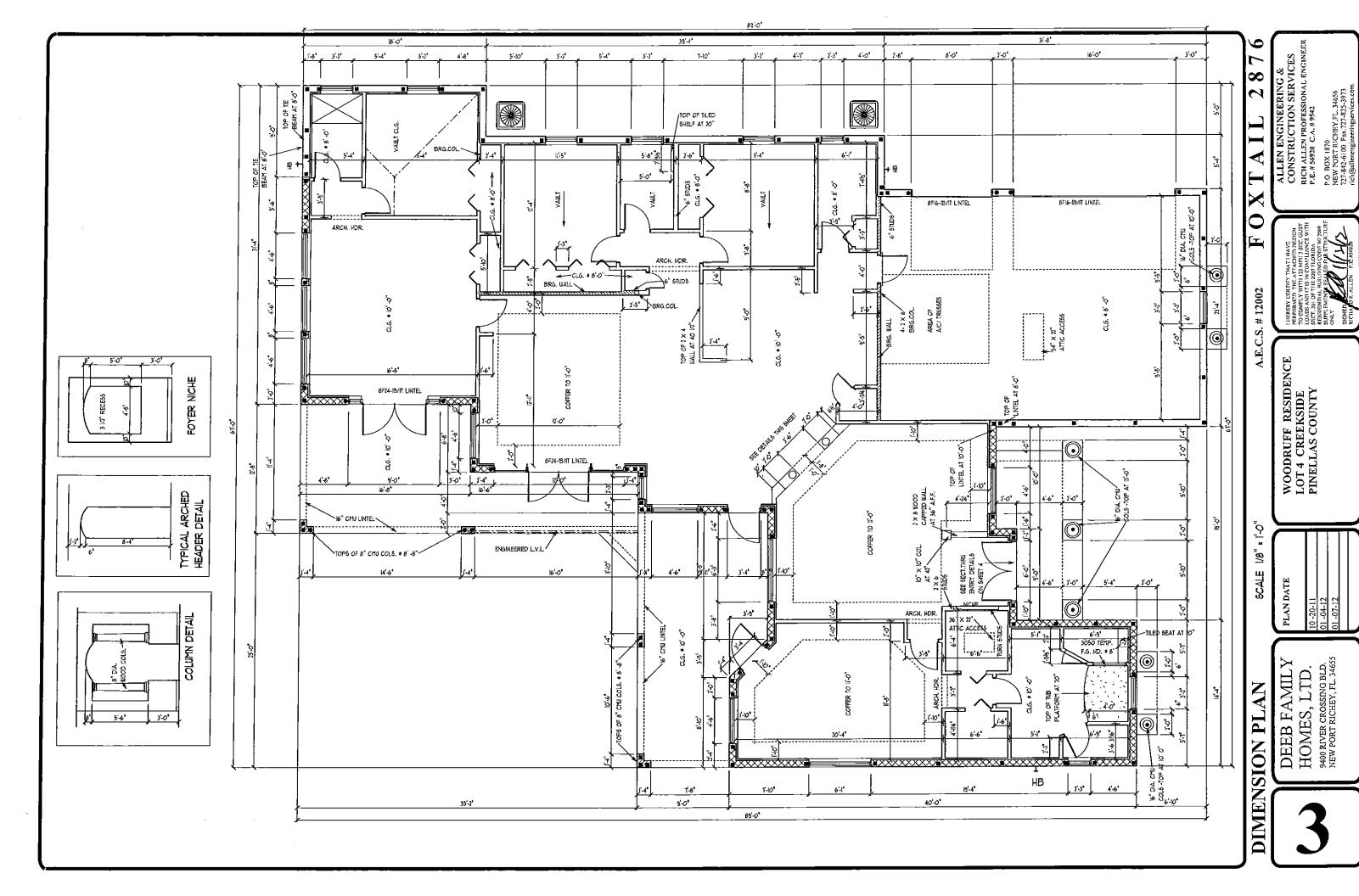
MANUFACTURE TO PROVIDE SPECS OF MINIMUM PRESSURE RESISTANCE BASED UPON ABOVE COMPONENTS AND CLADDING PRESSURES AND INSTALLATION DETAILS.

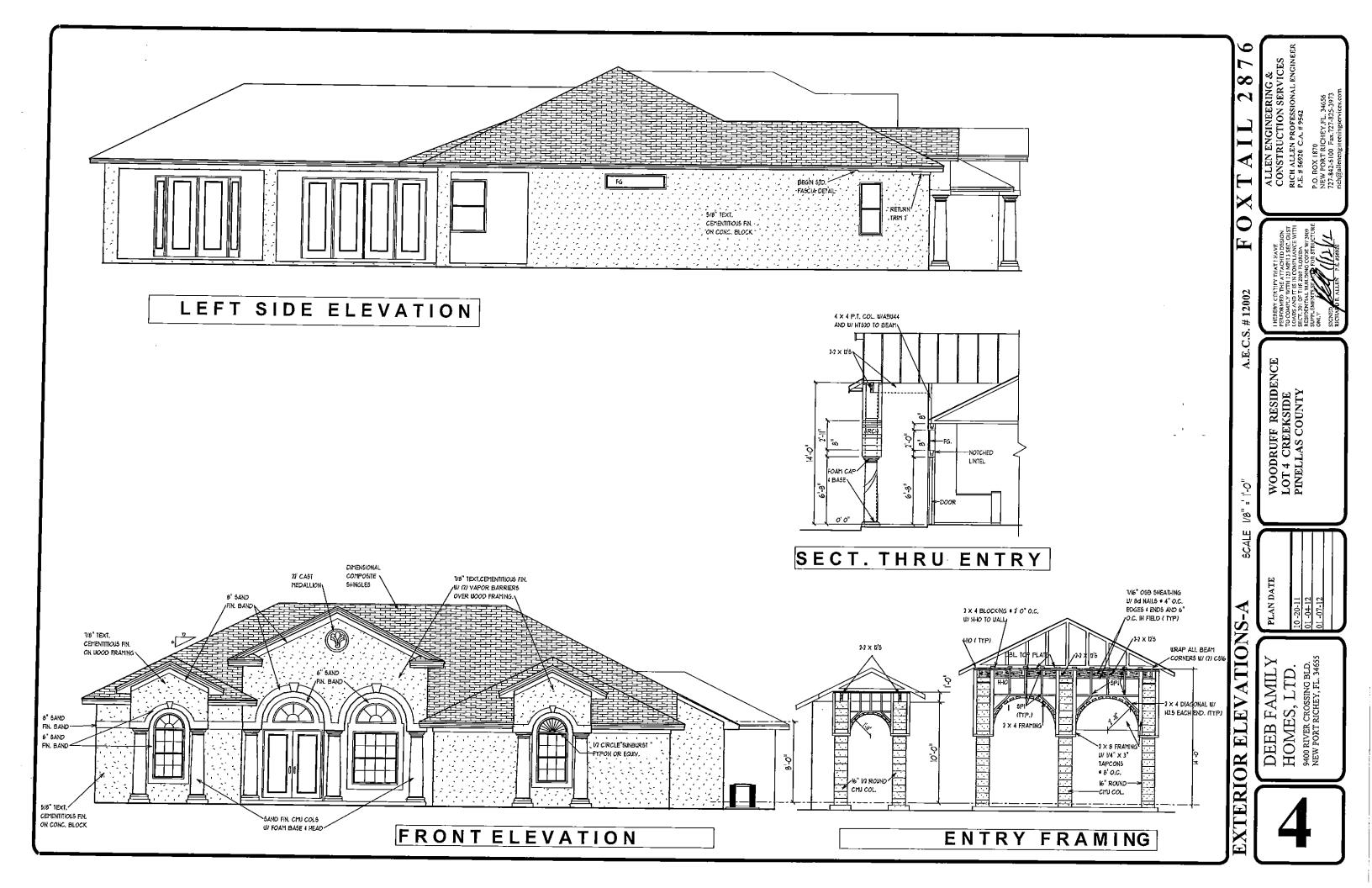
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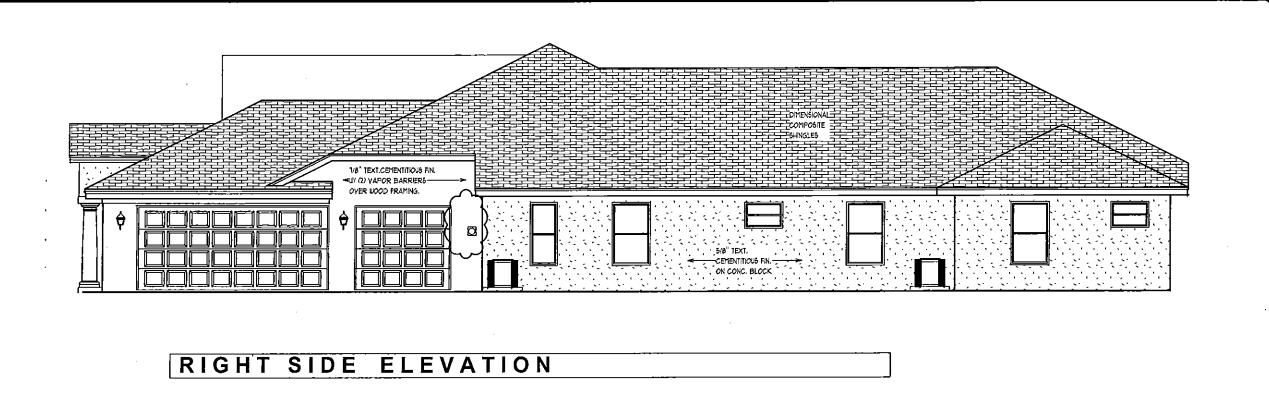
WIND LOAD DESIGN DAT

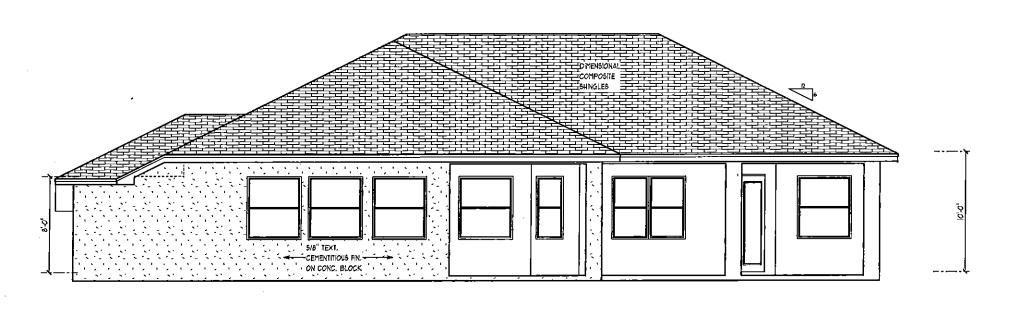












REAR ELEVATION

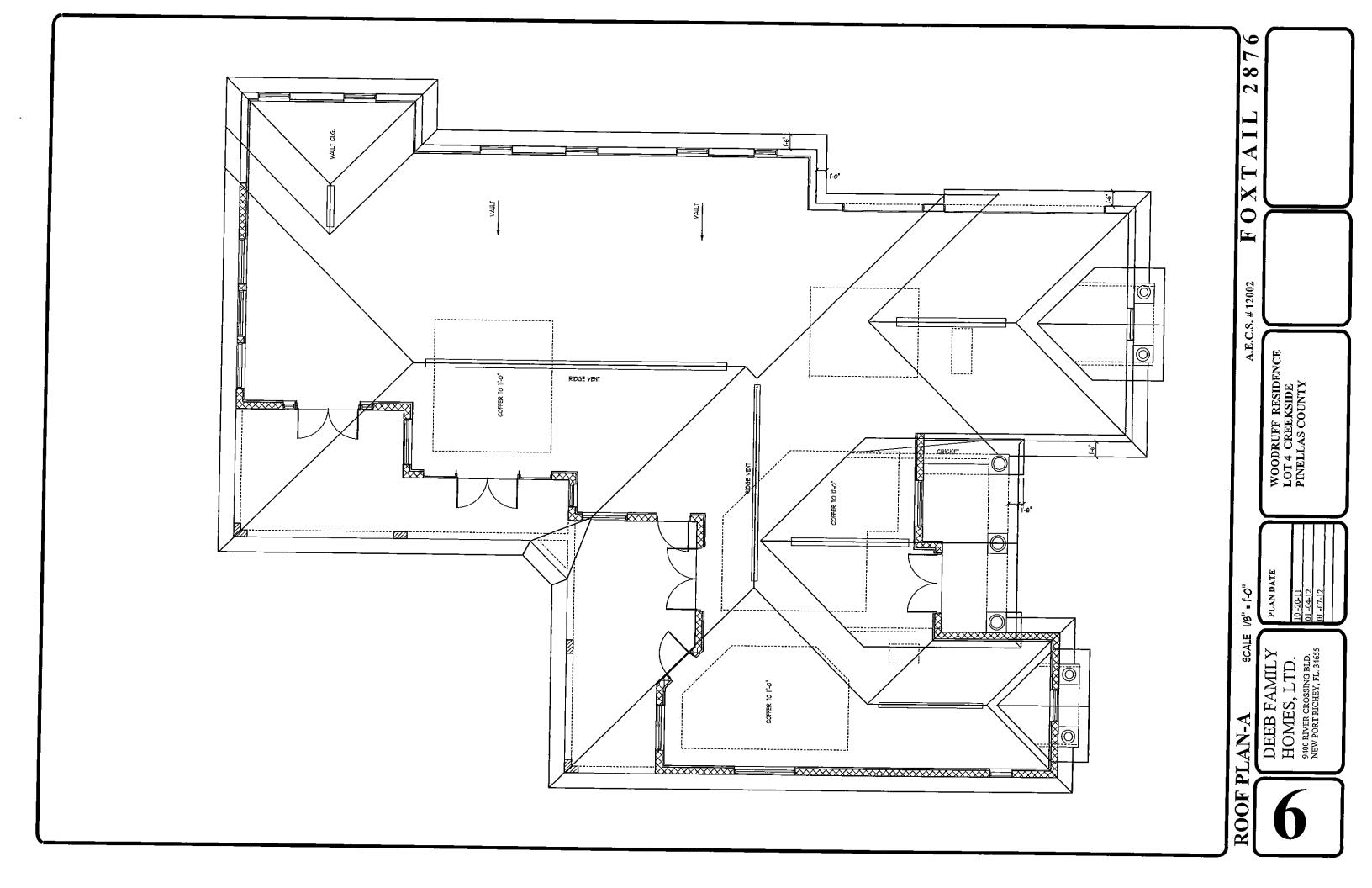
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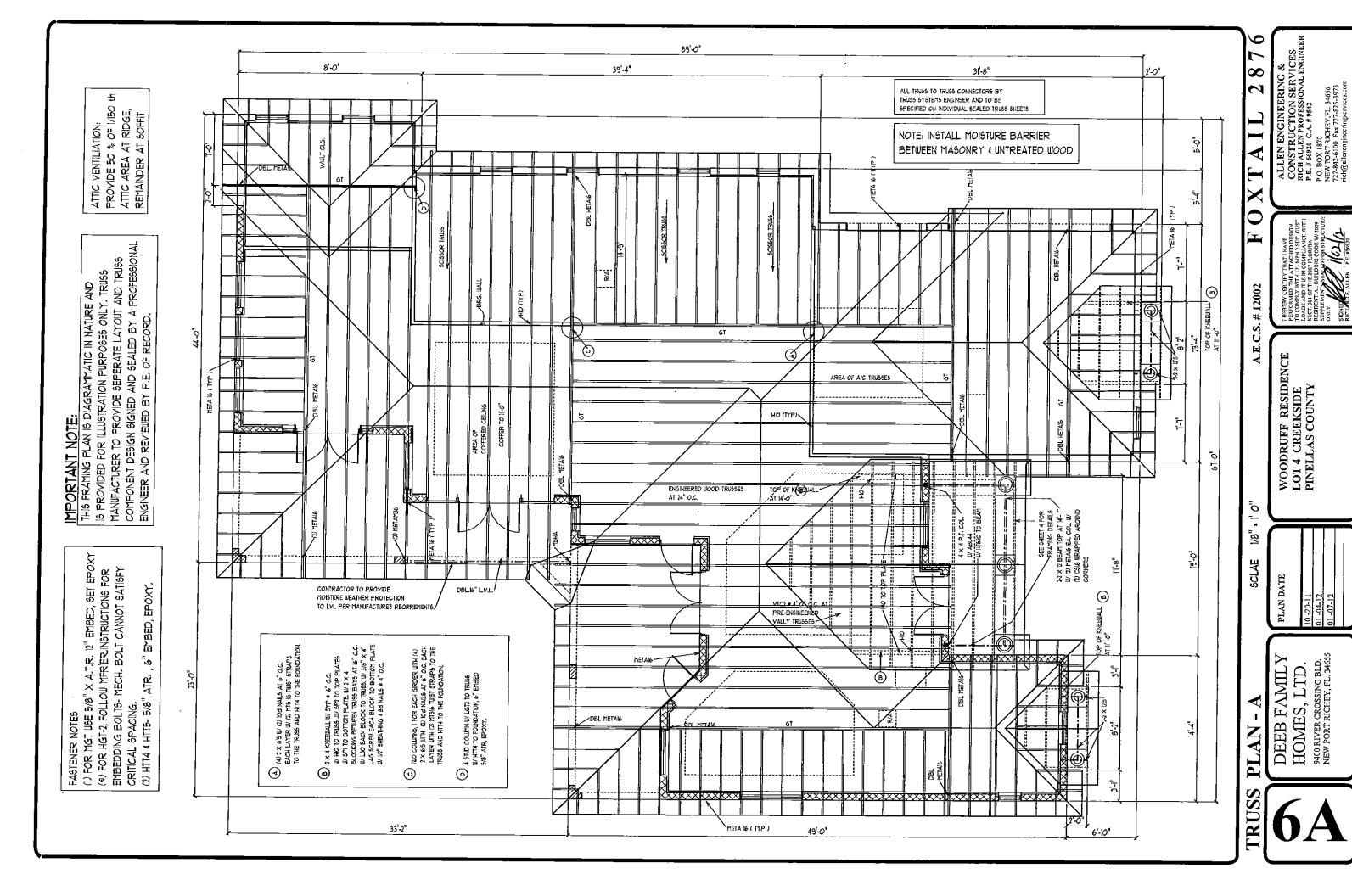
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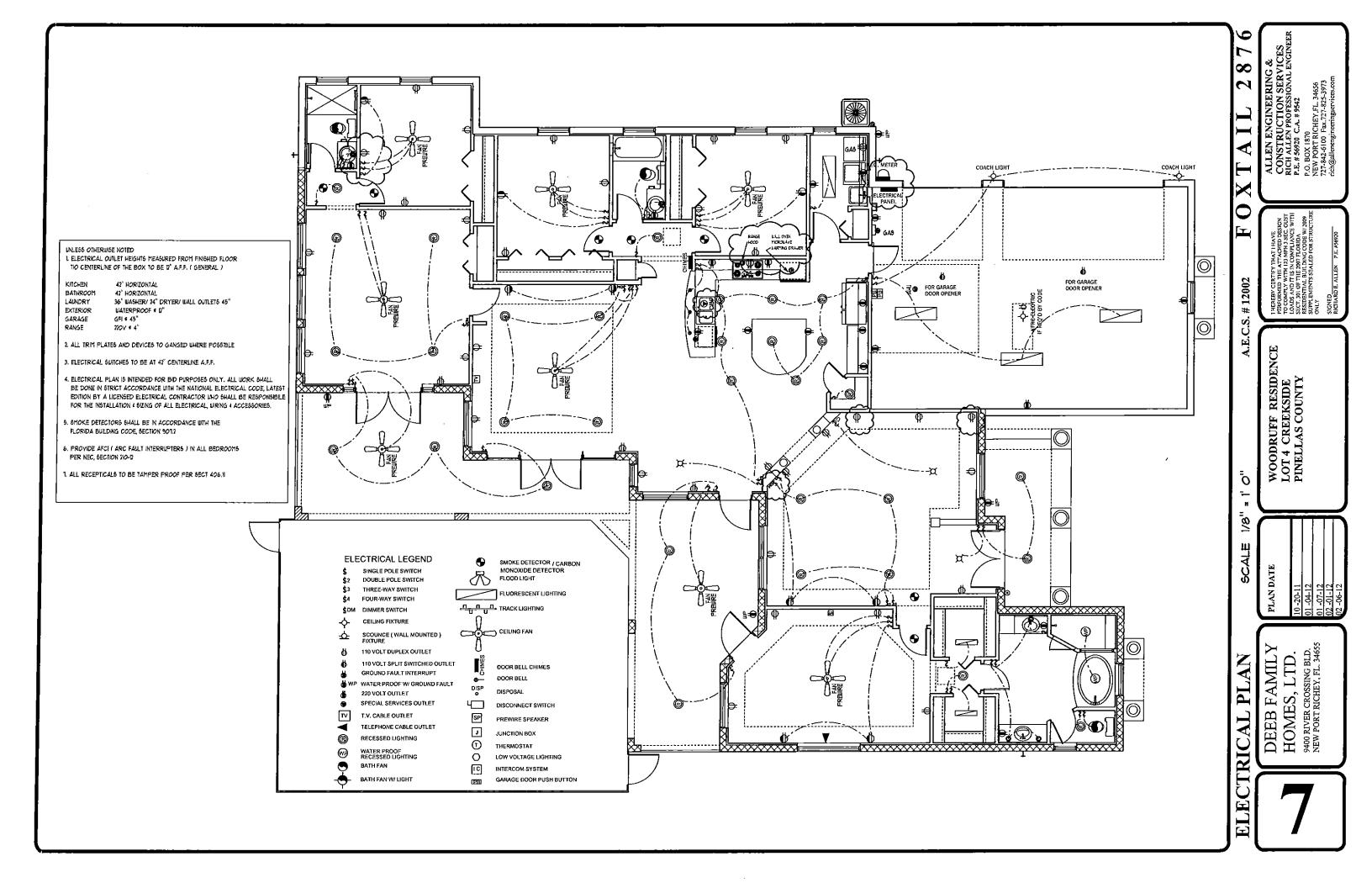
PLAN DATE

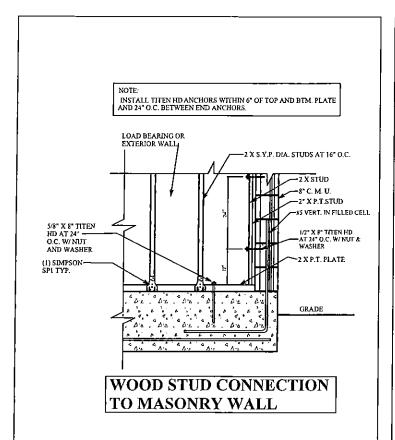
DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655

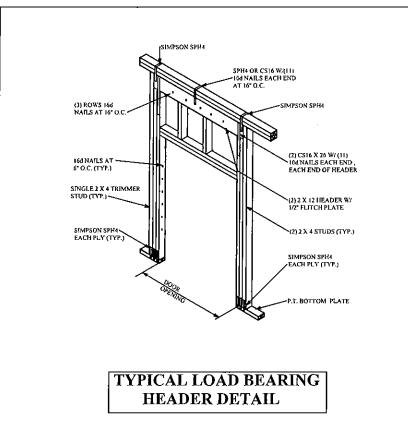
EXTERIOR ELEVATIONS

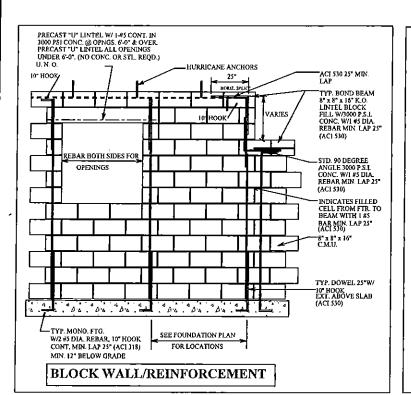


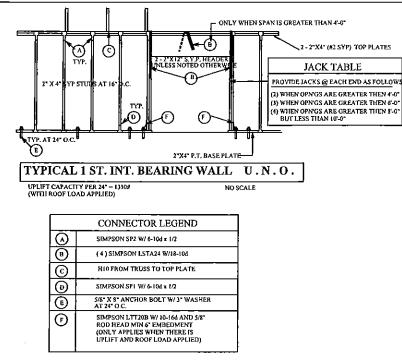


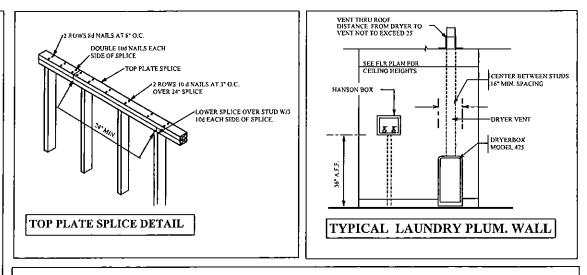


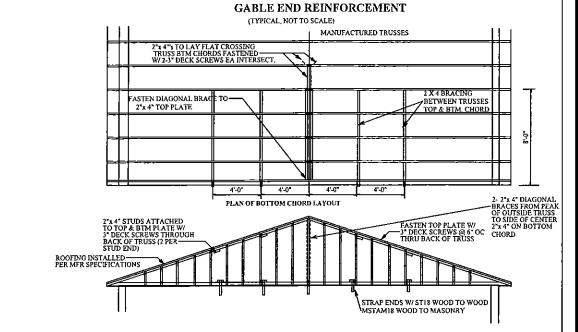


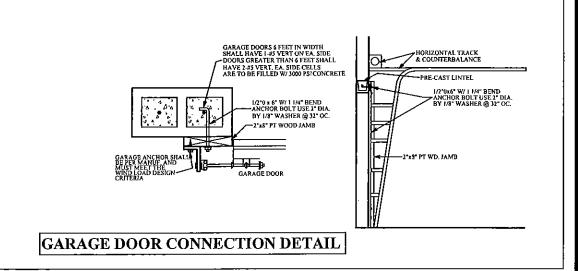














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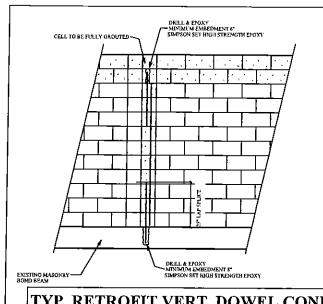
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DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655

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TYP. RETROFIT VERT. DOWEL CONDITION

NOTE:
MISSING DOWELS: WHERE FOOTING DOWELS ARE PLACED INCORRECTLY OR MISTAKENLY ELIMINATED,
REPLACE DOWEL AT PROPER LOCATION W/ GRADE 40 #5 BAR. INSTALL IN SLAB W/ 8" MINIMUM
EMBEDMENT, USE EPOXY GROUT.

MISSING ANCHOR BOLTS AT BEARING WALL:

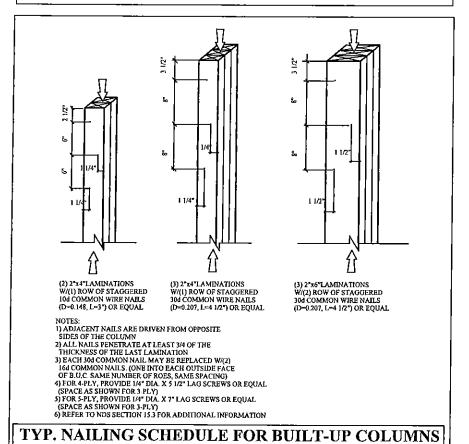
EXTERIOR BEARING WALL:

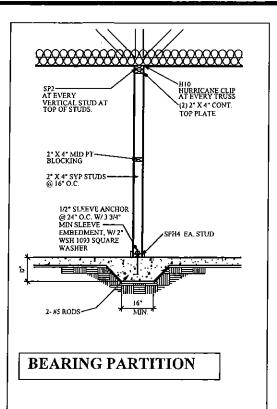
IN ADDITION TO THE GENERAL PLACEMENT REQUIREMENTS:

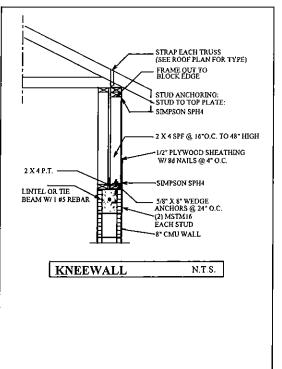
1.) 5/8" DIAMETER x 6" EMBEDMENT SIMPSON TITEN HD ANCHORS SPACED A MAXIMUM OF 24" O.C. INTERIOR BEARING WALL:

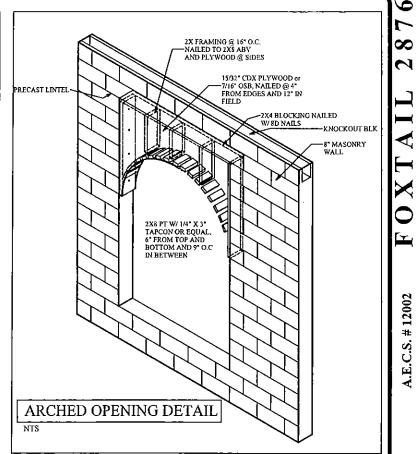
IN ADDITION TO THE GENERAL PLACEMENT REQUIREMENTS:

5.8" DIAMETER x 6" EMBEDMENT SIMPSON TITEN HD ANCHORS SPACED A MAXIMUM OF 24" O.C. IF RESISTING UPLIFT LOADS OR 3 1/2" EMBEDMENT AT 48" O.C. IF RESISTING GRAVITY LOADS









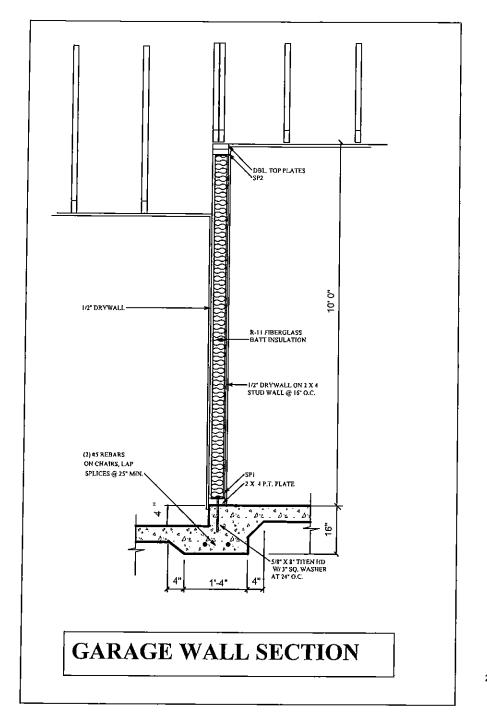


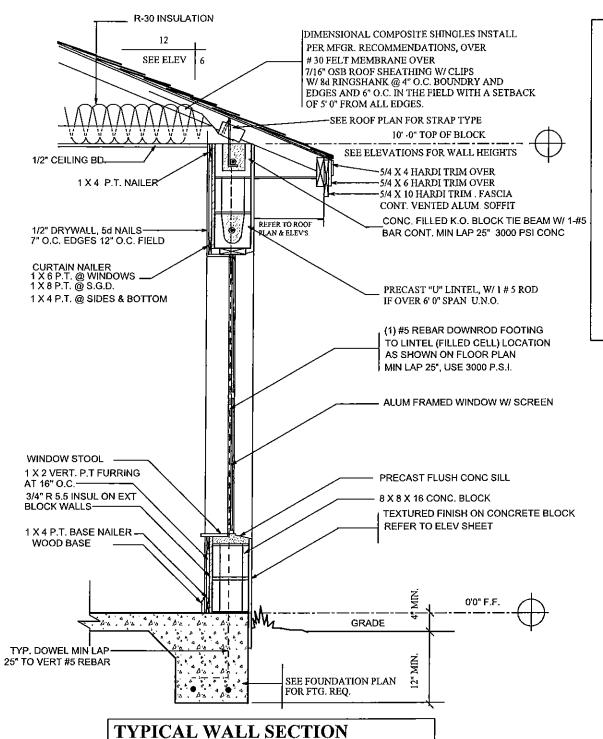
ALLEN ENGINEERING & CONSTRUCTION SERVICES
RICH ALLEN PROFESSIONAL ENGINEER
P.E. # 56920 C.A. # 9542
P.O. BOX 1870
NEW PORT NICHEY, FL. 34656
727-842-6100 Fax. 727-825-3973
rich@allenengineeringservices.com

PLAN DATE

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TRUCTION DET





TERMITE SPECIFICATIONS:

INSTALL "BORA-CARE" TERMITE PROTECTION SYSTEM PER MANUF'. SPECIFICATIONS

CONNECTOR TABLE

STANDARD FASCIA

2 X 6 SUB

FASCIA

STANDARD FASCIA ACROSS

REAR AND SIDES

5/4 X 8 PRIM TRIM

SIMPSON	FLORIDA PRODUCT NUMBERS PER INDEX 3-27-09
MBHA3.56/11.88 H2 H6 H10 LGT2 MGT LSTA/18 LSTA/24 SP1 SP2 HTS20 HTS20 META16 L30 MSTAM/24 MSTAM/26 MSTAM/36 MSTAM/36 MSTAM/36 MSTCM/60 CS16 SPH4 SPH6 HTT4 HTT5 ABU66	10868.12 10456.10 10456.16 10456.6 11470.6 11470.7 10852.4 10852.4 10456.42 10456.42 10456.23 10456.23 10455.22 11473.17 10446.11 11473.19 11473.19 11473.19 11473.19 11473.19 11473.19 11473.19 11473.19 11473.19 11473.19

WOODRUFF RESIDENCE LOT 4 CREEKSIDE PINELLAS COUNTY

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RUCTION DET

